PROJECT	DESIGNATION
1593276	1593276
CONTRACT	BRIDGE FILE
B-42017	(421) 39-12-01792C

STRUCTURE INFORMATION				
STRUCTURE	TYPE	SPAN AND SKEW	OVER	STATION
(421) 39-12-01792C	Steel Truss Bridge with Prestressed Concrete Box Beam End Spans	3 Spans: 30'-8½", 125'-0", 30'-8½" Skew: Square	South Fork Wildcat Creek	Sta. 43+50.00 Line "A"

# INDIANA DEPARTMENT OF TRANSPORTATION



# BRIDGE REHABILITATION PLANS

FOR SPANS OVER 20 FEET

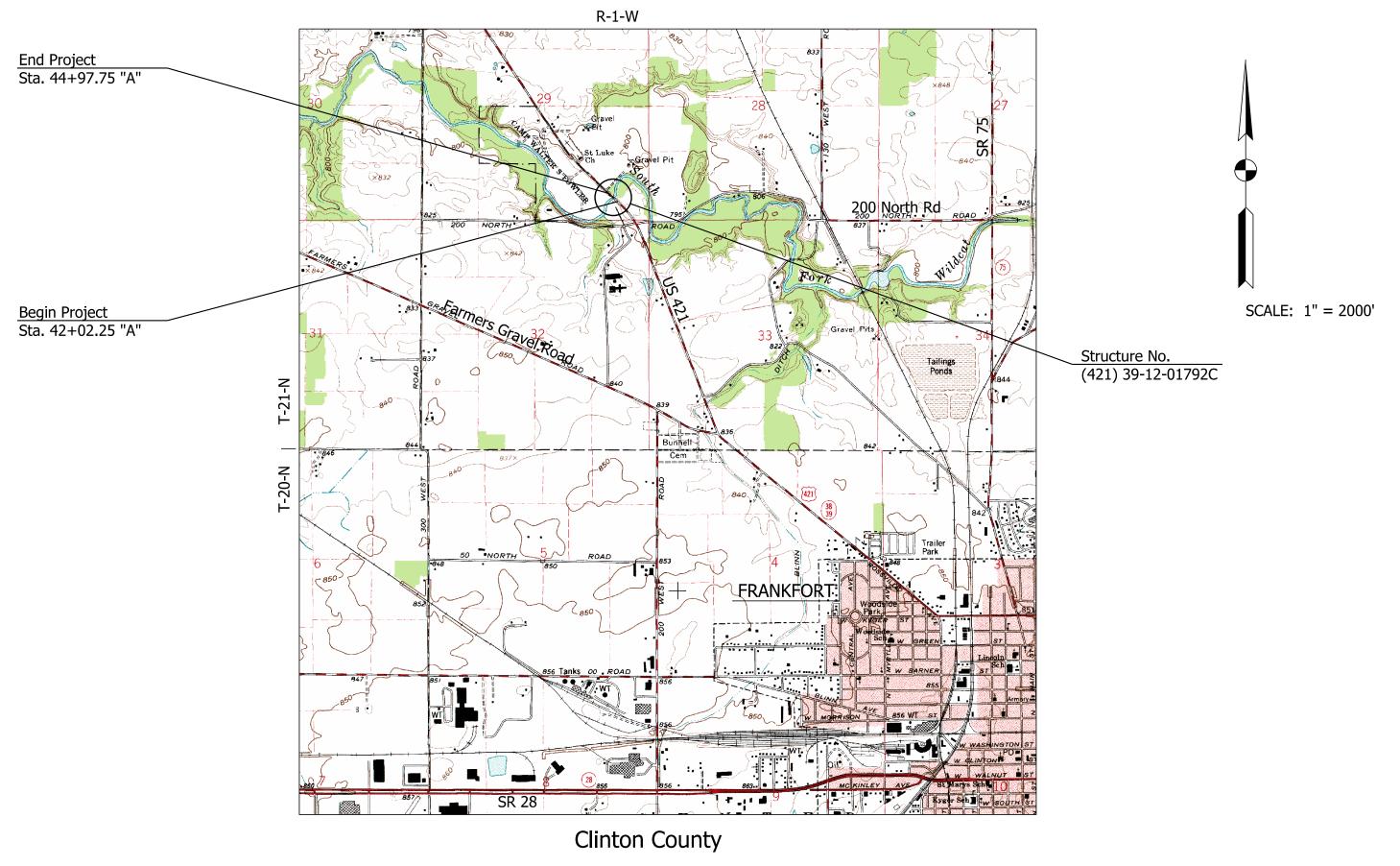
ROUTE: US 421 AT: RP 126+82

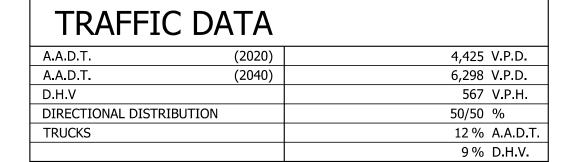
PROJECT NO. 1593276 P.E.

NO ADDITIONAL RIGHT-OF-WAY REQUIRED FOR THIS PROJECT

R/W 1593276 CONST.

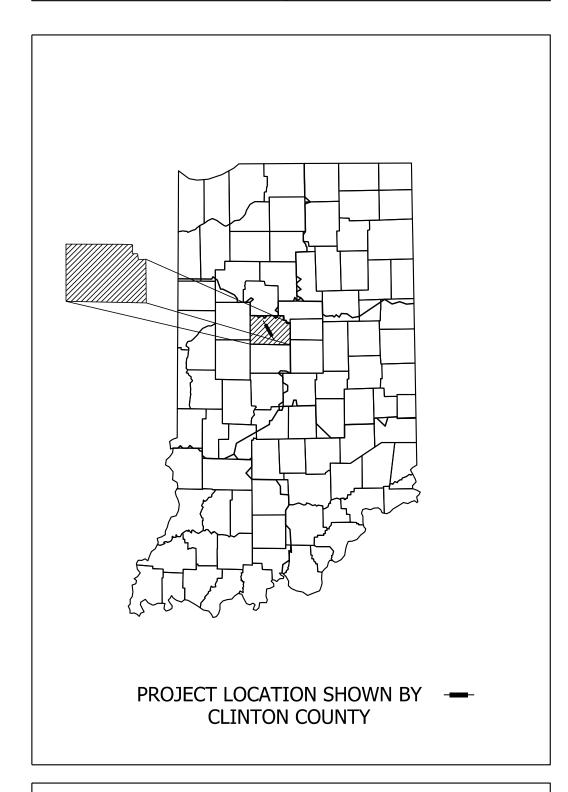
Bridge Deck and End Spans Replacement on US 421 over South Fork Wildcat Creek, located 2.24 Miles south of SR 38, in Section 29, T-21-N, R-1-W, Union Township, Clinton County, Indiana





#### **DESIGN DATA**

DESIGN SPEED	55 M.P.H.
PROJECT DESIGN CRITERIA	3R (NON-FREEWAY)
FUNCTIONAL CLASSIFICATION	MINOR ARTERIAL
RURAL/URBAN	RURAL
TERRAIN	LEVEL
ACCESS CONTROL	NONE



LATITUDE: 40° 18' 59" N LONGITUDE: 86° 32' 48" W

BRIDGE LENGTH:	0.037	N
ROADWAY LENGTH:	0.020	N
TOTAL LENGTH:	0.057	 N
MAX. GRADE:	0.0	 9



#### gai consultants

Indianapolis: 201 North Illinois Street, Suite 1700, Indianapolis, IN 46204 Fishers: 9998 Crosspoint Boulevard, Suite 110, Indianapolis, IN 46256

INDIANA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS DATED 2020 TO BE USED WITH THESE PLANS.

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	(421)	39-12-01	792 C
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	1593276		
		SHEETS	
	1	of	30
CONTRACT		PROJECT	
B-42017		1593276	

PLANS
PREPARED BY:
GAI Consultants, Inc.

CERTIFIED BY:
APPROVED
FOR LETTING:
INDIANA DEPARTMENT OF TRANSPORTATION

GAI Consultants, Inc.

(317) 436-9150
PHONE NUMBER

DATE

# Communications AT&T - Distribution 116 E Taylor St Kokomo, IN. 46901 Curtis Miller (765) 454-4054 cm5432@att.com Electric Frankfort City Light & Power 1000 Washington Avenue Frankfort, IN 46041 Michael Myers (765) 654-4424 mmyers@fmu-in.com

	DEL/TOTOLIC				
		REVISIONS			
SHEET NO.	DATE	REVISED			

#### **GENERAL NOTES**

Roadway stationing is based off of Bridge Contract No. 2097 original construction plans, dated 1940.

All disturbed areas shall be mulch seeded, unless noted otherwise.

INDEX			
SHEET NO.	DRAWINGS INDEX		
1	TITLE		
2	INDEX		
3	MAINTENANCE OF TRAFFIC		
4	EROSION CONTROL DETAILS		
5	LAYOUT		
6-7	GENERAL PLAN		
8-9	REMOVAL DETAILS		
10-11	ABUTMENT NO. 1 DETAILS DETAILS		
12-13	PIER NO. 2 & NO. 3 DETAILS		
14-15	ABUTMENT NO. 4 DETAILS DETAILS		
16-19	STRUCTURAL STEEL DETAILS		
20	FRAMING PLAN		
21	BEAM DETAILS		
22-25	SUPERSTRUCTURE DETAILS		
26	DRAINAGE DETAILS		
27	CONCRETE RAILING DETAILS		
28	APPROACH SLAB DETAILS		
29	BRIDGE SUMMARY TABLES		
30	ROADWAY SUMMARY TABLES		

#### **BENCH MARKS:**

CLIN C-22 - Located on US 421/SR 39/SR 38, 2.2 miles southeast of SR 38 north split on top of Pier Cap #3 (northwest corner) of bridge over South Fork Wildcat Creek. Elevation 792.60

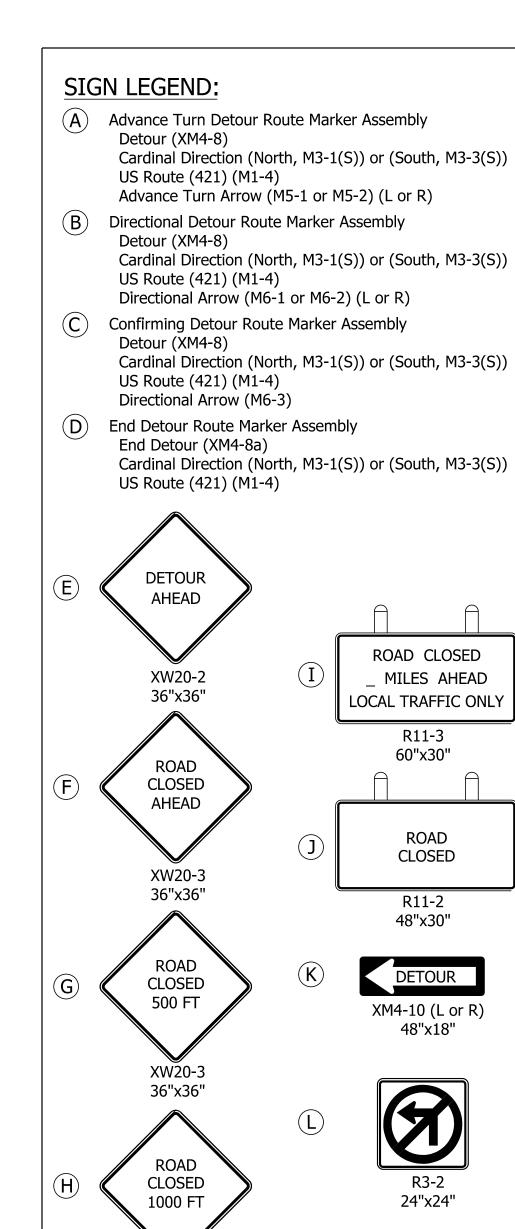
TBM #1 - Top of mag spike set in Westerly Side of power pole #6765 on N. Side of US 421 at Northwest end of bridge. Elevation: 789.91

TBM #2 - Top of mag spike set in Westerly Side of power pole #6768 on N. Side of US 421 at Southeast end of bridge. Elevation: 792.07

REFERENCE POINT #1	REFERENCE POINT #2	REFERENCE POINT #3
S.E. CORNER GUARD RAIL POST  N.E. CORNER AT EDGE OF PAV'T	S.W. CORNER CONCRETE WALL  S.W. CORNER GUARD RAIL POST  S.W. CORNER GUARD RAIL POST  S.W. CORNER DELINEATOR POST	MAG SPIKE FOUND SOUTHERLY SIDE POWER POLE# 6768 (TBM #2)  N.W. CORNER SIGN POST  N.W. CORNER GUARD RAIL POST
N. 178626.909 / E. 802438.340	N. 178727.531 / E. 802328.853	N. 178938.159 / E. 802063.401
5/8" REBAR W/"TRAV PT GAI 0007"CAP SET	MAG SPIKE W/"GAI 0007" BRASS DISK SET	5/8" REBAR W/"TRAV PT GAI 0007"CAP SET



RECOMMENDED FOR APPROVAL DESIGN ENGINEER DATE		INDIANA DEPARTMENT OF TRANSPORTATION	SCALE	BRIDGE FILE	
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XM4-10 (L or R) 48"x18"



#### **LEGEND**

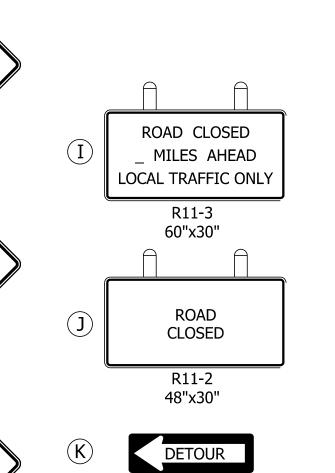
**Detour Route** 

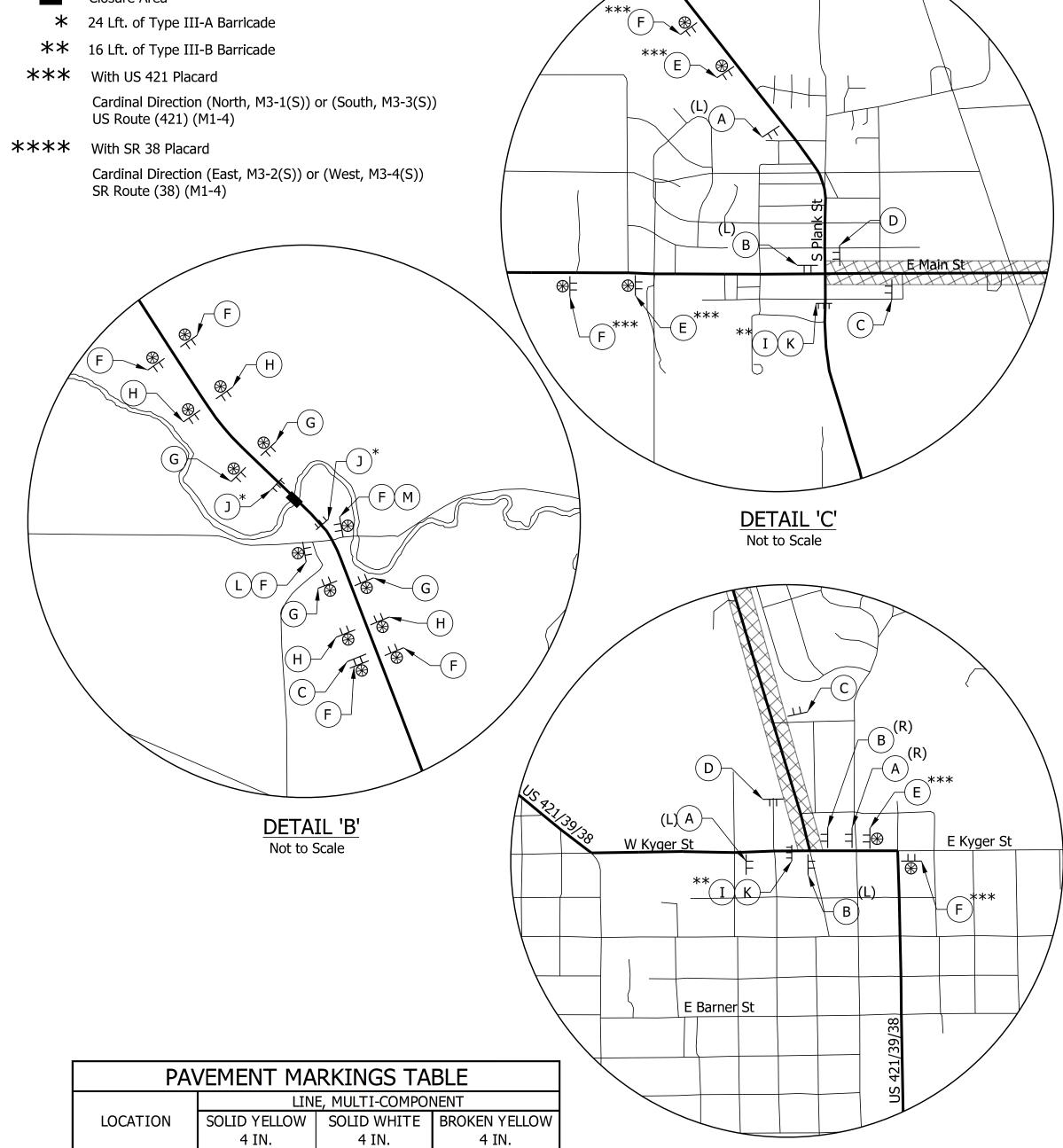
> Construction Sign Low Intensity Construction Warning Light, Type A

Type III Barricade

Closure Area

\*\*\* With US 421 Placard



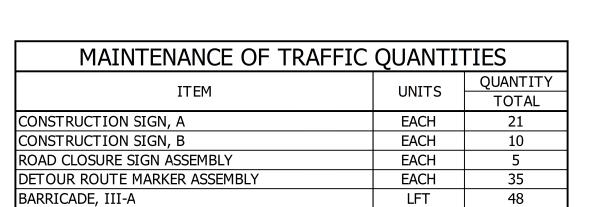


Rossville C			26)
C - C	$\begin{array}{c c}  & \times $		
See Detail 'C'	,		
	C		
		C	
421			
	(L) <sub>A</sub>	B (R) (R) (L)	4 E00 M
****(E) (L) (B)		B (L)	d 500 N
**** F (L) A	F	C	
W Co Rd 300 N	1**	C 75	Co Rd 300 N
		C	
Project Location —		C	— See Detail 'A'
See Detail 'B'	C 38		Frankfort
	DETOUR PLAN  SCALE: 1" = 5,000'		Н

#### NOTES:

XW20-3 36"x36"

- 1. Refer to INDOT Standard Drawing E 801-TCDT-01 for placement of signs and devices in a rural detour and E 801-TCDT-04 for Detour Route Marker Assembly Details.
- 2. Two "Route Number Closed On or After \_\_\_\_" Signs (XG20-5) to be placed as directed by the engineer prior to construction.
- Upon completion of the project restripe the double yellow centerline and solid white edge lines.
- 4. Install R11-2 Road Closure sign assemblies on Type III-A barricades. Install R11-3 Road Closure sign assemblies on Type III-B barricades.
- Conduct flagging during paving operations in accordance with E 801-TCTC-05.



