

# ENGINEERING ASSESSMENT

## US 41 at Hillsdale Road Intersection Improvements

Vanderburgh County  
Des. No. 1400005

August 21, 2019

Prepared for:      INDOT Vincennes District  
                          3650 Old US Hwy 41  
                          Vincennes, IN 47591

Project Manager:  Mr. Matthew Bullock  
                          (812) 830-9683  
                          [MBullock1@indot.in.gov](mailto:MBullock1@indot.in.gov)

Prepared by:  
First Group Engineering, Inc.



5925 Lakeside Blvd.  
Indianapolis, Indiana 46278  
(317) 290-9549

## Table of Contents

Purpose of Report: .....	3
Project Location: .....	3
Purpose and Need: .....	3
Existing Facility: .....	3
Existing Drainage: .....	4
Existing Utilities: .....	4
Field Check: .....	5
Coordination:.....	5
Previous Studies: .....	5
Traffic Analysis:.....	6
Crash Data and Analysis:.....	6
Alternatives: .....	8
Recommended Alternative: .....	10
Maintenance of Traffic During Construction: .....	13
Cost Estimate:.....	14
Environmental Issues:.....	14
Railroad Agreement: .....	15
Survey Requirements:.....	15
Right of Way Impacts: .....	15
Utilities Impacts: .....	15
Drainage Structures: .....	16
Changes to Proposal:.....	16
Report Concurrence Block: .....	17
APPENDICES TABLE OF CONTENTS: .....	18

## **Purpose of Report:**

The purpose of this report is to document the engineering assessment phase of project development.

## **Project Location:**

The Hillsdale Road intersection is located at RP 11.07 along US 41 in Vanderburgh County, situated at the southeast corner of the Town of Darmstadt. The project intersection lies within the Evansville MPO urbanized boundary.

## **Purpose and Need:**

The existing intersection experiences a crash history that merits an evaluation of causes and applicable countermeasures. Traffic growth over a period of years has contributed to the safety concerns for vehicles entering US 41 from Hillsdale Road. This Engineering Assessment examines alternative intersection treatments that will improve the traffic operations and improve the overall safety performance of this intersection.

## **Existing Facility:**

US 41 is classified as a Principal Arterial - Other on the National Highway System (NHS). The area surrounding the intersection is rural/suburban. US 41 is a 4-lane divided highway with a 60 MPH posted speed limit. The pavement material is asphalt. There is a single left turn lane for each NB and SB US 41. The lane widths are 12 feet. There are 10-foot paved shoulders along the outside travel lanes and 4-foot paved shoulders along the median edges. The median width is 64 feet as measured between the inside thru lanes. The width of the paved median surface as measured longitudinally along the US 41 centerline is 38 feet. The terrain is generally level.

Hillsdale Road is an east-west, two-lane local roadway with a posted speed limit of 45 MPH. The Hillsdale Road approaches have no auxiliary lanes. The existing roadway width is approximately 20 feet. Hillsdale Road connects two north-south local collector roadways; Browning Road (approximately one mile east) and Old State Road (approximately 0.3 mile west).

A single set of CSX railroad track is approximately parallel to US 41, situated approximately 135 feet west of the US 41 centerline. There is 2-lane bridge carrying Hilldale Road over Little Pigeon Creek whose west abutment is approximately 160 feet east of the US 41 centerline.

The traffic volumes along US 41 are significantly higher than those along Hillsdale Road (approximately 90% vs. 10%.) Commercial truck volume is approximately 13% along

US 41.

### **Existing Land Use:**

The land uses adjoining the US 41 / Hillsdale Road intersection are residential, commercial and agricultural.

### **Existing Drainage:**

Little Pigeon Creek crosses under US 41 approximately 1,045 feet north of Hillsdale Road (60" x 48" RC box culvert) and crosses under Hillsdale Road approximately 200 feet east of the US 41 centerline (bridge). A tributary of Little Pigeon Creek crosses under US 41 approximately 1,590 feet south of Hillsdale Road.

The existing roadside drainage along US 41 is conveyed in ditches and pipes into Little Pigeon Creek or its tributaries.