LFT

780

LFT

LFT

100

LFT

390

UNIT

BARRICADE, III-B

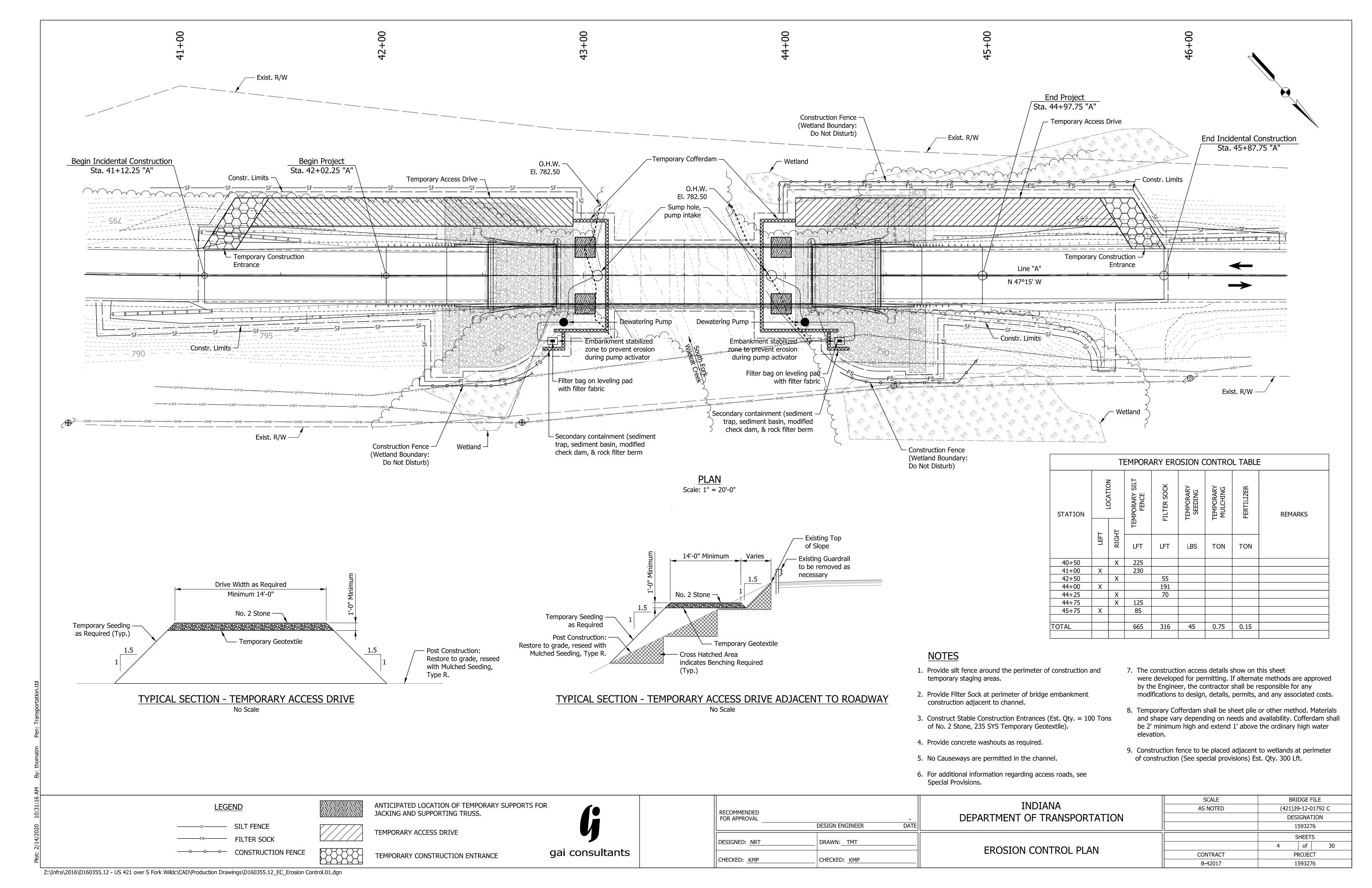
TOTALS

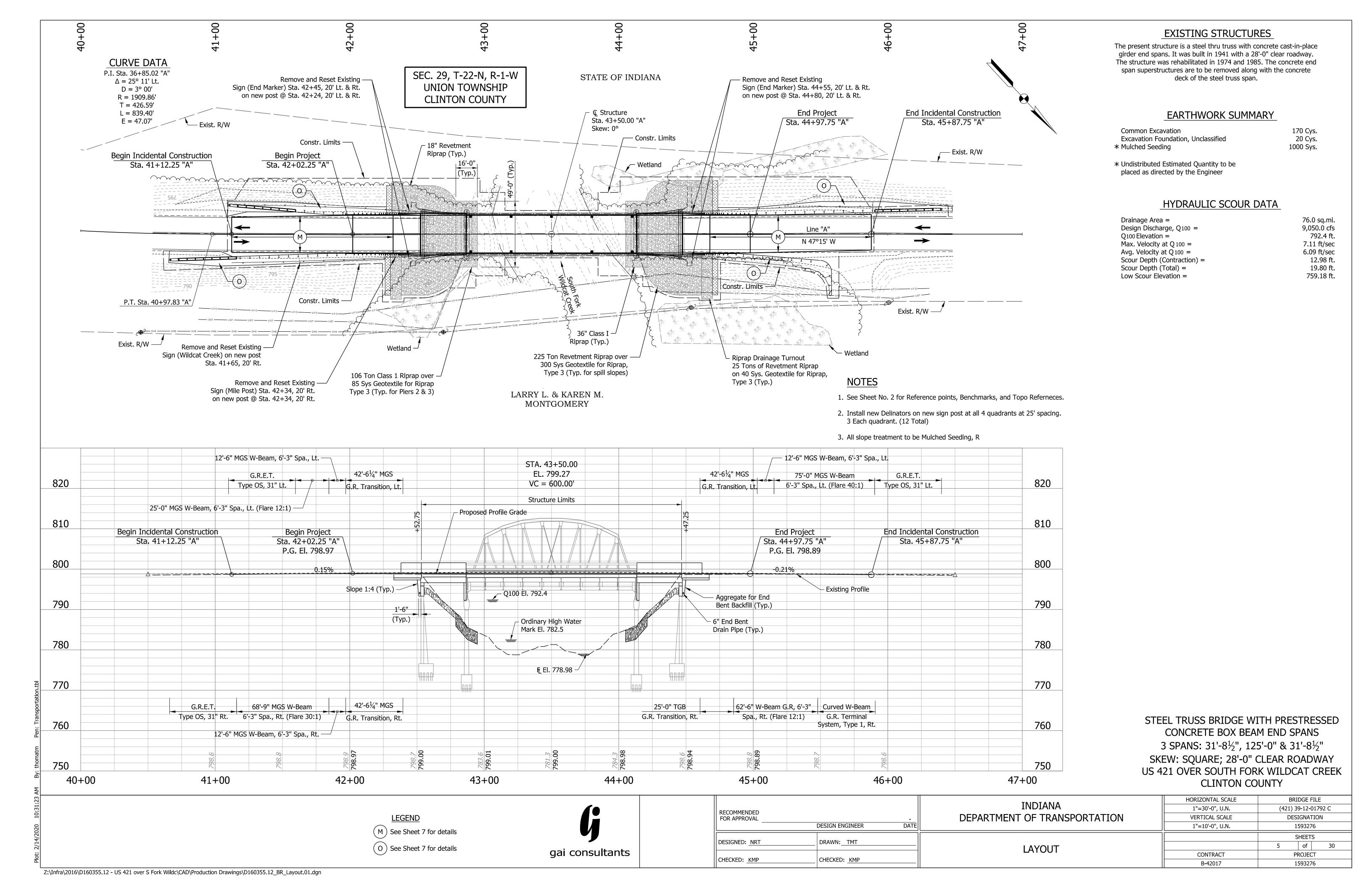
gai consultants

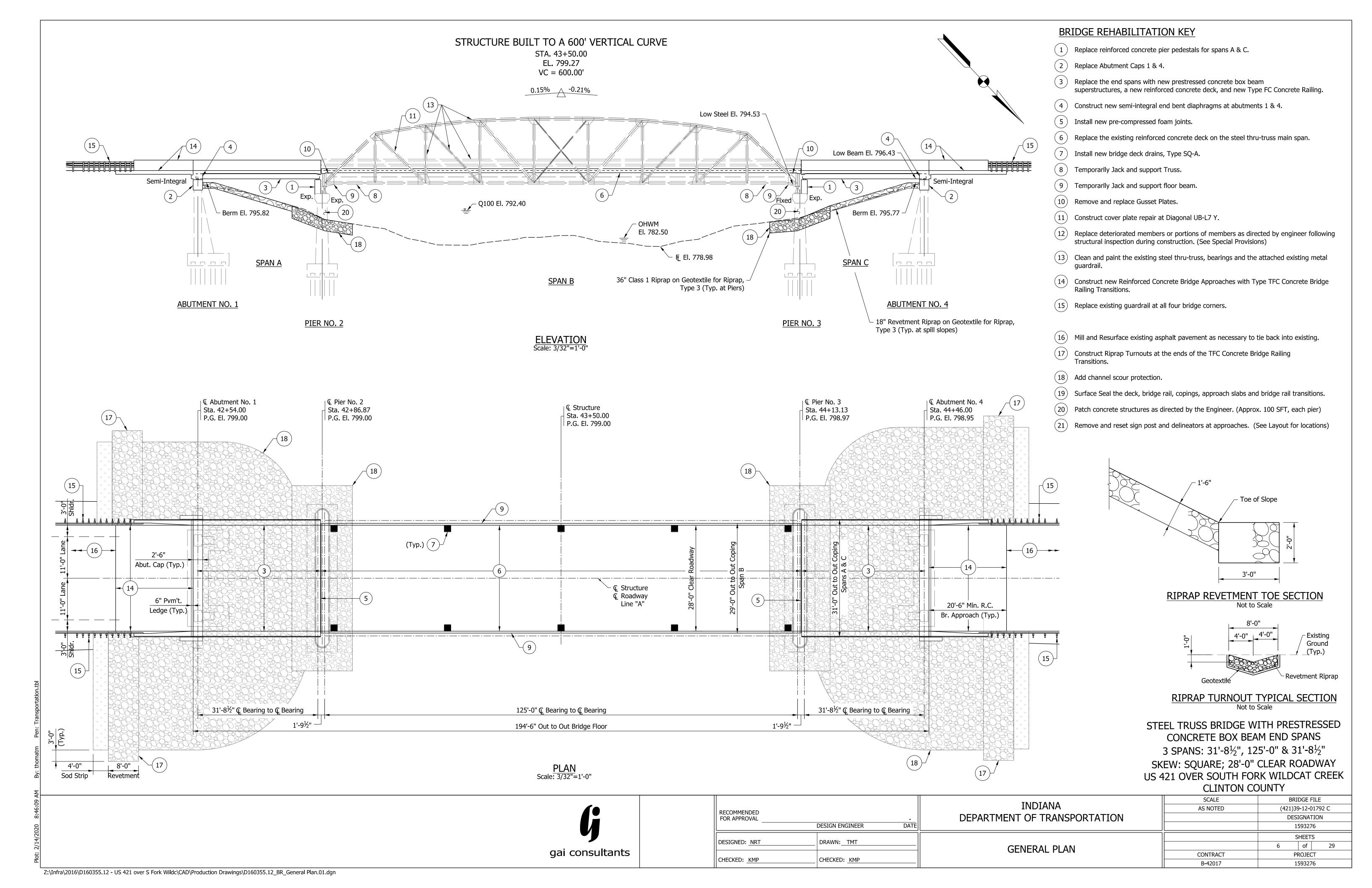
DETAIL 'A'

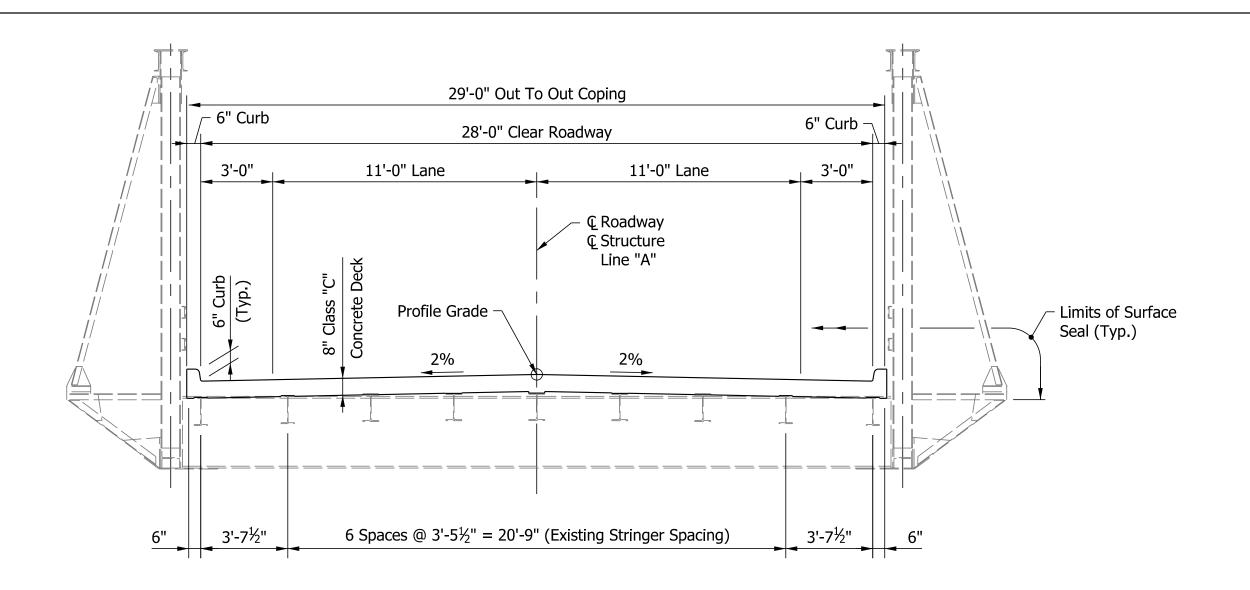
Not to Scale

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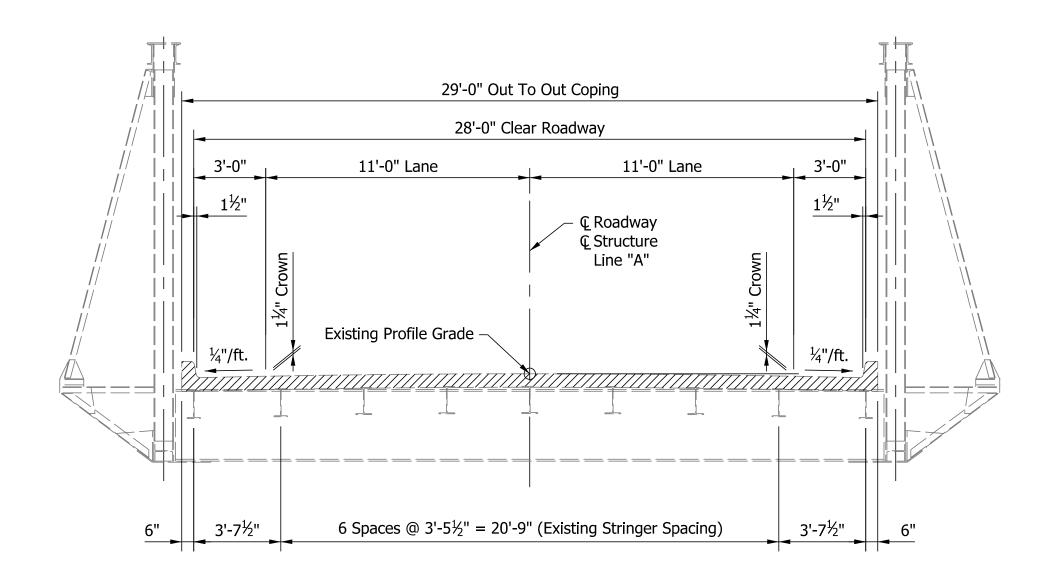


#### 31'-0" Out To Out Coping 28'-0" Clear Roadway 11'-0" Lane 3'-0" 3'-0" 11'-0" Lane Limits of Surface -Seal (Typ.) • **©** Roadway © Structure Line "A" Profile Grade -2% 2% - CB 17" x 48" Prestressed Concrete Box Beam (Typ.) 3'-0" 3'-0" 3 Spaces @ 8'-4" = 25'-0"

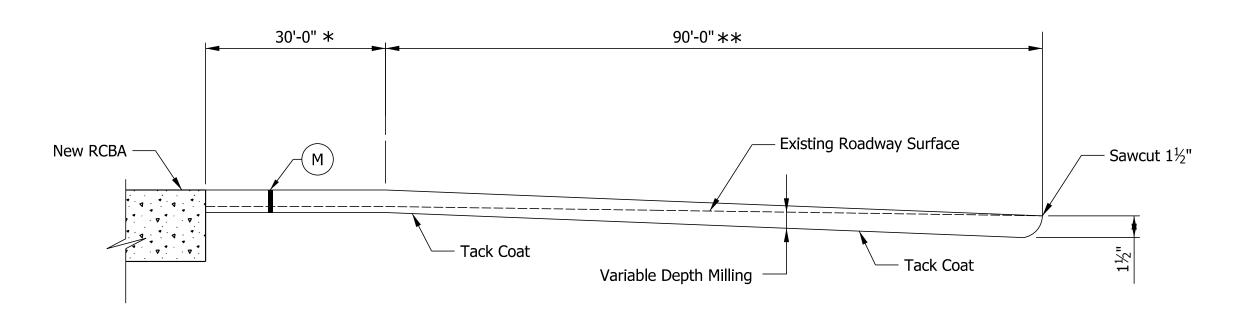
#### TYPICAL PROPOSED SECTION SPAN "B"

Scale: $\frac{1}{4}$ " = 1'-0"

#### TYPICAL PROPOSED SECTION SPANS "A" & "C" Scale: $\frac{1}{4}$ " = 1'-0"

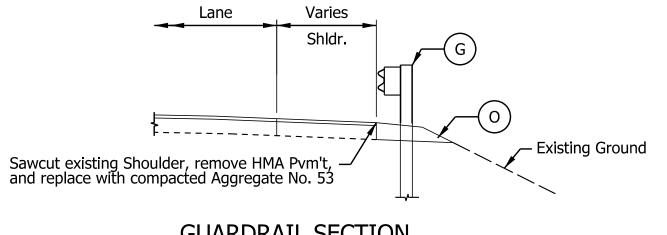


TYPICAL REMOVAL SECTION SPAN "B" Scale:  $\frac{1}{4}$ " = 1'-0"



\* - Wedge to be a continuation of bridge profile. \*\* - Incidental Construction.

#### HMA WEDGE AND LEVEL DETAIL Not to Scale



**GUARDRAIL SECTION** Appicable where new guardrail posts fall within existing shoulder footprint

#### **GENERAL NOTES**

Reinforcing steel cover shall be 2 1/2"in top and 1" minimum in bottom of floor slab, and 2" in all other parts, unless noted otherwise.

Plans for the existing structure are on file in the central office of the Indiana Department of Transportation as bridge file (421)39-12-01792, (421)39-12-01792 A, and (421)39-12-01792 B and are available upon request.

Where new work is to be fitted to old work, the contractor shall check all dimensions and conditions in the field, report any errors or discrepancies to the engineer and assume responsibilty for their correctness and the fit of the new part to the old.

#### **DESIGN DATA**

New Superstructure and deck designed for HL-93 loading in accordance with AASHTO LRFD Bridge Design Specifications Eighth Edition and interims through

#### DEAD LOAD

Actual weight plus 35 psf (composite) for future wearing surface and 15 (non-componsite) for permanent metal deck forms (New Spans Only).

#### FLOOR SLAB

Designed with a 7 1/2" structural depth plus a 1/2" sacrificial wearing surface.

#### **DESIGN STRESSES**

#### CONCRETE

Class "A" Concrete: f'c = 3,500 psiClass "C" Concrete: f'c = 4,000 psi

#### REINFORCING STEEL

Grade 60 Fy = 60,000 psi

#### CONSTRUCTION LOADING

The exterior beam has been checked for strength, deflection, and overturning using the construction loads shown below. Cantilever overhang brackets were assumed for support of the deck overhang past the edge of the exterior beam. Finishing machine was assumed to be supported 6 in. outside the vertical coping form. The top overhang brackets were assumed to be located 6 in. past the edge of the vertical coping form. The bottom of overhang brackets were assumed to be braced against the Box Beam Superstructure (New Spans Only).

#### DECK FALSEWORK LOADS

Designed for 15 lb/ft2 for permanent metal stay-in-place deck forms, removable deck forms, and 2-ft exterior walkways (New Spans Only).

#### CONSTRUCTION LIVE LOAD

Designed for 20 lb/ft2 extending 2 ft past the edge of coping and 75 lb/ft vertical force applied at a distance of 6 in. outside the face of coping over a 30-ft length of the deck centered with the finishing machine (New Spans Only).

#### FINISHING MACHINE LOAD

4500 lb distributed over 10 ft along the coping (New Spans Only).

#### WIND LOAD

Designed for 70 mph horizontal wind loading in accordance with LRFD 3.8.1 (New Spans Only).

> STEEL TRUSS BRIDGE WITH PRESTRESSED CONCRETE BOX BEAM END SPANS 3 SPANS: 31'-8½", 125'-0" & 31'-8½"

> SKEW: SQUARE; 28'-0" CLEAR ROADWAY US 421 OVER SOUTH FORK WILDCAT CREEK CLINITON COLINITY

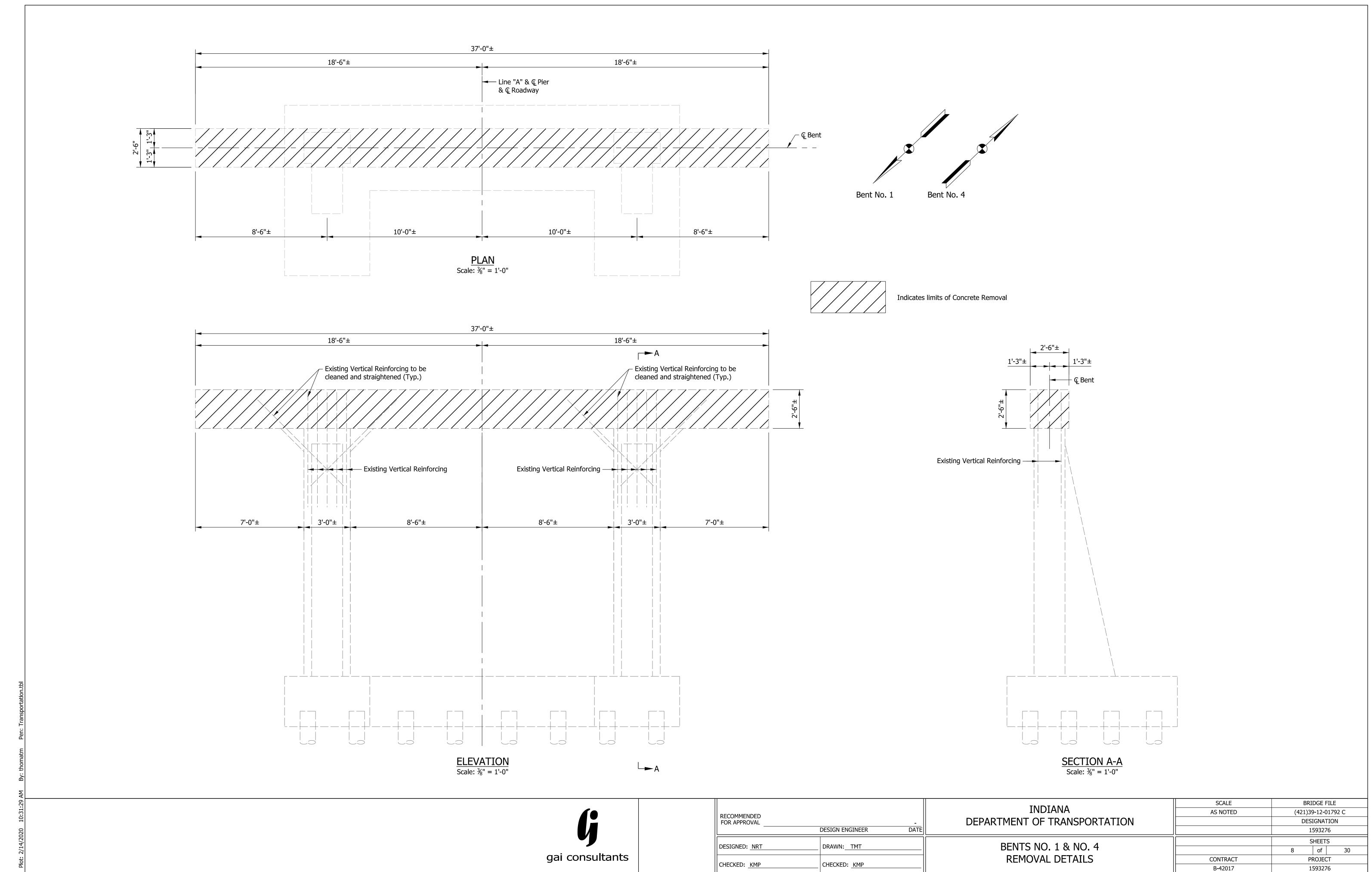
**LEGEND** 

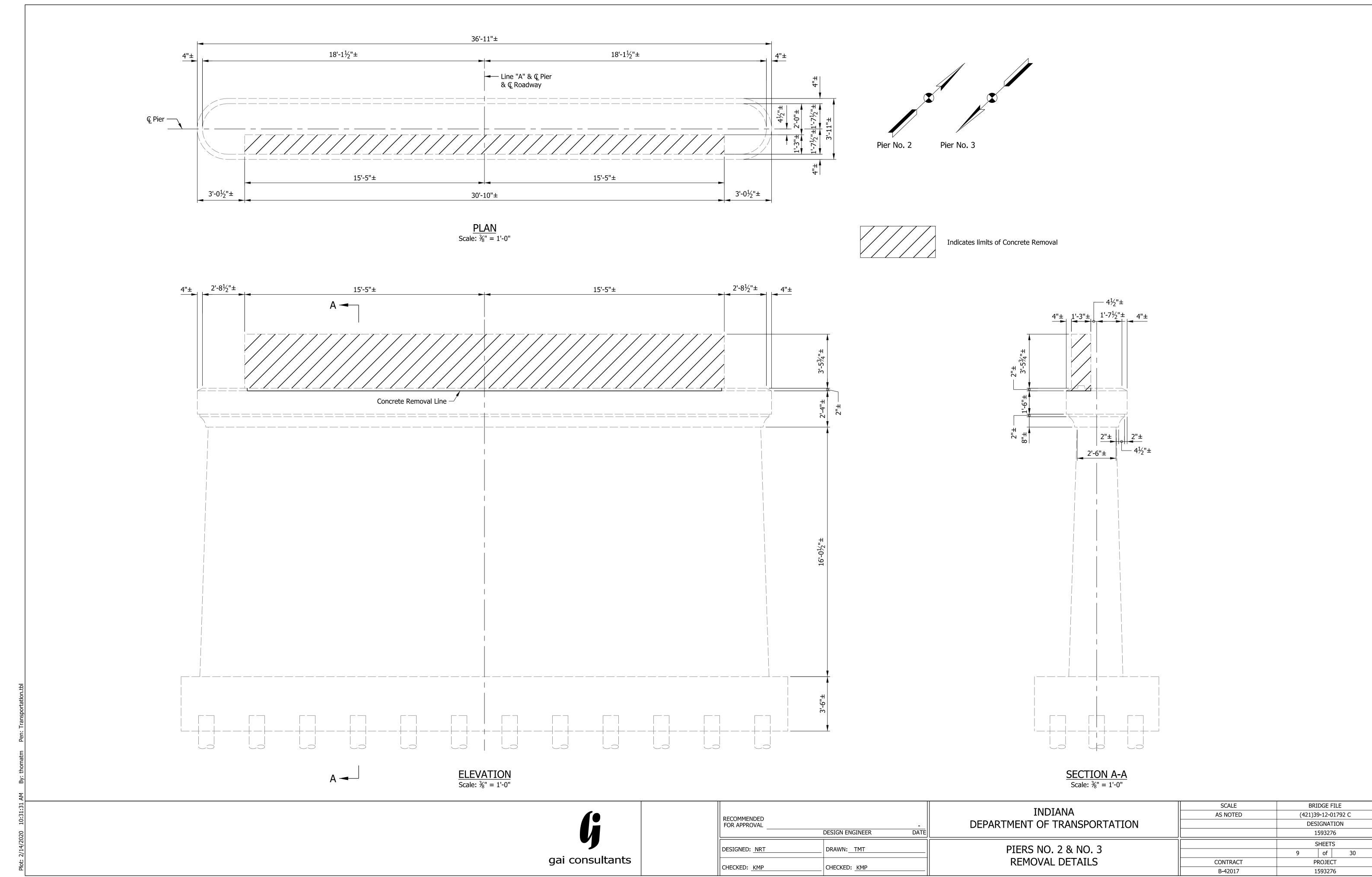
- G MGS Guardrail
- M Transition Milling and 165#/Sys. QC/QA-HMA, 3, 64, Surface, 9.5 mm
- O Compacted Aggregate No. 53 for Shoulders

gai consultants

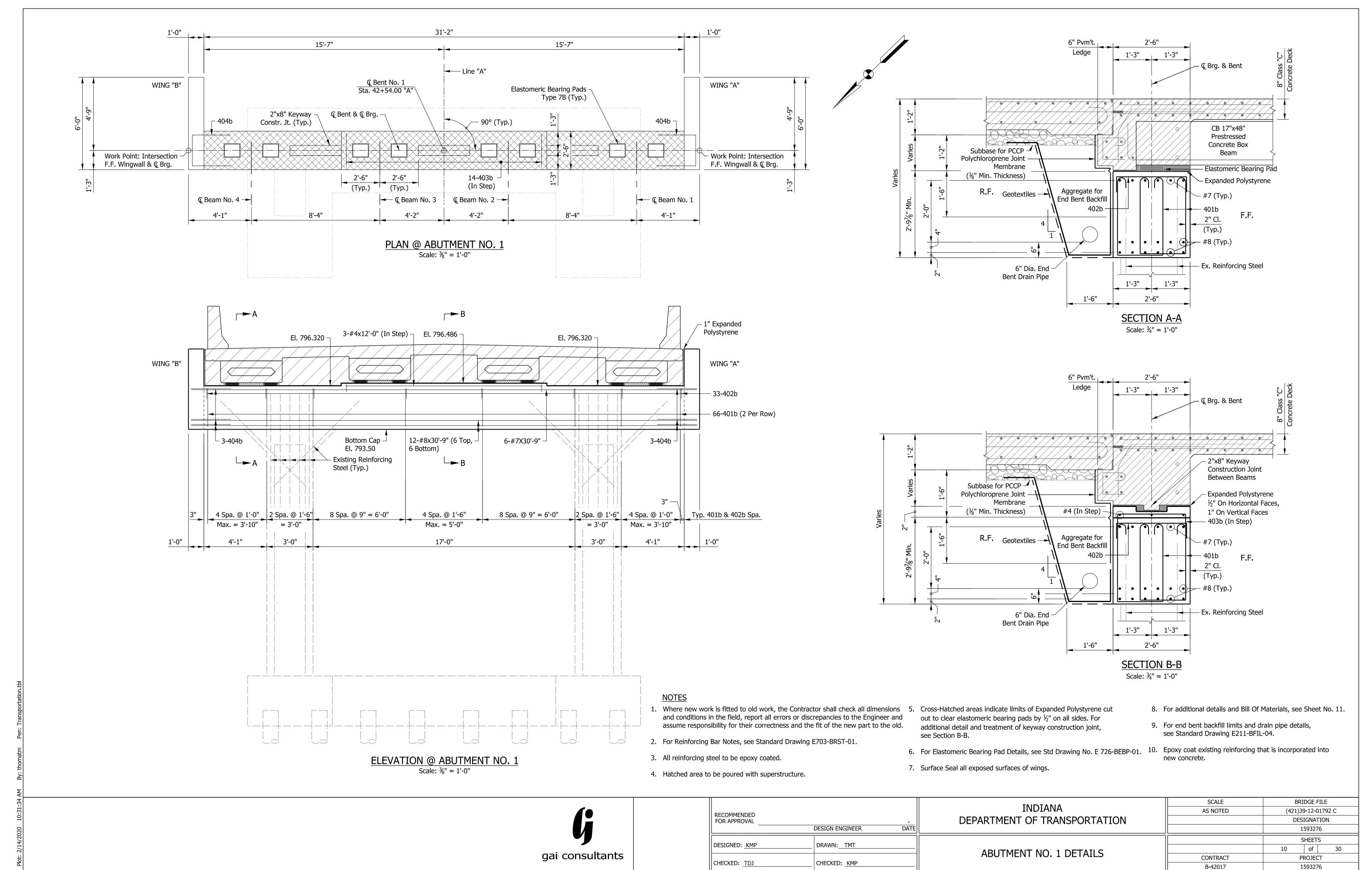
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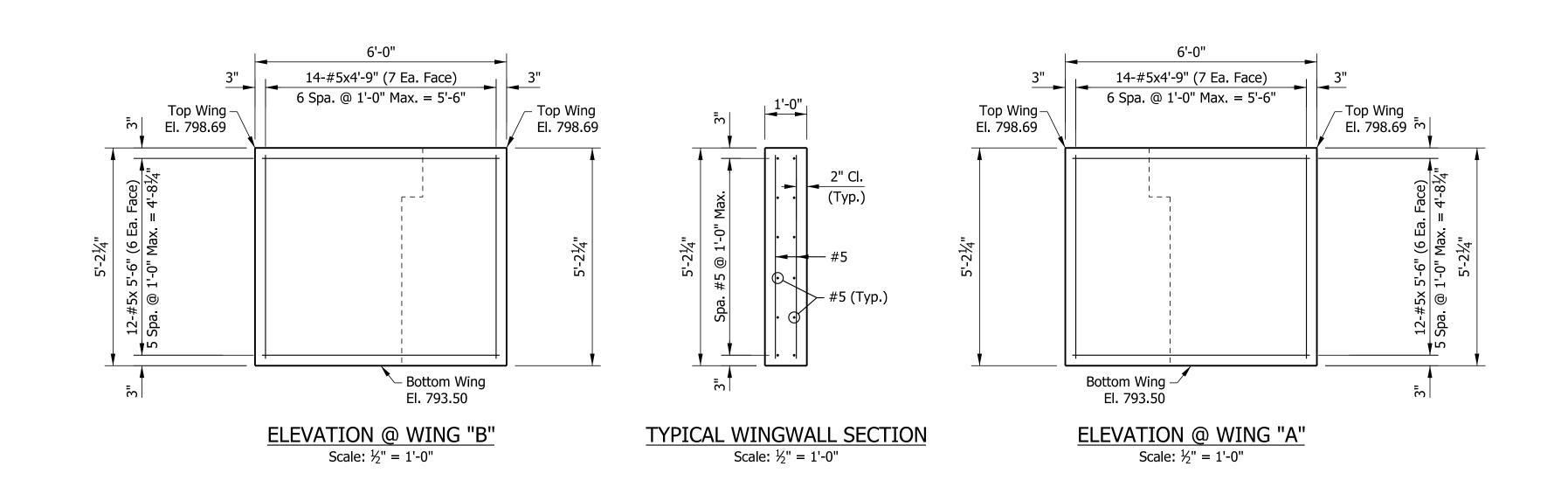
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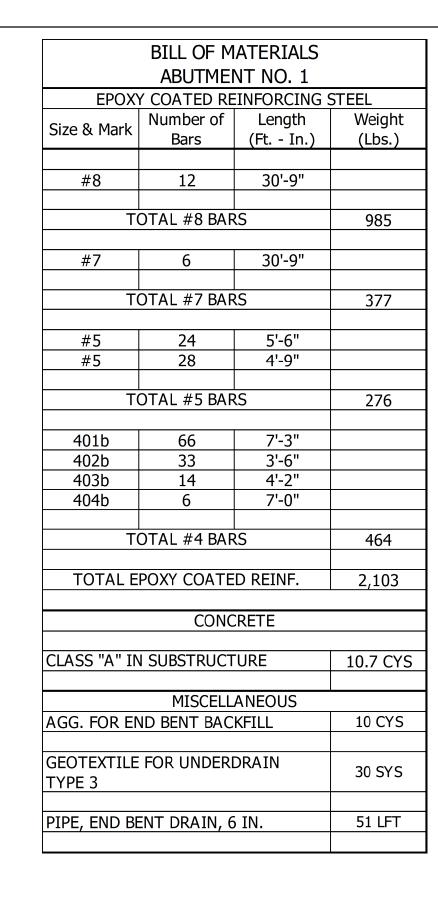