### **Existing Utilities:**

Several underground utilities and overhead electric distribution and located within the project area. An electric substation is located at the southwest quadrant of the intersection, approximately 225 feet west of the US 41 centerline.

The following utilities cross US 41 relative to the Hillsdale Road centerline:

- 4 each fiber optic cable (approximately 20 feet south)
- Water pipe (approximately 55 feet south)
- Electric (overhead) (approximately 25' north)
- Sanitary sewer pipe (approximately 120 feet north)

The following utilities are under Hillsdale Road relative to the US 41 centerline:

- Fiber optic cable (approximately 100 feet west)
- Fiber optic cable (approximately 150 feet west)
- Water pipe (approximately 210 feet west)
- Fiber optic cable (approximately 250 feet west)
- Sanitary sewer pipe (approximately 90 feet east)
- Fiber optic cable (approximately 100 feet west)
- Fiber optic cable (approximately 150 feet west)
- Fiber optic cable (approximately 290 feet east)

The following utilities parallel US 41:

- Sanitary sewer pipe (approximately 80' east)



- Water pipe (south of Hillsdale Road) (approximately 85 feet east)

The following utilities parallel Hillsdale Road west of US 41:

- Fiber optic cable (approximately 20 feet north)
- Fiber optic cable (approximately 13 feet north)
- Water pipe (approximately 10 to 14 feet south)
- 3 each fiber optic cable (approximately 18 to 26 feet south)

The following utilities parallel Hillsdale Road east of US 41:

- Fiber optic cable (approximately 15 feet south)
- Water pipe (approximately 18 feet south)
- 3 each fiber optic cable (approach) (approximately 22 feet south)
- Gas (at Walnut Road and east) (approximately 17' north)
- Electric (overhead) (both sides)

### **Field Check:**

A field check was held December 2, 2015 with the minutes provided in the appendices.

### **Coordination:**

A conference call meeting was held on was held December 18, 2018. The minutes of this meeting are provided in the appendices.

### **Previous Studies:**

A Corridor Study was completed in 2018 that examined the traffic operations for a large segment of the US 41 corridor. The project intersection is included in that study. The Corridor Study recommends that the Hillsdale Road intersection be converted to a J-Turn intersection with the west approach closed. The Corridor Study also recommends that the close-by intersections of Radio Avenue (0.25 mi north) and Old State Road/Campbell Road (0.50 mile north) be modified. The recommended modification for Radio Avenue is closure and the recommended modification for Old State Road/Campbell Road is a J-Turn with the east approach closed and Old State Road north of Campbell Road realigned to intersect Campbell Road.

This assessment has taken the recommendations of the 2018 Corridor Study under consideration and has examined the traffic redistribution effects with the Hillsdale Road intersection improvements.

A Mini-Scope that was completed by INDOT in 2013 is included in the appendices.

## **Traffic Analysis:**

Traffic count data was obtained at all intersections within the area bordered by Hillsdale Road, Old State Road, Campbell Road and Walnut Road. This area encompasses the traffic network for considering local redistribution for various intersection modification scenarios. The obtained traffic data was provided by INDOT for all intersections within the study area.

An analysis of the network was conducted to determine the impact of redistributed traffic on the intersections (1) when converting the Radio Avenue intersection to a right-in / right-out configuration and; (2) when converting the west leg of Hillsdale Road to a right-in / right-out configuration.

The existing local road network north of Hillsdale Road and east of US 41 (Radio Avenue, Walnut Road and Campbell Road) consists of low volume streets serving primarily low-density residential properties. Old State Road is a north-south major collector roadway. Hillsdale Road is an east-west local road linking Old State Road (major collector) and Browning Road (major collector). Wortman Road is an east-west local road linking Darmstadt Road (minor arterial) and Old State Road (major collector).

The general findings of the network analysis is that (1) converting the Radio Avenue intersection at US 41 to a right-in / right-out configuration will have no significant impact on the local street intersections and; (2) reconfiguring the west leg of the Hillsdale Road intersection to a right-in / right out access and not providing a south median U-turn (MUT) at US 41 will exacerbate the operational deficiencies at the Old State Road/Campbell Road intersection at US 41.