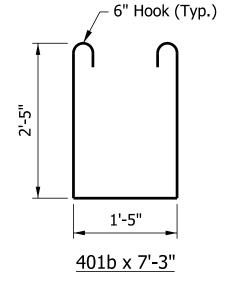


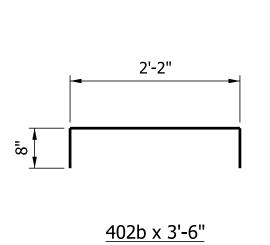
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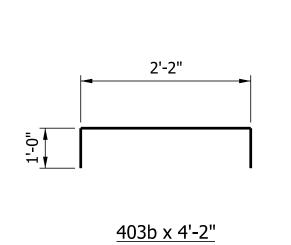


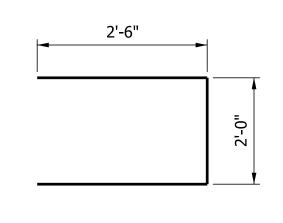












#### 404b x 7'-0"

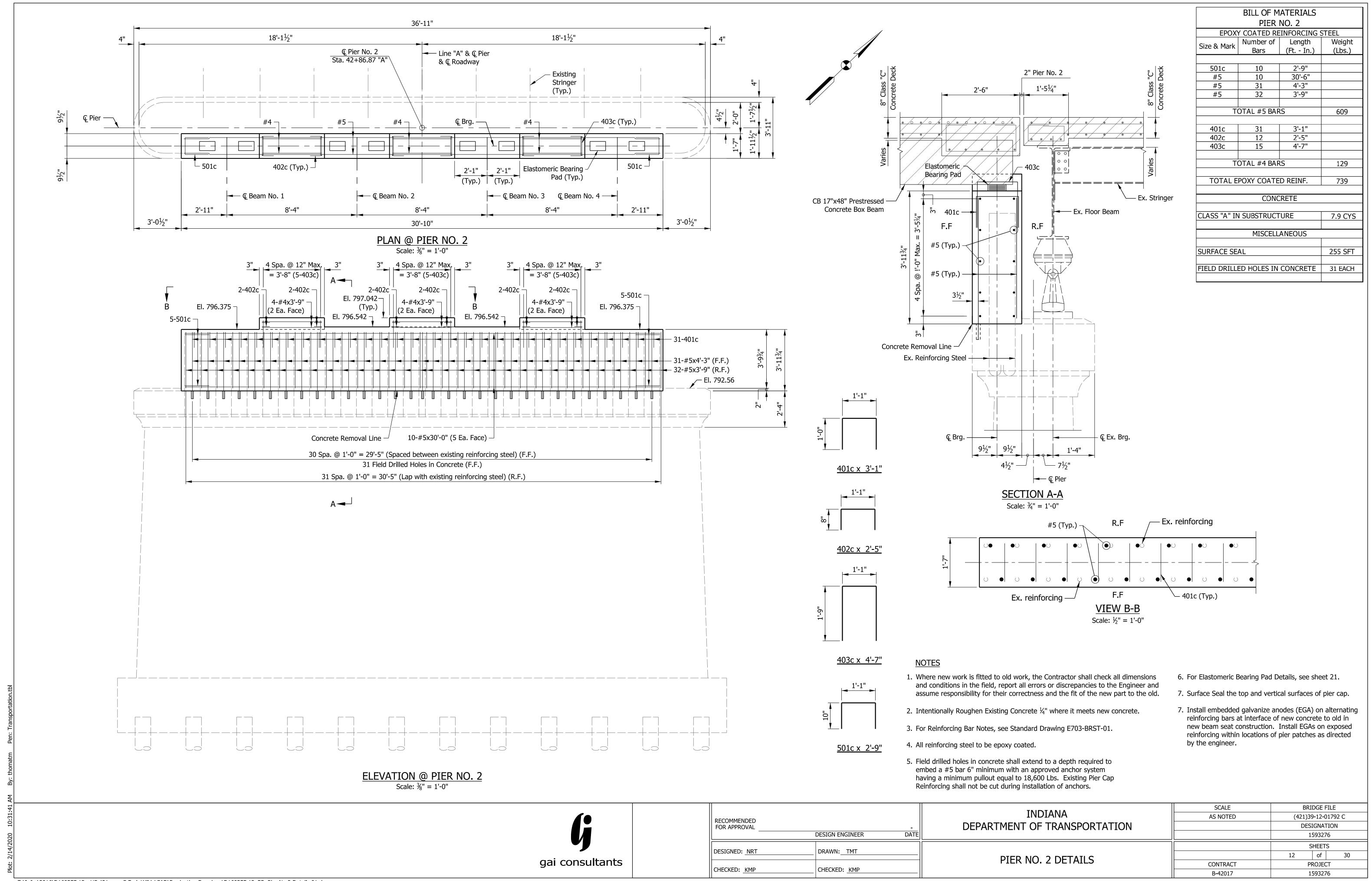
#### <u>NOTES</u>

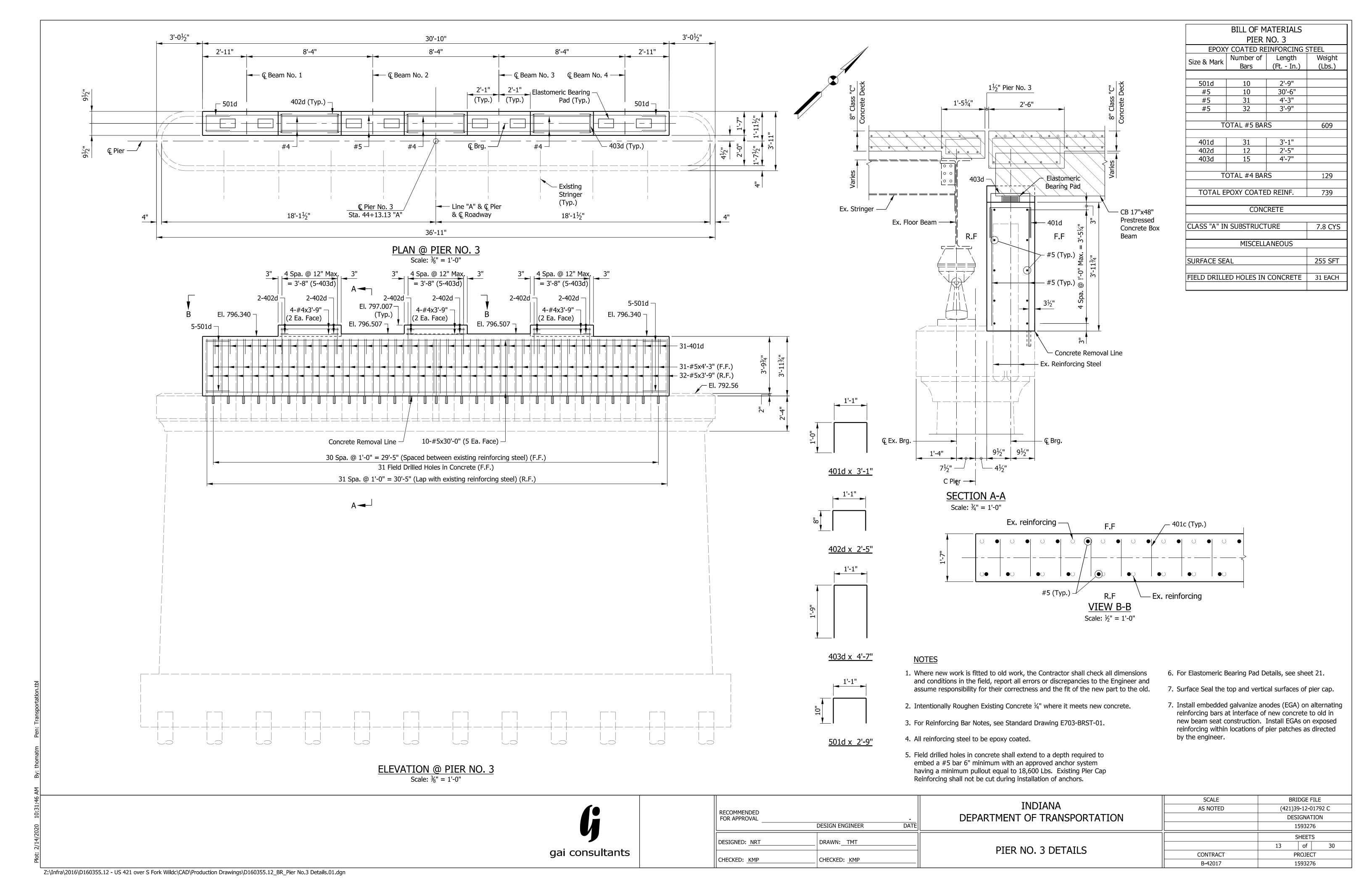
- Where new work is fitted to old work, the Contractor shall check all dimensions and conditions in the field, report all errors or discrepancies to the Engineer and assume responsibility for their correctness and the fit of the new part to the old.
- 2 For Reinforcing Bar Notes, see Std. Dwg. E 703-BRST-01.
- 3. All reinforcing bars shall be Epoxy Coated.
- 4. Epoxy coat existing reinforcing that is incorporated into new concrete.
- 5. Surface Seal all exposed surfaces of wings.

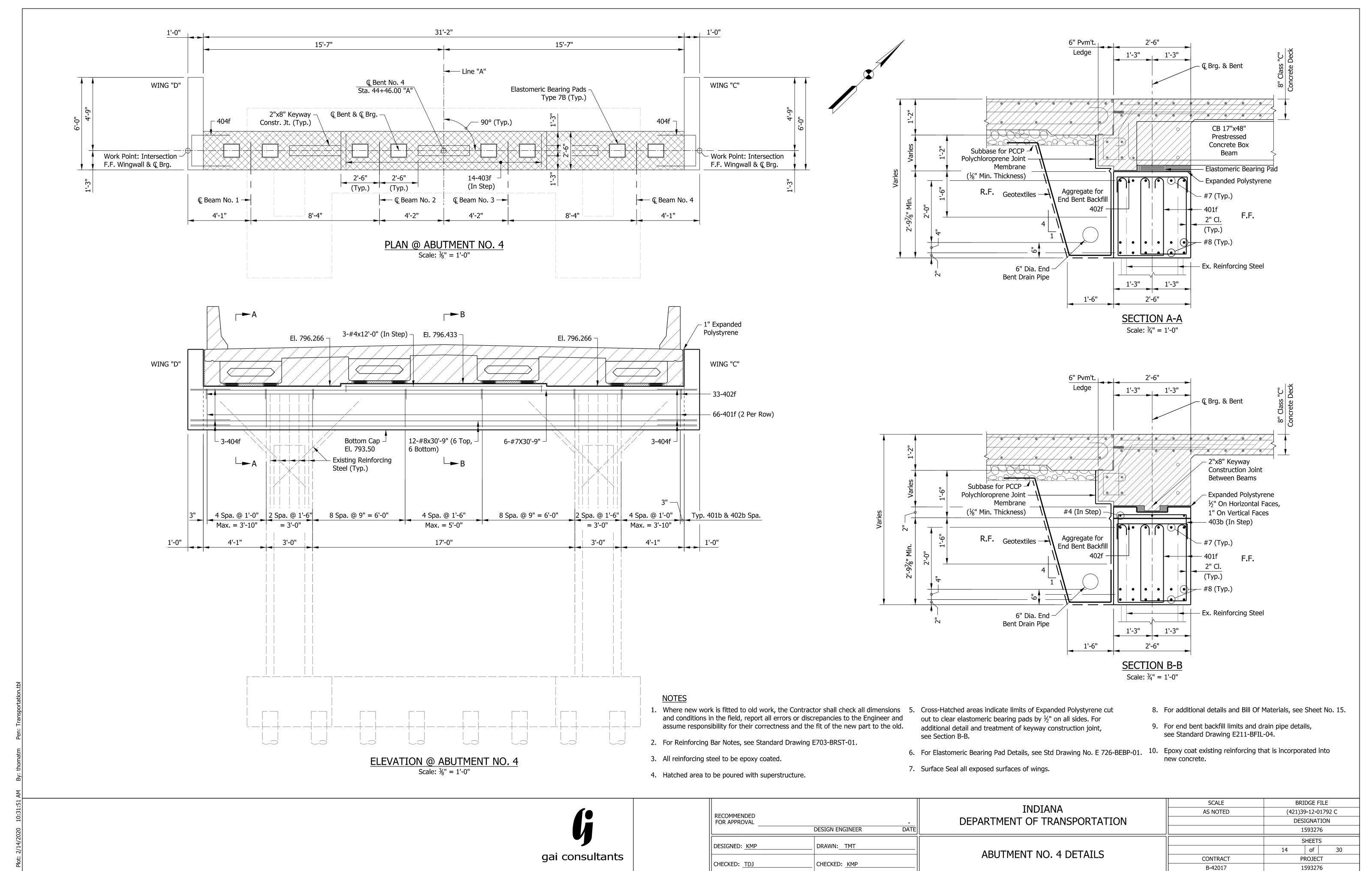
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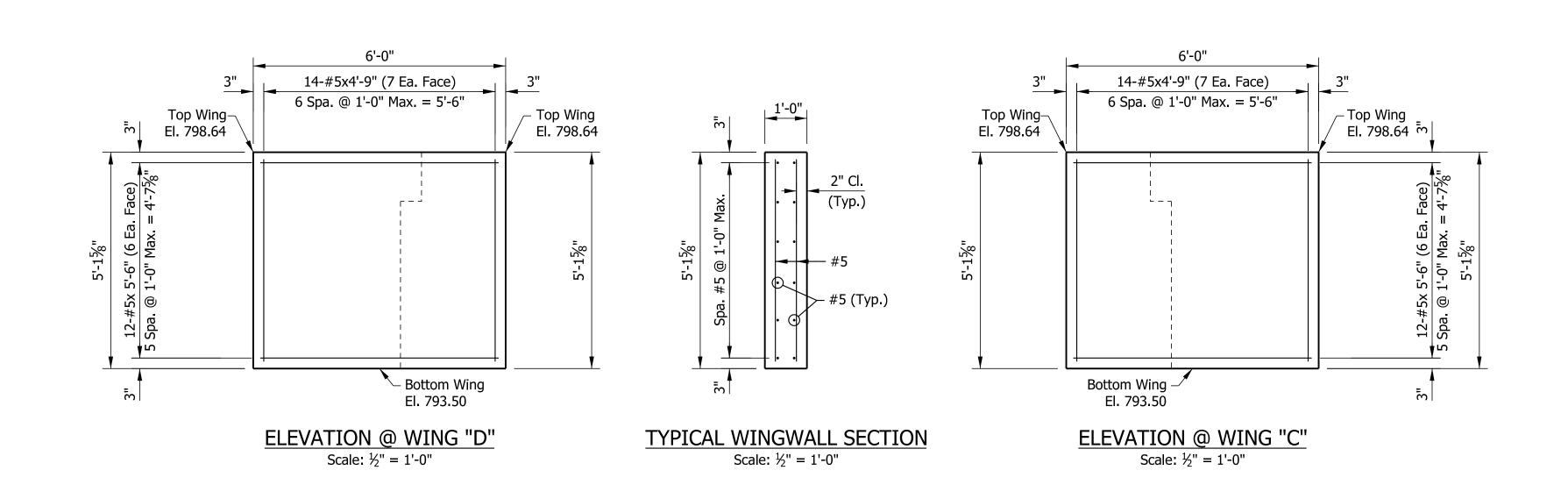
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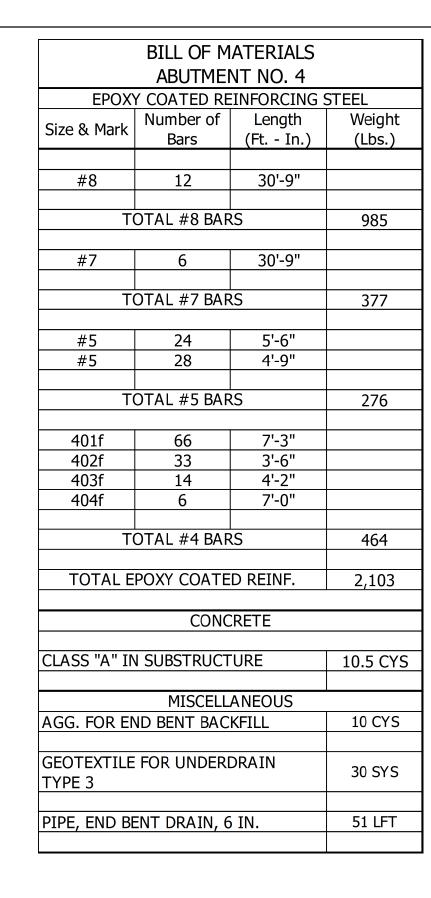
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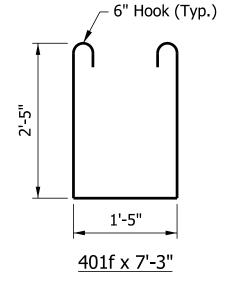


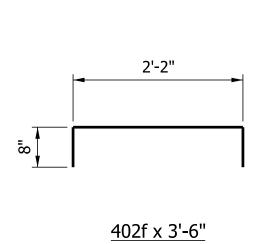


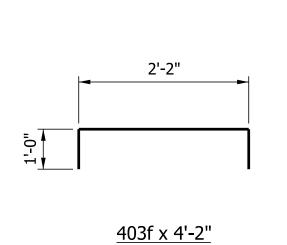


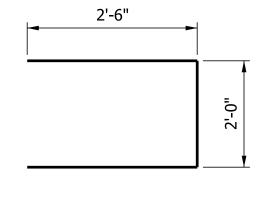












404f x 7'-0"

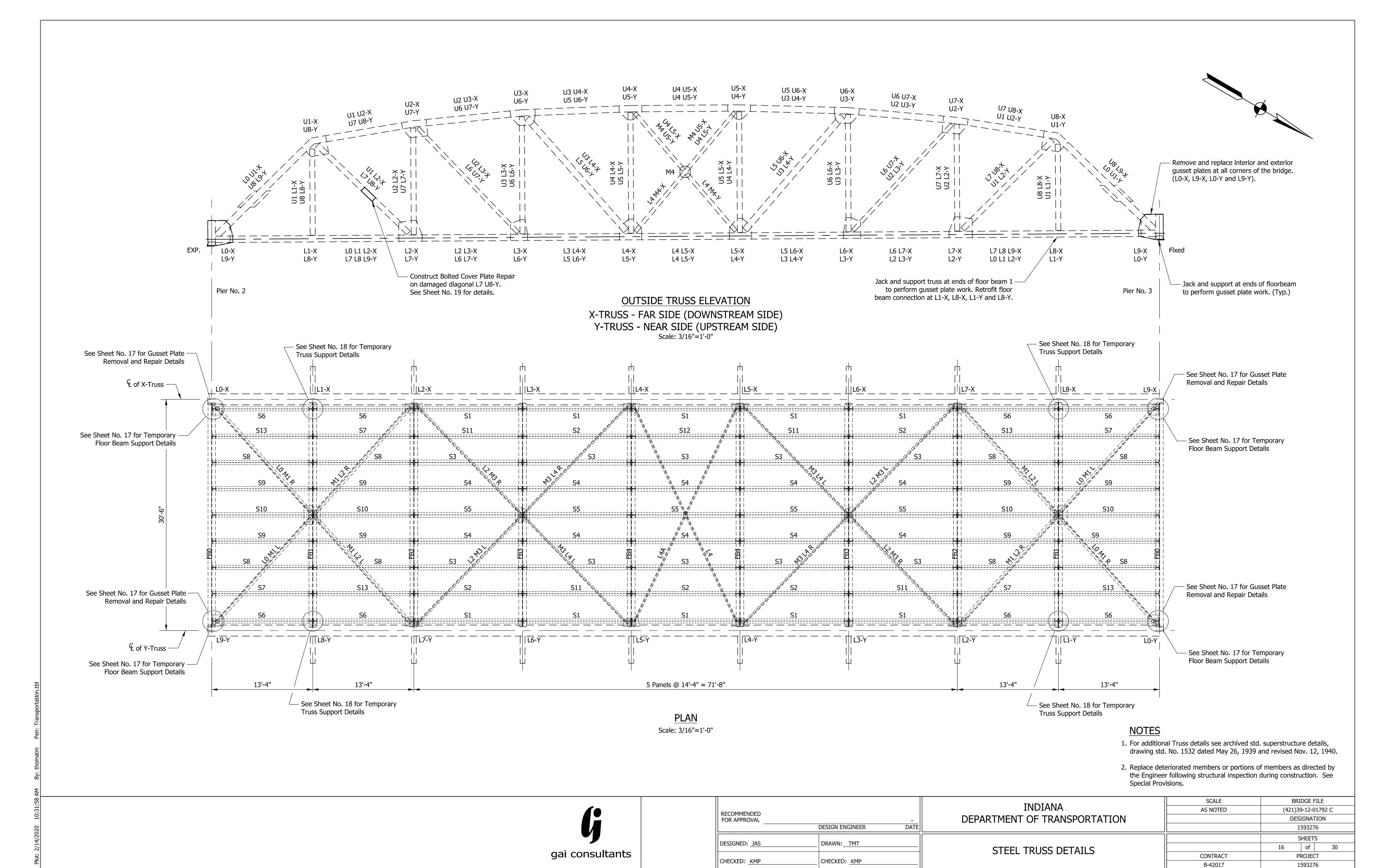
#### **NOTES**

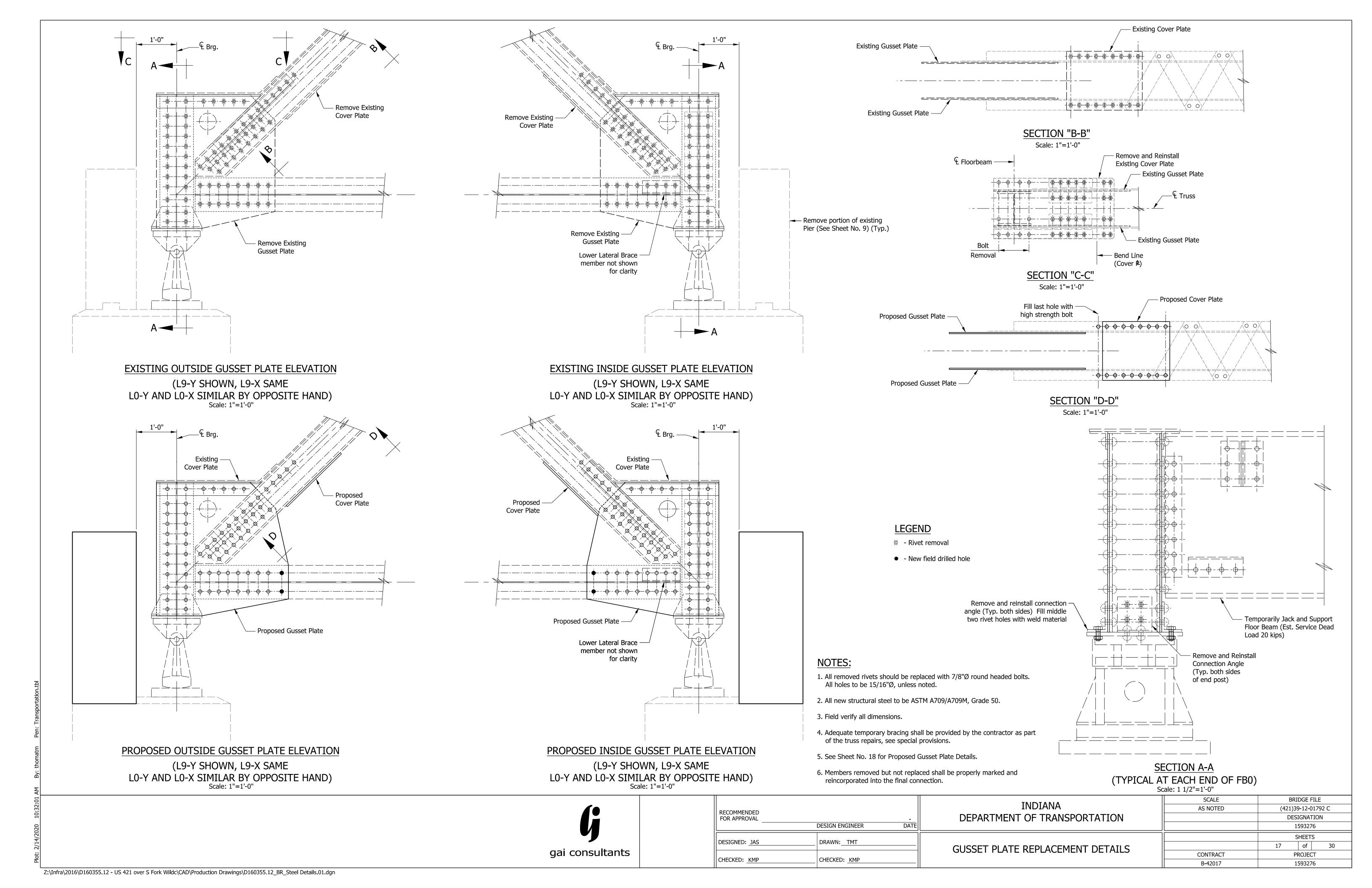
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- 2 For Reinforcing Bar Notes, see Std. Dwg. E 703-BRST-01.
- 3. All reinforcing bars shall be Epoxy Coated.
- 4. Epoxy coat existing reinforcing that is incorporated into new concrete.
- 5. Surface Seal all exposed surfaces of wings.

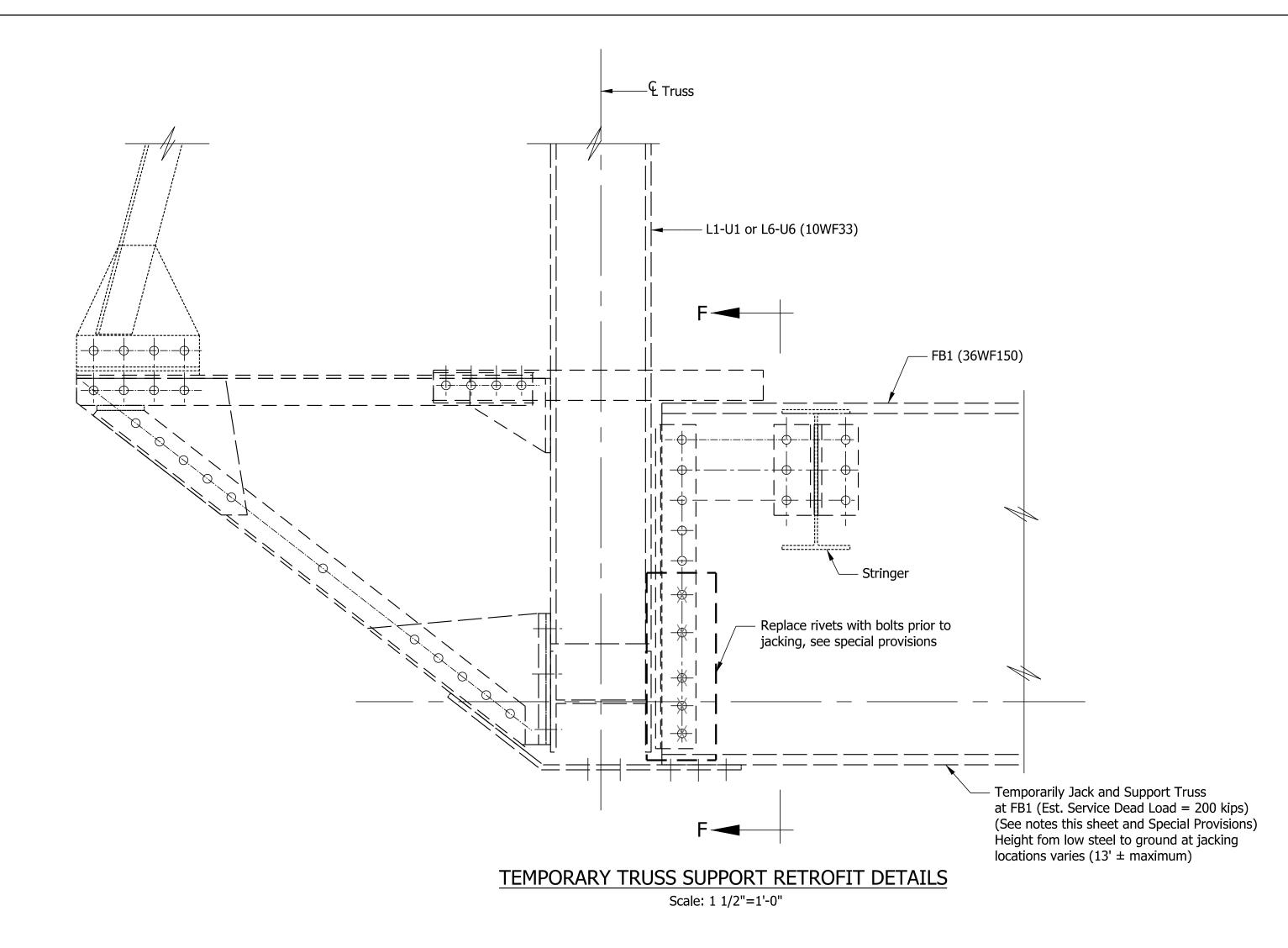
gai consultants

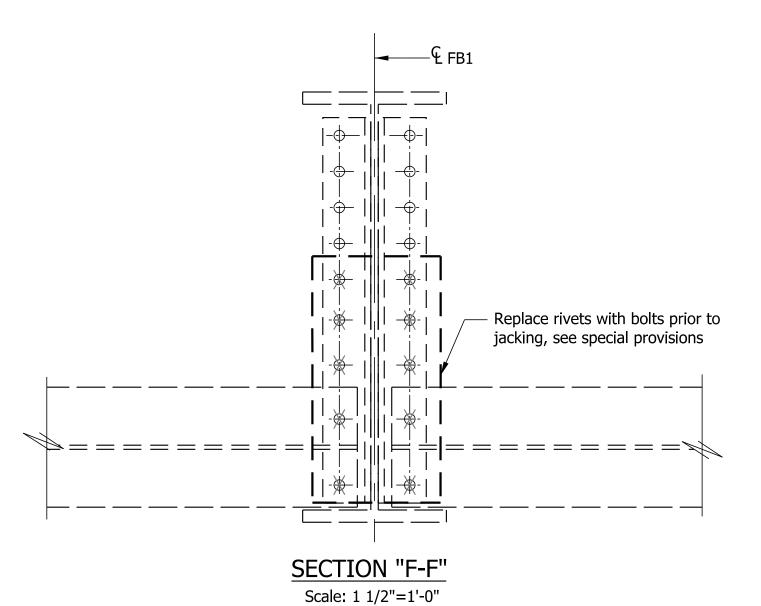
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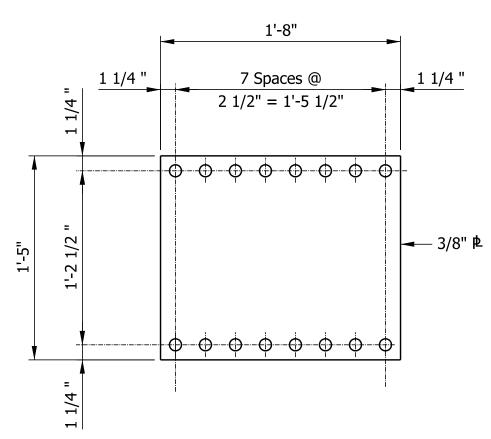








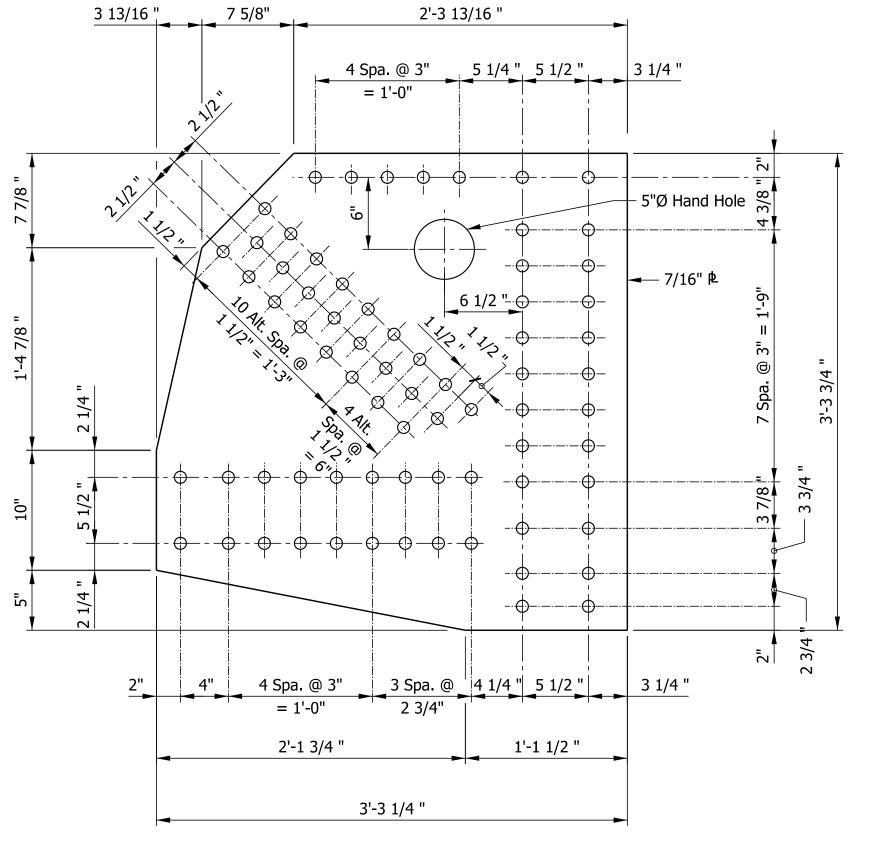
(Stringers Not Shown for Clarity)



PROPOSED COVER PLATE

Scale: 1 1/2"=1'-0"

**G** gai consultants



PROPOSED GUSSET PLATE DETAIL

Scale: 1"=1'-0"

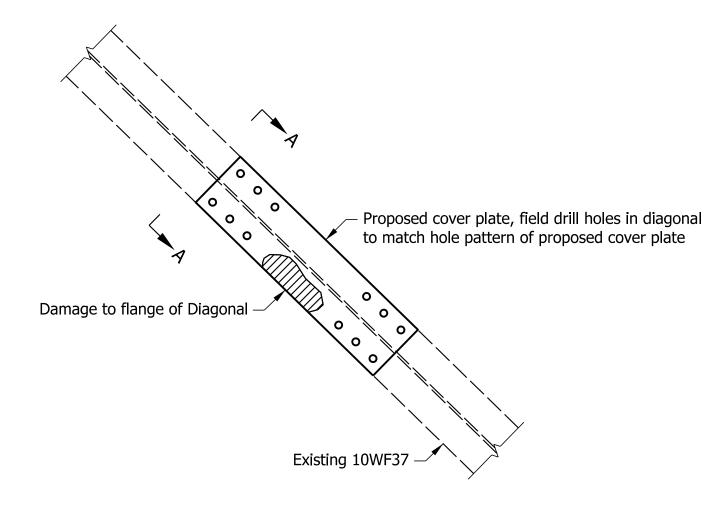
#### **LEGEND**

- New field drilled hole

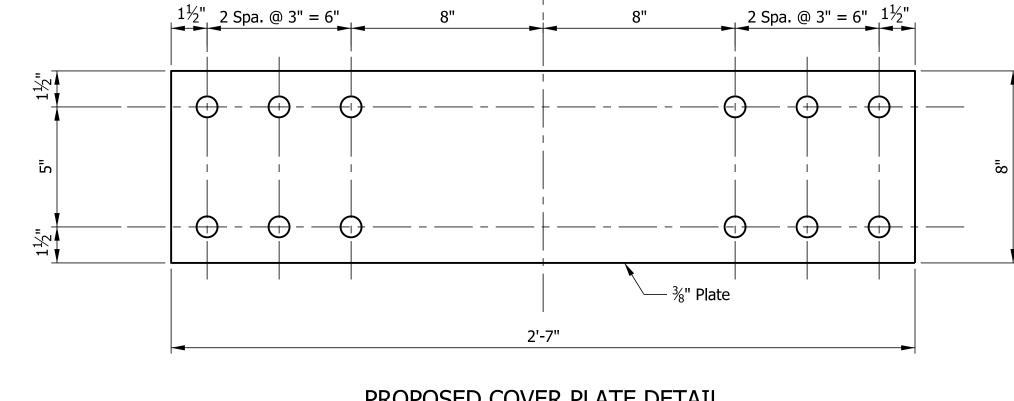
#### **NOTES**

- Estimated Dead Load is at each end of FB1 and assumes a sequence of work such that temporary support and gusset replacement occurs only on one side of the creek at a time, with the deck slab completely removed prior to jacking.
- 2. Alternate jacking and supporting locations and methods may be used if approved by the engineer. (See Special Provisions).
- 3. All removed rivets should be replaced with 7/8"Ø round headed bolts. All holes to be 15/16"Ø.
- 4. All new structural steel to be ASTM A709/A709M, Grade 50.
- 5. Field verify all dimensions.
- Adequate temporary bracing shall be provided by the contractor as part of the truss repairs, see special provisions.
- 7. Members removed but not replaced shall be properly marked and reincorporated into the final connection.