### **Intersection Alternatives**

A Two-Way Stop-Controlled (TWSC) (existing), conventional traffic signal, J-Turn and R-Cut alternatives were examined for traffic operations. The general findings are a conventional traffic signal, J-Turn and R-Cut intersections will significantly outperform the TWSC configuration and the J-Turn intersection significantly outperforms the traffic signal alternatives. A summary table of intersection traffic operation performance is included in the appendices.

## **Crash Data and Analysis:**

Crash data was obtained for the period extending from January 2016 through December 2018. The crash data was gleaned to include only those crashes that occurred within the limits of the Hillsdale Road intersection. This data was then used to determine crash index values using RoadHAT. The cataloged crash data is included in the appendices. The following table summarizes the severity of crashes at the Hillsdale Road / US 41 intersection:

<b>Crash Severity</b>	<b>2016</b>	<b>2017</b>	<b>2018</b>	<b>Total</b>
Property Damage	2	4	3	9
Non-Incapacitating Injury	0	1	0	1
Incapacitating Injury	0	2	3	5
Fatal	1	0	0	1
<b>Totals</b>	<b>3</b>	<b>7</b>	<b>6</b>	<b>16</b>

In reviewing the RoadHAT report, these are important factors:

Index of Crash Frequency:

The index of crash frequency ( $I_{CF}$ ) measures the difference between expected and reported number of crashes divided by the standard deviation of the difference estimate. For example,  $I_{CF} = 2$  indicates that the number of crashes exceeds the expected number of crashes for that location by two standard deviations.

Index of Crash Cost:

The index of crash cost ( $I_{CC}$ ) measures the difference between expected and estimated crash cost at the location divided by the standard deviation of the difference. For example,  $I_{CC} = 2$  implies that the crash cost at the location exceeds the expected crash cost for that location by two standard deviations. This method uses crash cost to incorporate severity.

The Index of Crash Frequency was found to be 2.58. The Index of Crash Cost was determined to be 2.56.

According to the Traffic Engineering Handbook, 6<sup>th</sup> Edition, potential factors in Right-Angle Crashes include restricted sight distance, excessive speed, inadequate roadway lighting, inadequate advance warning signs, large traffic volumes, and inadequate traffic control devices. Countermeasures to address these accidents include removal of the sight obstruction, provide all-way stop or signal, install or improve warning signs, reduce speed limit with enforcement, install rumble strips, install or improve lighting, install or improve warning signs, or reroute traffic.

Left & right turn crashes can be caused by large turn volumes, restricted sight distance, and excessive speed. Possible countermeasures include adding turn lanes, prohibiting left turns, rerouting left turn traffic, providing a traffic signal with a turn phase, removing sight obstructions, reducing the speed limit and improving enforcement.

Rear end crashes can be caused by large turning volumes, slippery pavement, or inadequate roadway lighting. Potential countermeasures include prohibiting left turns,

provide turn lanes, increasing the corner radius for right turns, reducing the speed limit with enforcement, overlaying the pavement or use an ultra-thin bonded wearing course, provide adequate drainage, groove the pavement, provide “Slippery When Wet” signs, and improve the lighting.

The findings of the crash analysis indicate that the most frequent accident type is right angle collisions (56%) followed by left turn collisions (19%). With 75% of the intersection collision involving cross-traffic conflicts, the most likely factor leading to crashes is traffic volume.

## **Alternatives:**

### **No-Build**

The current performance of the existing two-way stop intersection indicates that an improvement to this intersection is desirable both on a safety basis and a level-of-service basis. This alternative is not recommended because it will perpetuate these undesirable measures.

### **Slotted Turn Lanes**

This alternative was considered to improve visibility for left-turning vehicles on US 41 prior to initiating the turn maneuver. This alternative would not provide the desired safety improvements for vehicles entering US 41 from Hillsdale Road.

### **Traffic Signal (Conventional)**

Installing a conventional traffic signal will provide improved safety performance for turning movements, however the highest percentage (approximately 90%) of intersection traffic is the through movement along US 41. The delay introduced to the northbound and southbound through movements would negatively impact corridor mobility by introducing intersection delay for the through movements.

If implemented, the Hillsdale Road approaches would require a significant amount of reconstruction in order to accommodate left turn lanes for eastbound and westbound traffic entering US 41. It is estimated the construction cost for this alternative would be \$2 million to reconstruct 500 feet of both the east and west approaches in order to accommodate EB and WB left turn lanes, construct SB and NB right-turn lanes, install the new traffic signal, remove the reverse crown along the outside SB lane and reconstruct the median opening. The east approach bridge over Little Pigeon Creek would need to be widened to accommodate the added lane. The profile grade along the west approach would have to be raised to improve the stopping sight distance and the sharp vertical breaks at the railroad crossing would need to be removed for continuous travel under a green light condition. There would be significant right-of-way impacts and utility impacts associated with the alternative.

A CFM countermeasure value of 0.23 (77% reduction) applies to the angle crash category when converting a stop-controlled intersection to a signal-controlled intersection as published in the Crash Modifications Factors Clearinghouse. However, it

may be expected that rear-end crashes will increase as indicated by a CFM countermeasure value of 1.58 (58% increase). The high speeds, high traffic volume and significant volume of commercial trucks (approximately 13%) make this alternative less desirable.

This alternative is not recommended because similar angle crash reduction benefits can be attained by other alternative treatments without increasing rear-end crashes while not significantly restricting the mobility for through traffic along US 41.

### **Traffic Signal (R-Cut - partial)**

An R-Cut intersection was considered as an alternative treatment to a conventional traffic signal installation. Traffic signals at the median U-turns were examined with no traffic signal at the main intersection for US 41 left turns onto Hillsdale Road due to their low volumes.

The WB movement for 2020 AM traffic was used to test the R-Cut signal and compare to the conventional signal performance. This directional movement was chosen because it is the critical conflict movement for both AM and PM volumes. It was found that there would be no performance benefit to a R-Cut traffic signal as compared to a conventional signal for this specific project. The R-Cut alternative is not recommended because it does not exceed the performance of a conventional traffic signal installation.

### **J-Turn RECOMMENDED**

Converting the existing intersection to a J-Turn configuration will improve the safety performance while maintaining corridor mobility. A CFM countermeasure value of 0.20 (80% reduction) applies to the angle crash category as published in the Crash Modifications Factors Clearinghouse. The predicted reduction in angle crashes and the low impact on corridor mobility meet the desired objectives of this project.

The J-Turn will add additional travel time for the eastbound and westbound traffic movements as compared to a conventional traffic signal. The added travel time is estimated to be approximately two minutes.

This alternative is recommended to advance to project development and ultimately to implementation.

### **J-Turn with West Approach Right-in/Right-out & No South Median U-Turn (MUT)**

Converting the existing intersection to a J-Turn configuration with right-in / right-out access will provide similar safety and operation benefits as the full J-turn intersection. Eliminating the south MUT alternative was considered in combination with the restricted access at the west approach. The directional movements that would be eliminated are the eastbound left turn, the eastbound through and the northbound left turn.

This alternative is not recommended because it would be disruptive to the local road network and it would direct additional traffic to the Old State Road / US 41 intersection. The Old State Road / US 41 west approach currently operates at a level of service F. Adding a dedicated left turn lane at this approach was tested and was found to not improve the west approach level of service.

## **Roundabout**

Installing a multi-lane roundabout would provide improved intersection safety but would have a negative impact on US 41 mobility. The approaches along Hillsdale Road would require significant reconstruction to attain adequate entry geometrics and sight lines.