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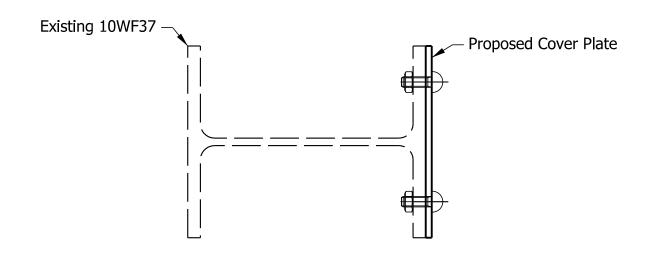
## BOLTED COVER PLATE REPAIR (L7 U8-Y) Scale: 1½" = 1'-0"



Approx. Center of damaged area

PROPOSED COVER PLATE DETAIL

Scale: 3" = 1'-0"



SECTION A-A
Scale: 3'' = 1'-0''

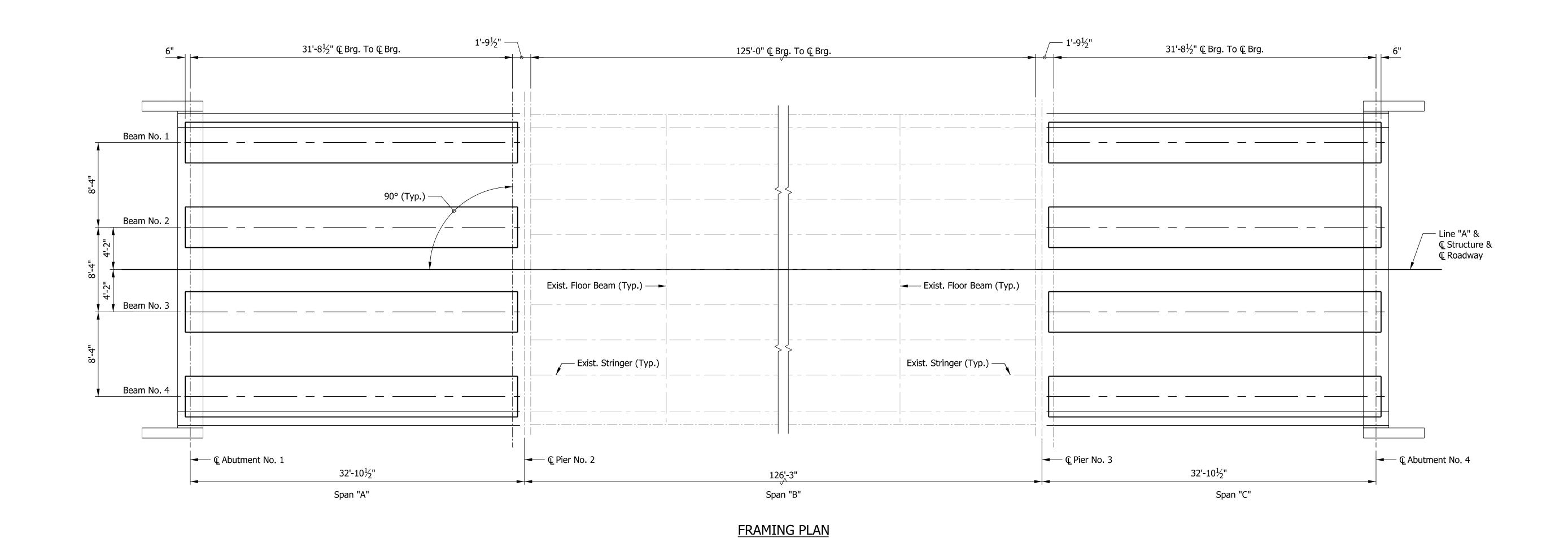
#### **NOTES**

- 1. All bolts shall be  $\frac{7}{8}$ "Ø round headed bolts. All holes to be  $\frac{15}{16}$ "Ø, unless noted.
- 2. All new structural steel to be ASTM A709/A709M, Grade 50.
- 3. Field verify all dimensions.
- Repair or replacement of additional members only required if determined by engineer following a structural inspection during construction. See unique special provisions.

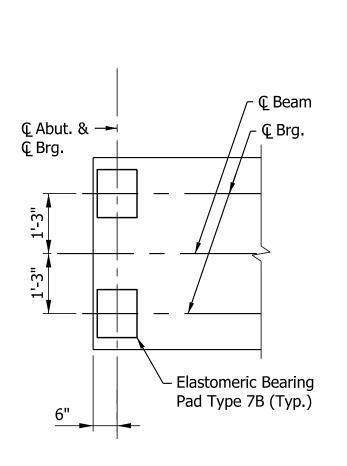


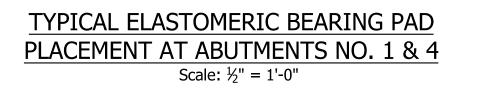
RECOMMENDED FOR APPROVAL	DESIGN ENGINEER DATE	INDIANA DEPARTMENT OF TRANSPORTATION	SCALE AS NOTED	BRIDGE FILE (421)39-12-01792 C DESIGNATION 1593276
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CHECKED: KMP	DESIGN ENGINEER DATE  DRAWN:TMT  CHECKED: _KMP	COVER PLATE DETAILS	CONTRACT B-42017	PROJECT 1593276

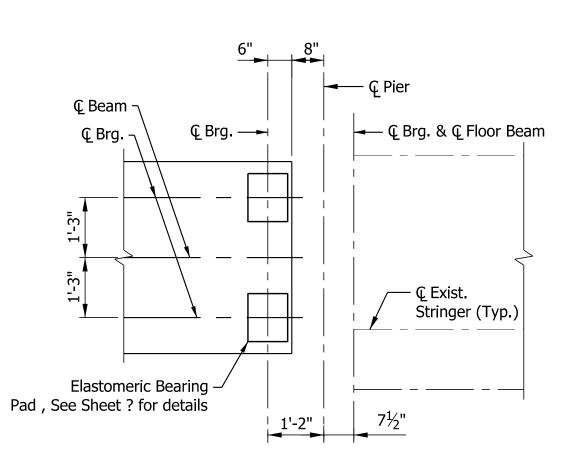
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Scale:  $\frac{3}{16}$ " = 1'-0"







TYPICAL ELASTOMERIC BEARING PAD PLACEMENT AT PIERS NO. 2 & 3 Scale:  $\frac{1}{2}$ " = 1'-0"

### Bottom of Slab — Fillet Varies — Top of Beam +**Beam Elevation**



#### Residual Camber Table Span "A" & "C" Initial Beam Camber -1/4" Superimposed Dead Load Deflection <del>1</del>/5" Residual Camber

Initial Beam Camber Equals Upward Deflection From Prestressing Force Minus Downward Deflection From Dead Load of the Beam In Inches.

# <u>Section</u>

#### BEAM DESIGN DATA

1. Prestressing steel shall be 0.5" diameter uncoated, special low relaxation, seven-wire strand, 270 ksi, with strand area = 0.167 sq.in.

3/4" Min. (Spans A & C)

- 2. Initial pull per prestressing strand to be 33.82 kips.
- 3. Concrete strength at release, f'ci = 5,000 psi.
- 4. Concrete strength at 28 days, f'c = 6,000 psi.
- 5. Mild reinforcing steel shall be Grade 60 ksi minimum yield strength.

#### **GENERAL BEAM NOTES**

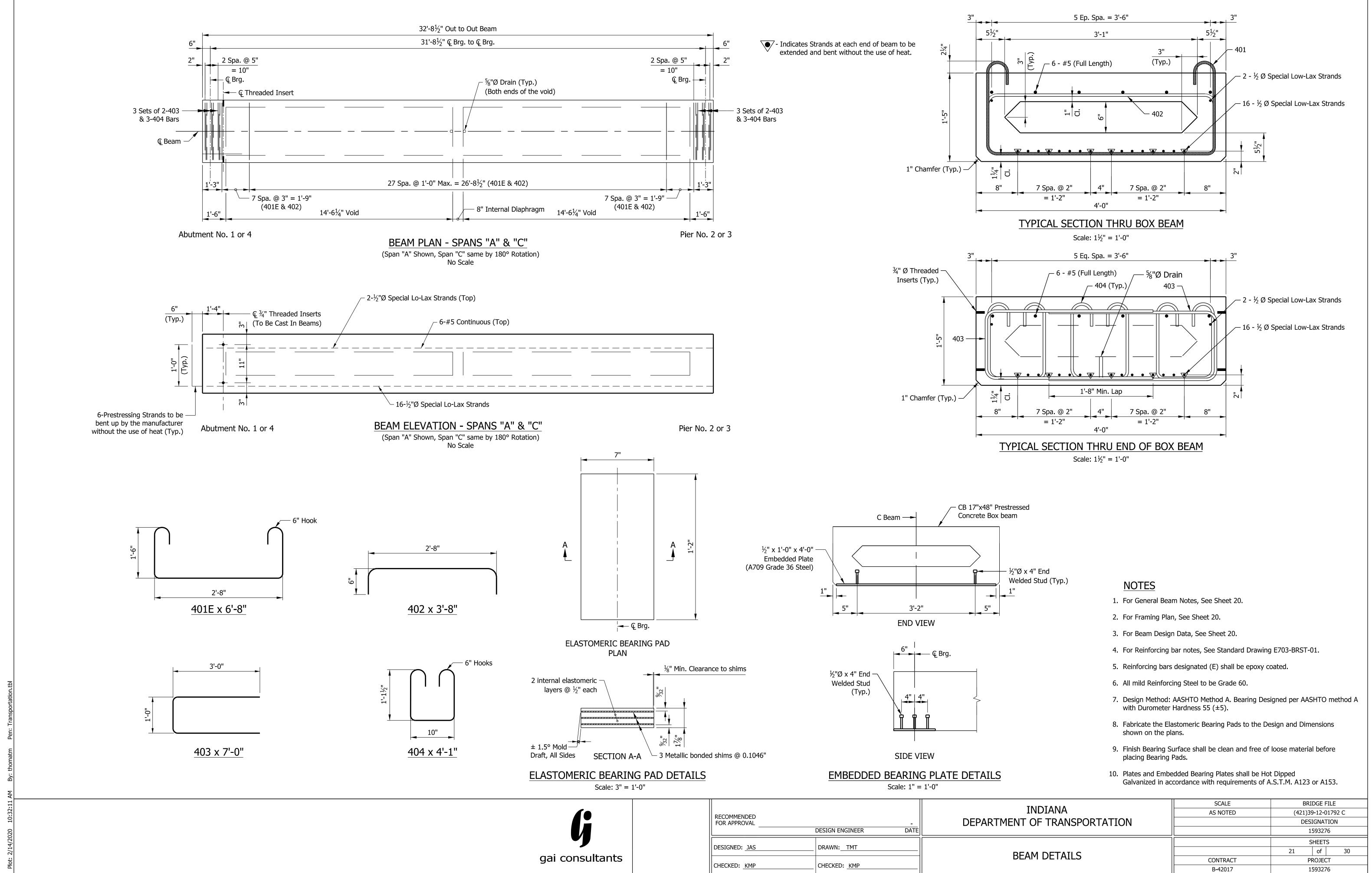
Drawings E 707-BPBF-01 and -04.

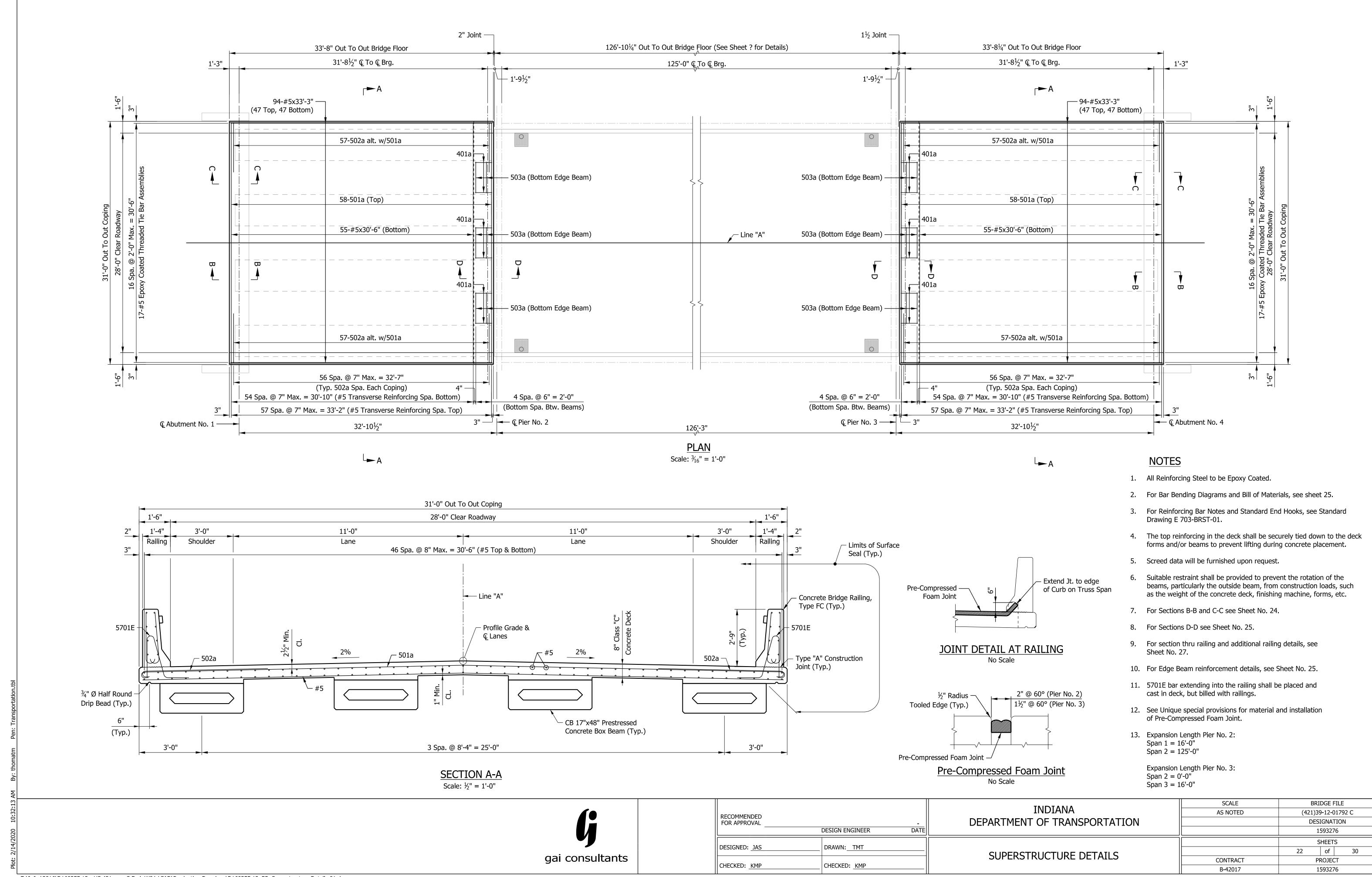
- 1. Beams shall be cast a minimum 30 days prior to pouring the deck.
- 2. Beams are to be lifted and supported at the bearing points during handling, storage, and transportation. Adequate bracing must be provided at all times during storage, transportation and lifting to resist lateral loads.
- 3. Allowance should be made in beam length for elastic shortening and grade.
- 4. For Fabrication Tolerances of Prestressed Beams, see Standard
- 5. Beams shall be maintained vertically at all times. Suitable restraint shall be provided to prevent the rotation of the beams, particularly the outside beam, from construction load, such as the weight of the concrete deck, finishing machine, forms, etc.
- 6. Top of beams shall be scored transversely at about 3" on center with pointed tool. Maximum depth of scoring should be  $\frac{1}{4}$ ".
- 7. The ends of the beams at the End Bents shall be cast so that the end of the beam is vertical when placed in final grade condition.
- 8. Acute angles of box beams shall be chamfered 3" (by Manufacturer).
- 9. Sealer on the outside face of exterior beams to be done by the fabricator in the shop. Do not rub.