Roundabouts provide safety benefits in several ways. Many potential vehicle conflict points are eliminated, crash severity potential is significantly reduced by nearly eliminating right-angle conflicts and reducing speeds through the intersection. A CFM countermeasure value of 0.17 (83% reduction) applies to the angle crash category as published in the Crash Modifications Factors Clearinghouse

The US 41 traffic volumes represent approximately 90% of the traffic through the Hillsdale Road intersection. Reducing operating speeds for all vehicles to accommodate Hillsdale Road traffic is inefficient from a traffic operations perspective. The appreciable truck volumes (approximately 13%) along US 41 would exacerbate those inefficiencies.

The construction cost for a multi-lane roundabout at this location is estimated to be approximately \$3 million. Approach work on Hillsdale Road would require grade modifications to the west leg and a bridge reconstruction on the east leg. This alternative was rejected because similar safety benefits can be attained by implementing a J-turn intersection while not significantly impacting the mobility for through traffic along US 41.

## **Interchange**

Constructing a new interchange at this location would provide improved safety while limiting the impact on US 41 corridor mobility. However, the cost and impacts associated with this type of intersection improvement would be of the highest magnitude as compared to the alternatives. Construction cost for a standard diamond configuration would be approximately \$18 million, and the property and environmental impacts would be significant. This alternative was rejected due to the high costs and impacts.

## **Grade Separation on New Alignment**

This alternative examined a realignment of Hillsdale Road with bridge structures spanning over US 41, the CSX railroad and Little Pigeon Creek. Connector roadways would be built off the realigned roadway to the existing Hillsdale Road to maintain local access to US 41. The existing Hillsdale Road intersection would be converted to right-in right-out access at US 41. This alternative would cost approximately \$12 million to construct, and the property and environmental impacts would be significant. This alternative was rejected due to the high costs and impacts.

## **Recommended Alternative:**

This section provides information on developing this project as a J-Turn intersection.

### Design Criteria

US 41 is a Principal Arterial highway on the National Highway System (NHS). Hillsdale Road is a Local Road. This project intends to make improvements to the existing at-grade intersection.

Project Location: RP 10.86 to 11.28  
38.0795 N  
87.5549 W

#### US 41

AADT (2020):	21,200 vpd
AADT (2045):	27,200 vpd
Design Speed:	60 MPH
Design Criteria:	3R (Non-Freeway)
Functional Classification:	Principal Arterial - Other
Rural / Urban:	Rural
Terrain:	Level
Access Control:	Partial

#### Hillsdale Road

AADT (2020)	1,825 vpd
AADT (2045)	2,350 vpd
Design Speed:	45 MPH
Design Criteria:	3R (Non-Freeway)
Functional Classification:	Local Road
Rural / Urban:	Rural
Terrain:	Level

### Project Description

The newly configured intersection will maintain all traffic movements. The westbound and eastbound left-turns and through traffic on Hillsdale Road will turn right, cross the two US 41 thru lanes, enter the J-turn median lane, proceed through the median U-turn, stop for oncoming US 41 traffic, and complete the U-turn movement onto US 41.

The median deceleration lane for left turns onto Hillsdale Road are established as a deceleration length of 530 feet (profile grades less than 2.00%), storage length of 50 feet (D<sub>HV</sub> < 60) and a taper length of 100 feet for a total minimum length of 680 feet. The separation between the deceleration lane taper and the inside U-turn edge is 150'. These dimensions result in the median U-turn being situated approximately 950 feet from the intersection centerlines of US 41 and Hillsdale Road.

The existing intersection with Radio Avenue is recommended to be converted to a right-in / right-out configuration in conjunction with this project. The Radio Avenue intersection is approximately 350 feet north of the U-turn for the north J-turn lane. There is an extremely low volume of traffic utilizing this intersection. Eliminating the westbound left-turn movement will remove a potential conflict between left turns onto

southbound US 41 from the north MUT and Radio Avenue.

The traffic analysis indicates that modifying the Radio Avenue access will not have a significant impact on the local road network's traffic operations. The existing pavement width along Radio Avenue, Walnut Avenue and Campbell Avenue vary from approximately 12 feet to 15 feet. Reconstructing Walnut Road is not recommended in association with the modifications to Radio Avenue due to the marginal benefits that would result.

The 2018 Corridor Study recommended removing the west approach to Hillsdale Road. Traffic data indicates that the Hillsdale Road corridor is a significant link in the local road network, terminating at north-south collectors both east and west of US 41. The nearest east-west links to Hillsdale Road that connect the east and west sides of US 41 are located approximately one mile north (Boonville-New Harmony Road) and 1.6 miles south (Mt. Pleasant Road). Because of the potential disruption to the local road network and the associated community impacts, the west leg of the Hillsdale Road / US 41 intersection is recommended to remain operational.

A northbound right-turn lane is warranted based upon traffic volumes for 2020. The southbound right-turn lane does not meet the warrant threshold for 2020 or 2045. However, a southbound right-turn lane is recommended because; (1) it will provide acceleration space for vehicles completing the J-turn maneuver, (2) it will allow vehicles completing the westbound through movement through the MUT and entering the southbound lanes to stay out of the through traffic stream, (3) it will provide corridor consistency for planned modifications to intersections north and south of Hillsdale Road, and (4) it will provide an alternative access for the southbound to westbound movement at the time the Old State Road / US 41 intersection modifications are constructed.

Due to the narrowed existing median, the north MUT section will require construction of a loon that will extend beyond the apparent existing right-of-way. Guardrail will be required along the perimeter of the loon due to the embankment slope conditions. Constructing the southbound right-turn lane will require work outside of the apparent existing right-of-way to reconstruct the fill slope and the roadside ditch between US 41 and the CSX track set. Guardrail is anticipated along the shoulder for this right-turn lane due the slope conditions of the existing embankment.

The south MUT will utilize a portion of the existing northbound right-turn lane to complete the turning maneuver. The existing northbound right-turn lane serving the commercial entrance south of Hillsdale Road should be extended to Hillsdale Road.

The south MUT has a grade differential of approximately 2 feet between the northbound and southbound inside edge of travel lane, separated by a horizontal distance of approximately 67 feet. The MUT can be constructed with a centerline profile grade of 3% or less, which is acceptable for at-grade intersections. The available intersection sight distance will exceed minimum requirements.

## Improvements



The following improvements are included in this recommendation:

- Add a northbound J-Turn lane and MUT in the median.
- Add a southbound J-Turn lane and MUT in the median.
- Extend the existing northbound right-turn lane for the east approach.
- Add a southbound right-turn lane for the west approach.
- Reconstruct the median opening to accommodate dedicated northbound and southbound left turn lanes.
- Reverse the crown on the southbound outside lane to improve intersection sight lines at the west approach.
- Reconstruct the west approach to the east edge of the railroad header.
- Mill and resurface the existing pavement within the project limits.
- Mill and resurface the east approach to the west bridge joint (bridge over Little Pigeon Creek).
- Install highway lighting along the northbound and southbound lanes for the length extending from the south MUT to the north MUT.
- Install new signage.
- Install new pavement markings within the project limits.

#### Modifying Radio Avenue:

- Reconstruct the median to remove the southbound left turn and the westbound left turn.
- Install new signs and pavement markings.

#### Future Improvement Considerations:

The Corridor Study identified US 41 intersection improvements at Hillsdale Road, Radio Avenue and Old State Road/Campbell Road that may be developed at some point in the future. This assessment of alternative improvements at Hillsdale Road and the recommended alternative are generally consistent with the conclusions of the Corridor Study. The following modifications are identified in the Corridor Study:

- Remove the Hillsdale Road west approach at US 41.
- Do not provide a south MUT at Hillsdale Road.
- Remove the Radio Avenue intersection at US 41.
- Remove the Campbell Road east approach at US 41.
- Construct a continuous link between Campbell Road and Old State Road on the east side of US 41.
- Construct improvements to Campbell Road and Walnut Avenue.

### **Maintenance of Traffic During Construction:**

The recommended alternative (J-Turn) can be constructed under traffic. The construction activity will primarily occur within the median with one lane of traffic for both

northbound and southbound US 41 traffic on the existing outside lanes. The paved intersection median at Hillsdale Road should remain open while the median lanes are constructed. The outside lanes of US 41 should be constructed in the second phase. The final phase of construction should be completing work in the median through the intersection. During this phase, only right turns out of the east and west approaches will be permitted.