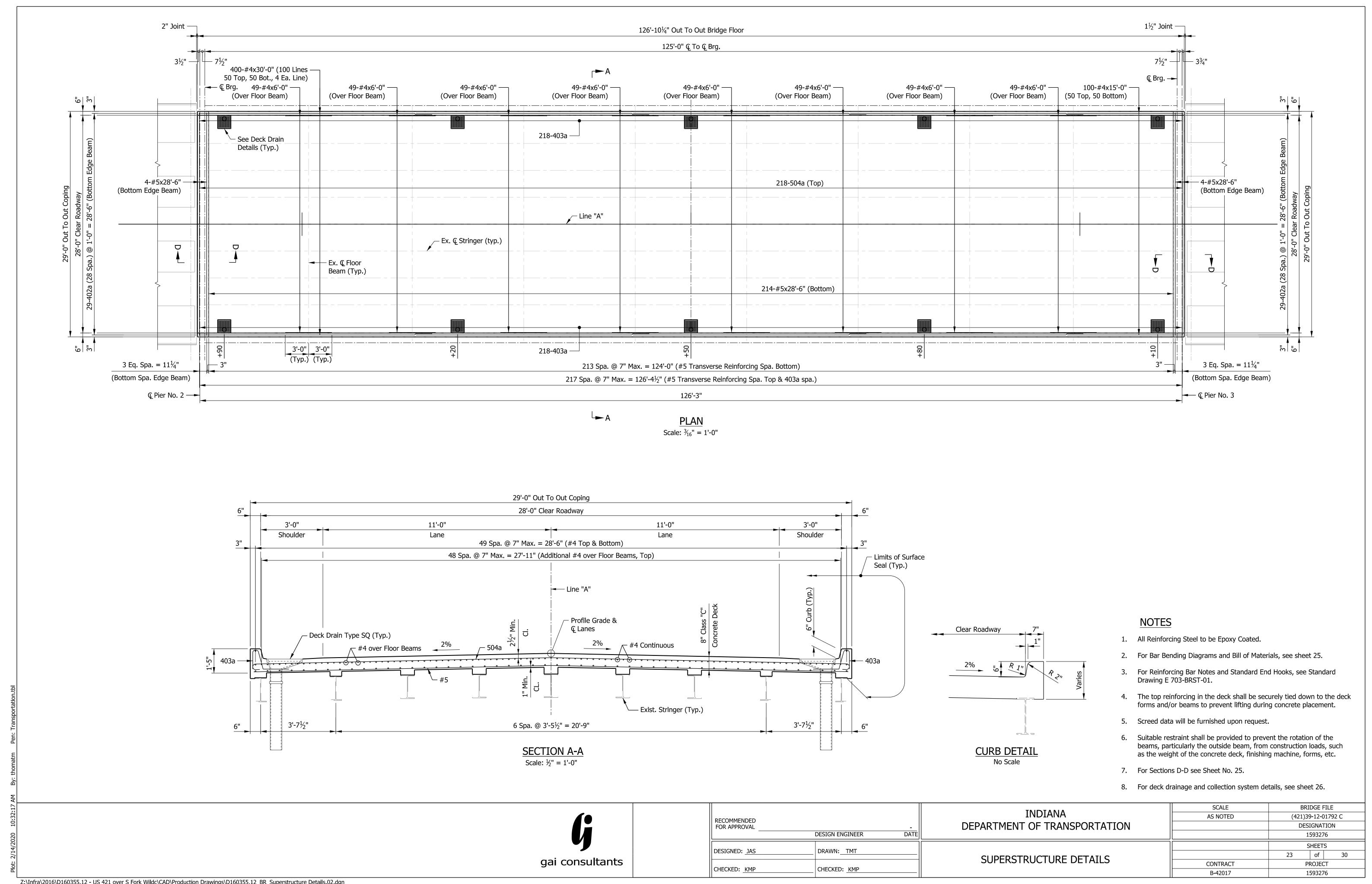


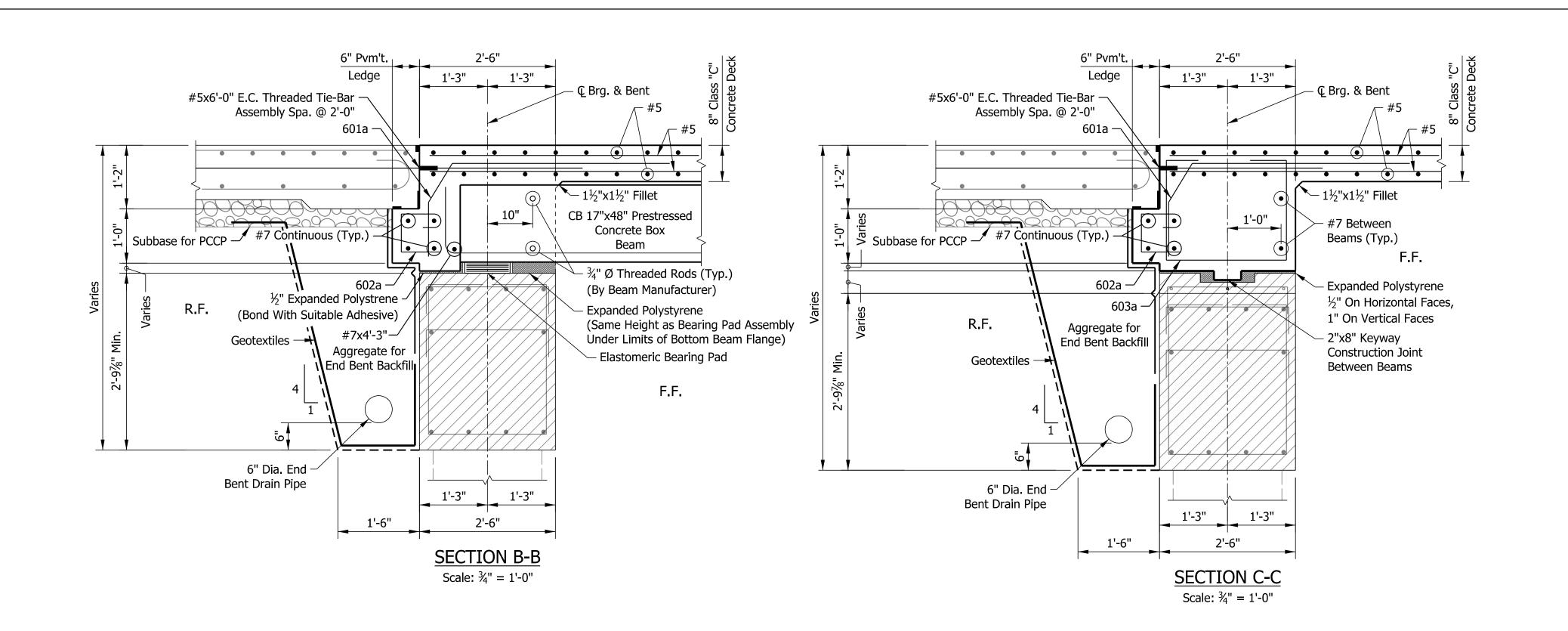
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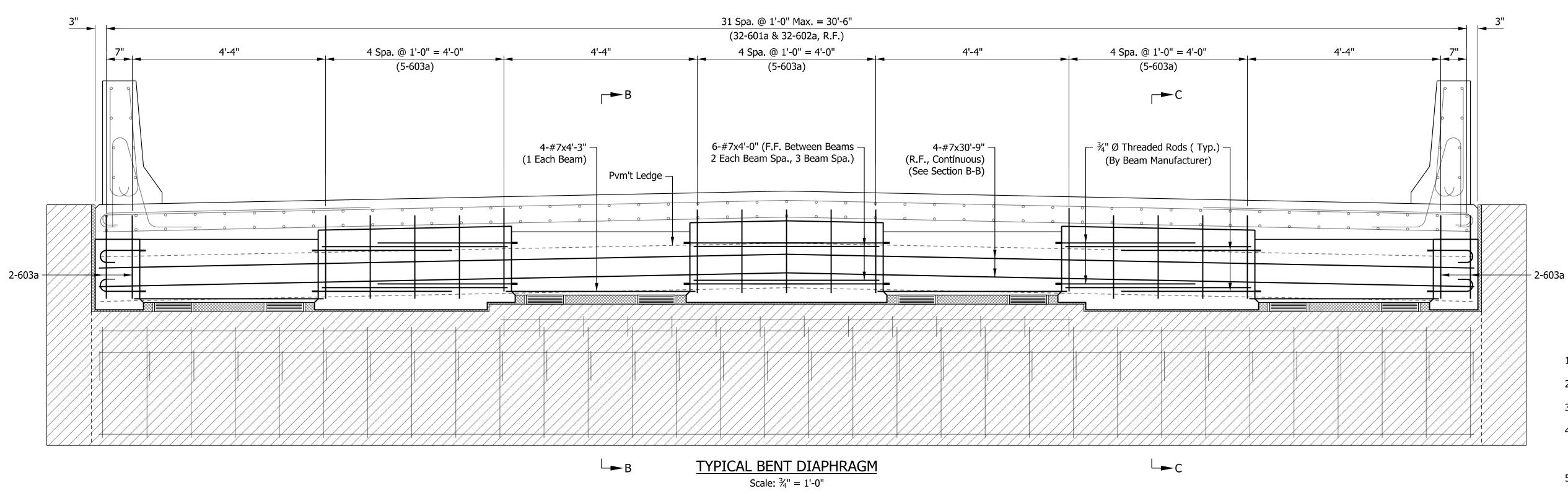
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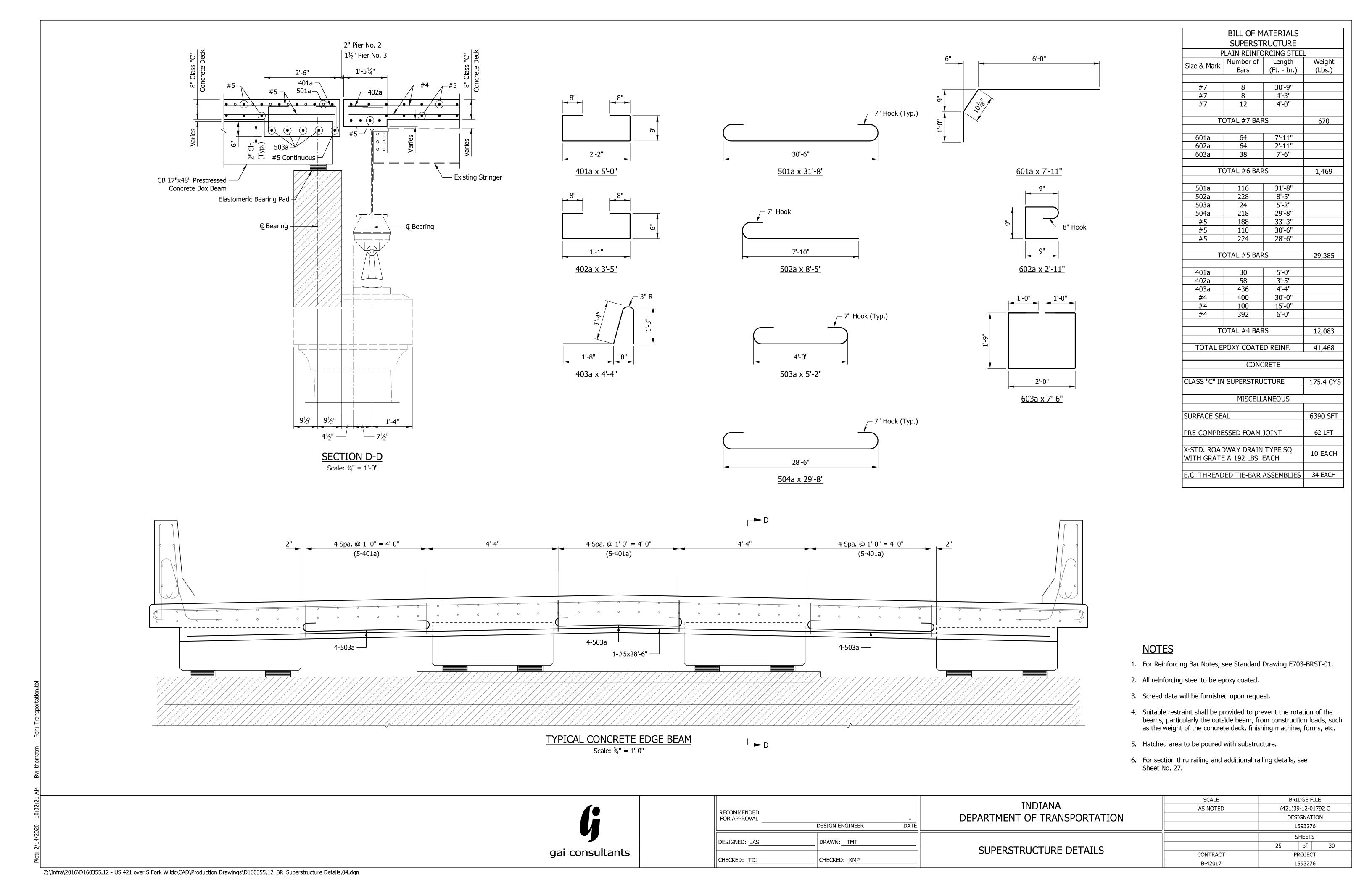


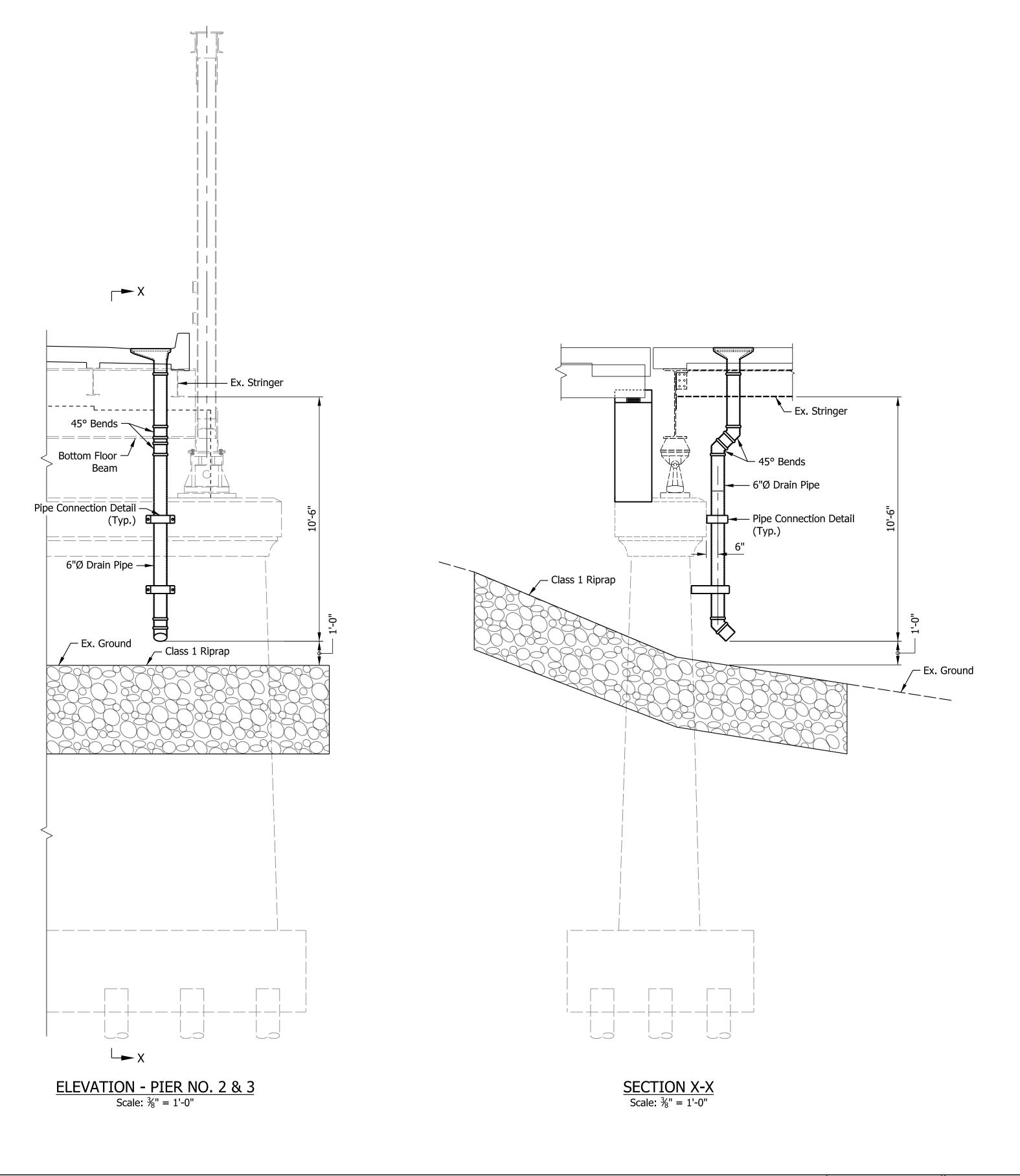
#### <u>NOTES</u>

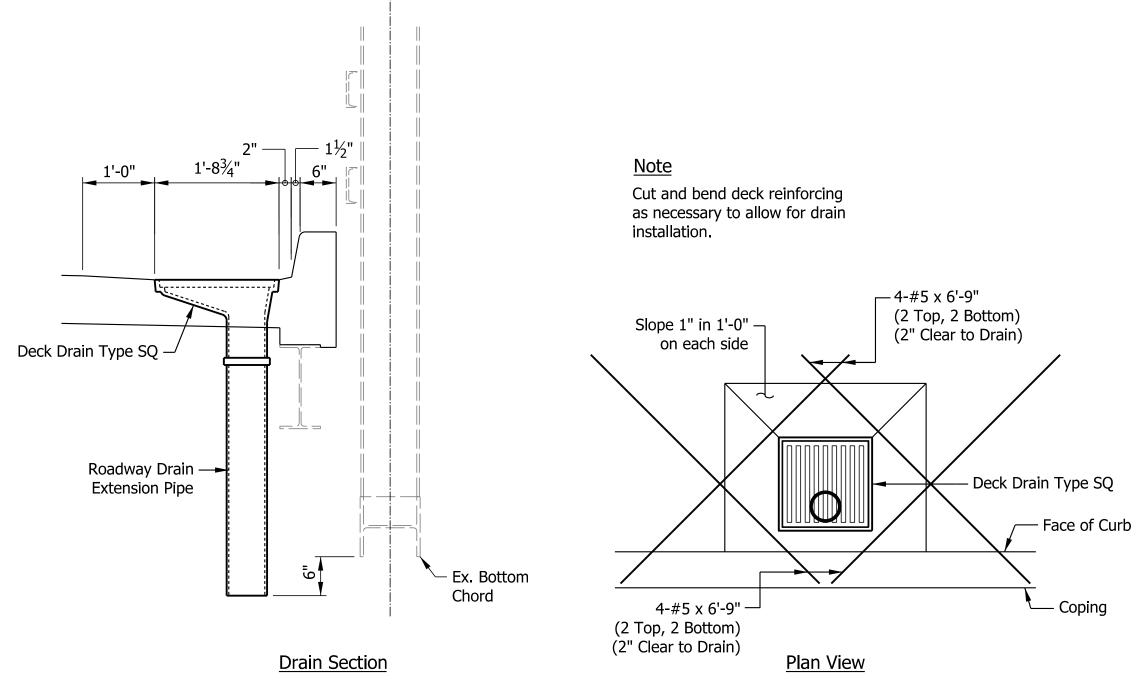
- 1. For Reinforcing Bar Notes, see Standard Drawing E703-BRST-01.
- 2. All reinforcing steel to be epoxy coated.
- 3. Screed data will be furnished upon request.
- 4. Suitable restraint shall be provided to prevent the rotation of the beams, particularly the outside beam, from construction loads, such as the weight of the concrete deck, finishing machine, forms, etc.
- 5. Hatched area to be poured with substructure.
- 6. For additional details and Bill Of Materials, see Sheet No. 25.
- 7. For section thru railing and additional railing details, see Sheet No. 27.



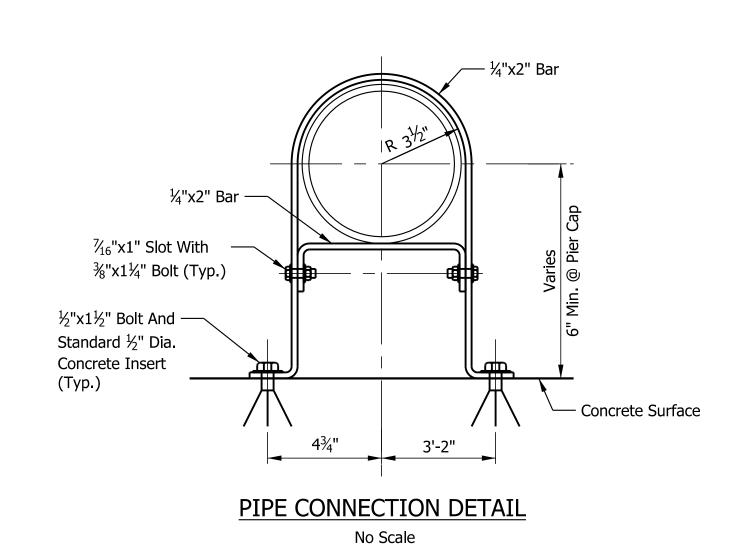
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		TAIDTANIA	SCALE	BRIDGE FILE
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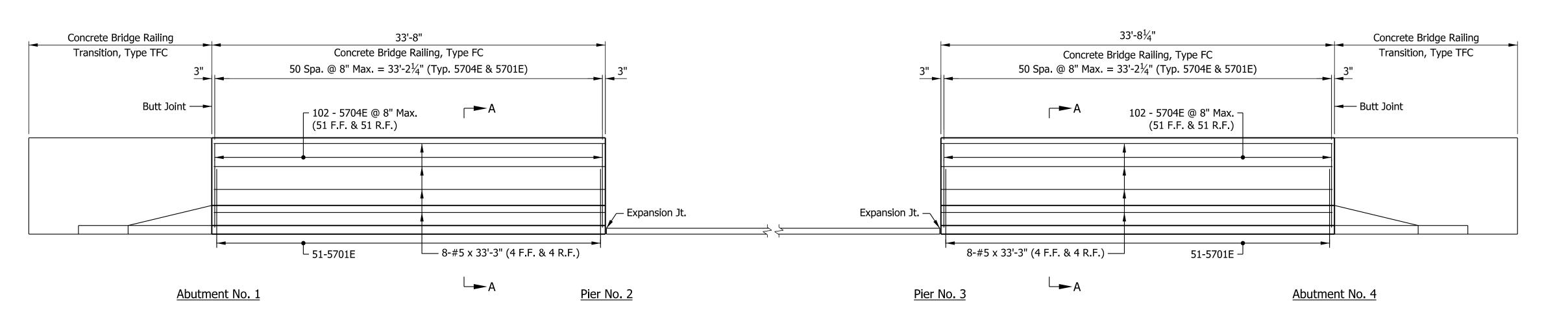


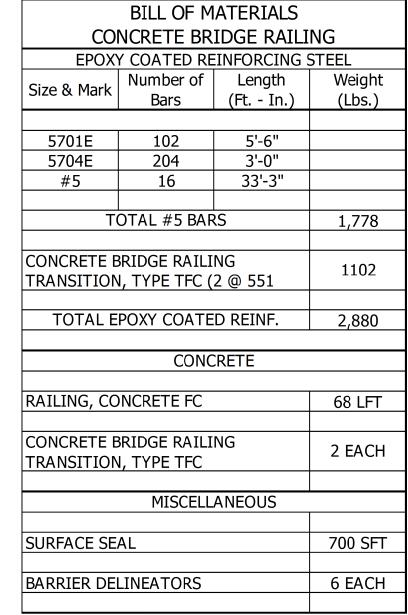
## DECK DRAIN DETAILS Scale: ¾" = 1'-0"





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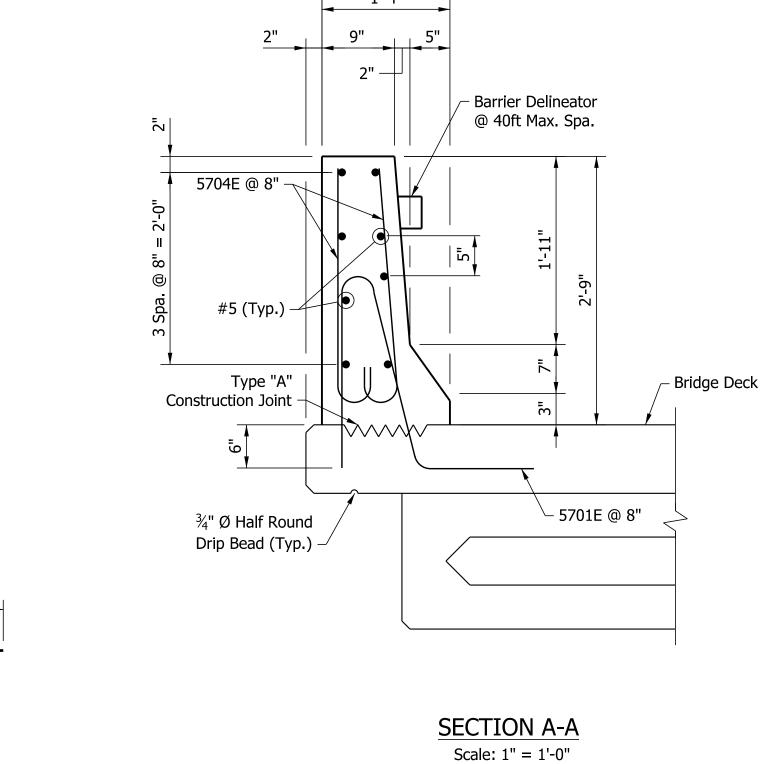


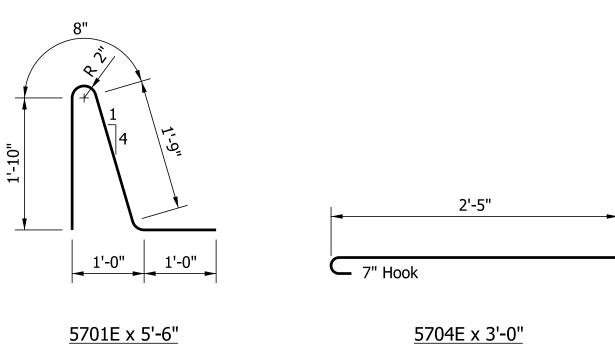


2 REQUIRED

#### CONCRETE RAILING ELEVATION

South Coping Shown, North Coping Same No Scale



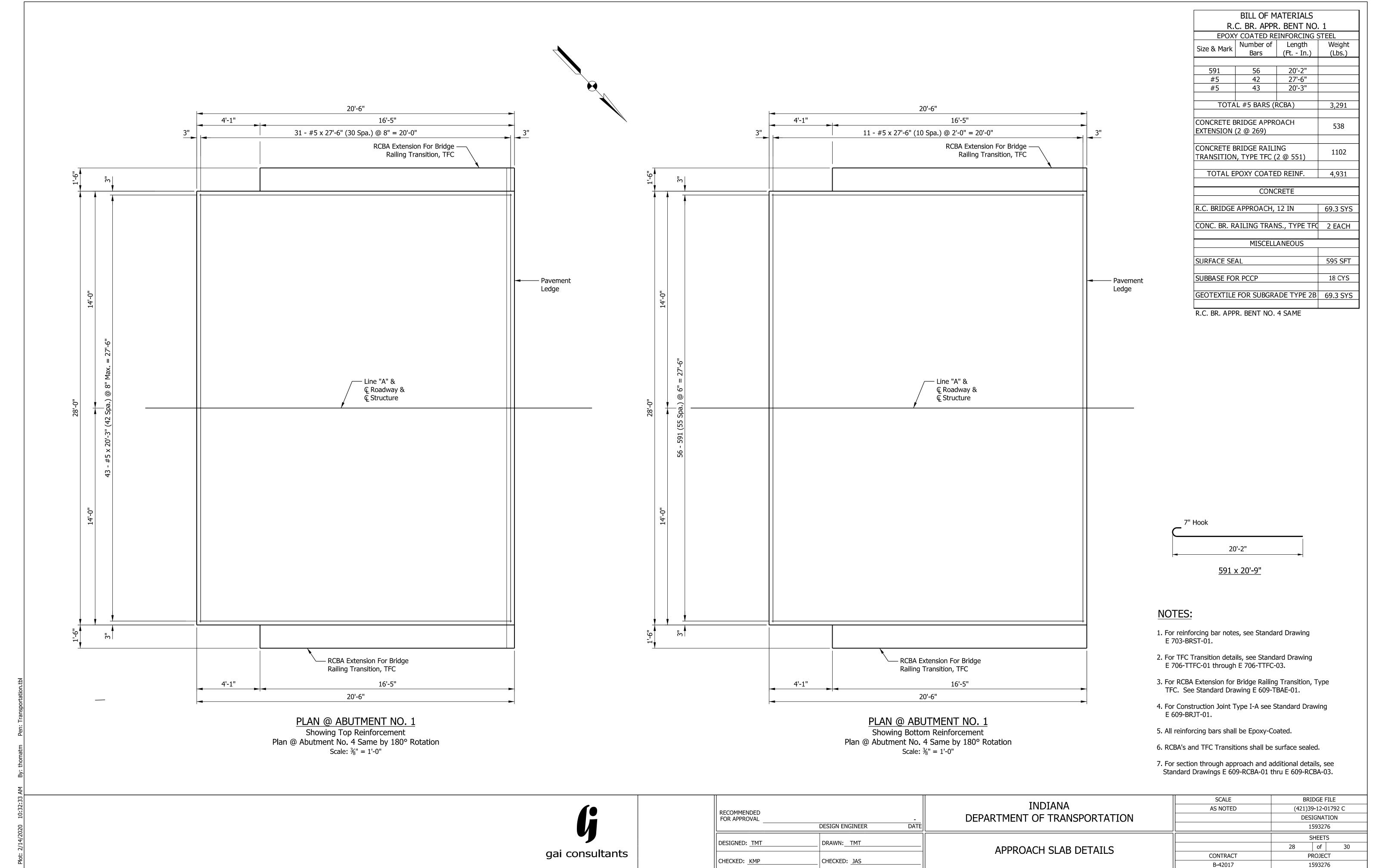


#### <u>NOTES</u>

- For Bridge Railing, Type FC details, see Std. Dwg. E 706-BRSF-02.
- 2. For TFC Transition details, see Std. Dwg. E 706-TTFC-01 through E 706-TTFC-03.
- 3. All reinforcing bars shall be Epoxy Coated.



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FOR APPROVAL		DEPARTMENT OF TRANSPORTATION		DESIGNATION
	DESIGN ENGINEER DATE			1593276
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											SUMI	MARY O	F BRIDGE	QUANTI	ITIES											
ITEM	CLASS C	CONCRETE CLASS A SUBSTR.		BARRIER, DELINEATOR	RAILING, CONCRETE, FC	CONCRETE BRIDGE RAILING TRANSITION	REINF. BARS,	REINFORCEM REINF. BARS	THREADED TIE BAR ASSEMBLY,	REINF. CONCRETE BRIDGE APPROACH	SUBBASE FOR PCCP	GEOTEXTILE FOR SUBGRADE TYPE 2B	EXCAVATION, FOUNDATION, UNCLASSIFIED	PRECOMPRESSED FOAM JOIN	PATCHING CONCRETE STRUCTURES	EMBEDDED GALCANIC ANODE	FIELD DRILLED HOLES IN	DRILL HOLES	DENT	GEOTEXTILE FOR UNDERDRAIN	PIPE, END BENT DRAIN, 6 IN	X-STD. ROADWAY DRAIN TYPE SQ WITH	STRUCTURAL STEEL *	STRUCTURAL STEEL**	CONCRETE BOX BEAM 17 IN. X 48 IN.	SURFACE
TFC TRANSITION, LFOXT BARS EF	EPOXY COATED EACH	12 IN. SYS	CYS	SYS	CYS SFT SFT			EACH	CONCRETE BACKFILL  EACH EACH CYS		SYS	LBS. EAC			CH		SFT									
SUPERSTRUCTURE	175.4						41468		34					62				28				10	1719	1000	262	6390
BENT No. 1			10.7				2103						10						10		51					
PIER No. 2		7.9					739								100	39	31			30						255
PIER No. 3		7.8					739								100	39	31			30						255
BENT No. 4			10.5				2103						10						10		51					
APPROACHES							9862			138.6	36	138.6														1190
CONCRETE BRIDGE RAILING				8	135	4	5760																			1400
TOTALS	175.4	15.7	21.2	8	135	4	62774	0	34	138.6	36	138.6	20	62	200	78	62	28	20	60	102	10	1719	1000 * Estimated qua	262	9490

	* Estimated quantity, to be paid for as "LSUM"	
* Estimated quantity,	work to be constructed at the direction of the Engineer, see Special Provisions	

	STRUCTURAL STEEL BILL OF MATERIALS												
Member	Member Piece	Locations  Total Width Thickne Height Weight  LO-X LO-Y L9-X L9-Y L7U8-Y No. (in) ss (in) (in) (lb)									_	Total Weight (lb)	
Gusset Plate	Gus PL 39 <sup>1</sup> / <sub>4</sub> " x <sup>7</sup> / <sub>16</sub> " x 3' 3 <sup>3</sup> / <sub>4</sub> "	2	2	2	2	1,00	8	39.25	0.4375	39.75	193.6	1548	
Cover Plate	PL 17" x <sup>3</sup> /8" x 1' 8"	1	1	1	1		4	17	0.375	20	36.2	145	
Cover Plate	PL 8" x <sup>3</sup> / <sub>8</sub> " x 2' 7"					1	1	8	0.375	31	26.4	26	
	Total											1719	

RIVE	ET REMOVAL SUMM	ARY
Location	Member Piece	No.
L0-X (SW)	Outside Gusset Plate	75
	Inside Gusset Plate	75
	L0 U1-X Cover Plate	18
	Top Cover Plate	28
	End Post	4
	Subtotal	200
L0-Y (NE)	Outside Gusset Plate	75
,	Inside Gusset Plate	75
	L0 U1-Y Cover Plate	18
	Top Cover Plate	28
	End Post	4
	Subtotal	200
L9-X (NW)	Outside Gusset Plate	75
	Inside Gusset Plate	75
	U8 L9-X Cover Plate	18
	Top Cover Plate	28
	End Post	4
	Subtotal	200
L9-Y (SE)	Outside Gusset Plate	75
. ,	Inside Gusset Plate	75
	U8 L9-Y Cover Plate	18
	Top Cover Plate	28
	End Post	4
	Subtotal	200
	Total	800

BOL	T REMOVAL SUMM	ARY
Location	Member Piece	No
L0-X (SW)	Top Cover Plate	10
LO-Y (NE)	Top Cover Plate	10
L9-X (NW)	Top Cover Plate	10
L9-Y (SE)	Top Cover Plate	10
` '	Total	40

DRILL HOLE SUMMARY										
Location	Member Piece	No.								
L0-X (SW)	L0 L1 L2-X	4								
L0-Y (NE)	L0 L1 L2-Y	4								
L9-X (NW)	L7 L8 L9-X	4								
L9-Y (SE)	L7 L8 L9-Y	4								
L7 U8-Y	L7 U8-Y	12								
	Total	28								



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FOR APPROVAL		DEPARTMENT OF TRANSPORTATION		DESIGNATION		
	DESIGN ENGINEER DATE			1593276		
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	PAVEMENT QUANTITIES AND APPROACH TABLE																		
LOCATION	DESCRIPTION (APPROACH TYPE OR CLASS)	CURB, ASPHALT, REMOVE	CURB HMA	PAVEMENT REMOVAL	EXCAVATIC COMMON		SURFACE 9 SURFACE 9 SURFACE 9 INTERM 19 DAGE 25		BASE 25.0 mm			COMPACTED AGGREGATE, NO.53  DEPTH 6" 3"		MILLING, TRANSITION	SUBGRADE TREATMENT, TYPE IC	LIQUID ASPHALT SEALANT	JOINT ADHESIVE, SURFACE	JOINT ADHESIVE, INTERMEDIATE	REMARKS
		LFT	LFT	SYS	CYS	CYS	TONS	TONS	TONS	SYS	SYS	TONS	TONS	SYS	SYS	LFT	LFT	LFT	
NORTH APPROACH				50	85		36				373.5	43		373.5		360	360		
SOUTH APPROACH				50	85		31				373.5	43		373.5		360	360		
TOTALS		0	0	100	170		67	0	0		747	86	0	747	0	720	720	0	

						POST												
PLAN	SIGN EXISTING	SIGN PROPOSED	CICN	CICN CIZE		OUND - MOUNTED  IGN AREA (ft²)		MOUNTED ON SIGN CATENARY, AREA		l	U CHANNEL		S 2" X 2" - 12 GA. (TYPE 2)			QUARE 2 1/4" X 2 1/4" - 12 GA. (TYPE 1		
SHEET NO.	LOCATION	LOCATION	SIGN CODE	SIGN SIZE (IN x IN)	516	IN AKEA (	π)	(ft <sup>2</sup> )	POST L	ENGTH	POST I	ENGTH	REINFORCED ANCHOR			REINFORCED ANCHOR		
/ LINE	(STA.)	(STA.)	CODE	(11/1 \ 11/1)	0.080"	0.100"	0.125"	0.080"	1	2	TYPE "A"	TYPE "B"	POS	T LENGTH	(FT.)	POS	T LENGTH (FT.)	
					0.080	0.100	0.125	0.060	FT.	FT.	FT.	FT.	1	2	TOTAL	1	TOTAL	
Line "A"																		
5	41+65	41+65	Wildcat Cre	eek												5.5	5.5	
	42+34	42+15	Mile Post													5.5	5.5	
	42+45	42+24	End Marke	r												5.5	5.5	
	42+45	42+24	End Marke	r												5.5	5.5	
	12 1 13	12.12.1	Eria Fiarice													3.5	313	
	44+55	44+80	End Marke	r												5.5	5.5	
	44+55	44+80	End Marke	r												5.5	5.5	
					0.00	0.00	0.00	0.00							0		33.0	

				F	RII	ΡĮ	R.A	۱P	, SE	EDING	S, AND	SOD	DING	SUMI	MAR	Y TAE	BLE						
	LOCATIC	N								AP TURNOUT		RIPRAP FOR SCOUR			SODDING			SEEDING					
FEFT HOITETS OT NOITETS WOULD BENT 1 BENT 4 BENT 4 LEFT LEFT LEFT LEFT LEFT LEFT LEFT LEFT						LEFT	RIGHI RIPRAP,	GEOTEXTILE FOR RIPRAP, TYPE 3	RIPRAP, REVETMENT RIPRAP, REVETMENT		GEOTEXTILE FOR RIPRAP, TYPE 3	FOR PAVED SIDE DITCHES FOR TURNOUTS		WATER	MOBILIZATION AND DEMOBILIZATION FOR SEEDING	MULCHED SEEDING U	MULCHED SEEDING R	MULCHING MATERIAL	FERTILIZER				
									TOI		TONS	TONS	SYS	SYS	SYS	KGAL	EACH	SYS	SYS	TON	TON		
			X						25						15	1							
		X							25						15								
									X 25						15								
		L				_		X	25	5 40					15								
				Χ							225		300										
		L			Х							106	85										
						X						106	85										
							Х				225		300										
TOTALS									10	0 160	450	212	770		60	1	1		1000				

	(-		ARDE	ΙΤΔ	SUMI	MARY	/ TAF	RIF							
GUARDRAIL SUMMARY TABLE															
LOCATION			W-BE	EAM .	TRANS	SITIONS	END TRE	ATMENTS	111						
APPROACH	LEFT	RIGHT	MGS GUARDRAIL, W-BEAM, 6 FT 3 IN. SPACING	GUARDRAIL, W-BEAM, 6 FT 3 IN. SPACING	GUARDRAIL, TRANSITION, MGS WITHOUT CURB	GUARDRAIL, TRANSITION, TYPE TGB	GUADRAIL END TREATMENT, OS	GUARDRAIL, TERMINAL SYSTEM, W-BEAM CURVED, 1	GUARDRAIL, END TREATMENT, REMOVE	GUADRAIL, REMOVE					
			LFT	LFT	EACH	EACH	EACH	EACH	EACH	LFT					
NORTH APPORACH		Χ		62.5		1		1		120					
NORTH APPORACH	X		87.5		1		1			140					
SOUTH APPROACH		Х	81.25		1		1			140					
SOUTH APPROACH	X		37.5		1		1			140					
TOTALS			206.25	62.5	3	1	3	1	0	540					

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			TAIDTANIA	SCALE	BF	RIDGE FILI	E	
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