Posting I-69 as an alternate route (4 miles east) during construction should be considered to remove a portion of through traffic along US 41 through the work zone and to provide motorists with an option to avoid the construction zone.

The construction speed limit should be posted at 50 mph. Temporary traffic barriers should be utilized to protect the work zone.

A queuing analysis was completed to estimate the sensitivity to traffic backups when applying a lane-drop along US 41 to provide safe working room during construction. The method used to conduct the analysis was the INDOT Queuing Analysis spreadsheet (version 1.26). The analysis indicates that the queuing is within policy limits for both northbound and southbound lane-drops. A copy of the analysis is included in the appendices.

**Cost Estimate:**

The cost estimate for the Recommended Alternate is as follows:

Construction Cost (CN).....	\$ 2,100,000
Preliminary Engineering (PE).....	\$ 170,000
Environmental Documentation.....	\$ 40,000
Right of Way Engineering and Acquisition.....	\$ 50,000
Railroad and Utility Coordination.....	\$ 150,000
Construction Engineering.....	<u>\$ 200,000</u>
Total Project Cost.....	\$ 2,710,000

**Environmental Issues:**

The construction activity for this project will be confined primarily to staying within the apparent existing right-of-way. A length of temporary right-of way will be needed along the west side of US 41 to reshape the roadside ditch. A small area of permanent right-of-way will be required at the north end of the project along the west apparent existing right-of-way to accommodate the J-turn loon and guardrail installation. Since the property owner along the west side of US 41 is CSX railroad, an Agreement may be utilized instead of acquiring right-of-way (temporary or permanent). The final decision

should be made during the plan development process.

It is anticipated that less than one acre of unpaved surface will be disturbed during construction and that more than 0.5 acre of combined temporary and permanent right-of-way will be required. It does not appear that any regulated wetlands or streams will be impacted.

### **Railroad Agreement:**

A Railroad Agreement with CSX should be initiated as soon as possible. The agreement should cover the work that will be required within CSX apparent property. This work includes grading, seeding, pavement construction, guardrail installation and drainage structure modifications, as well as flagging during construction (if needed).

### **Survey Requirements:**

The survey limits will extend from 1,500 feet south of Hillsdale Road to 1,500 feet north of Hillsdale Road, and 600 feet west to 600 feet east of the US 41 centerline. The median area survey extends to 700 feet north of Radio Avenue. Much of this information has already been gathered from a previous scope-of-work for this project. A supplemental survey will be necessary to obtain current information in order to update the changed elements.

### **Right of Way Impacts:**

Temporary right-of-way will be required along the southbound outside shoulder for construction of the north J-Turn loop and a southbound right turn lane. There is a single property owner at this location (CSX railroad). An additional amount of pavement surface will be constructed for the west approach which also extends into CSX apparent property. The additional permanent right-of-way need is approximately 0.18 acres and the temporary right-of-way is approximately 0.70 acres. The right-of-way needs may be included in the Railroad Agreement as opposed to acquisition.

### **Utilities Impacts:**

There are several utilities that exist within the project limits as detailed previously in this report. It is not anticipated that any utility relocations will be required as a part of this project.

## **Drainage Structures:**

Culverts located within the project limits:

- 48" RCP under the west approach with concrete headwalls/wingwalls approximately 88' west of US 41 centerline. This structure will need to be extended at both the south and north ends to accommodate approach widening. This structure is in good condition with a current rating of 7.
- There are 2 median inlets south of Hillsdale Road that will require adjustments due to median regrading.
- There are 2 median inlets north of Hillsdale Road that will require adjustments due to median regrading.

## **Changes to Proposal:**

The Project Manager and the District Scoping Engineer shall be consulted if deviation from the proposal is determined to be necessary during a later phase of project development. The person initiating the change shall send a memo to the Project Manager for concurrence. The memo shall include justification for the change and the estimated cost difference.

## Report Concurrence Block:

This document was prepared by:

*Michael Koyak*

Date: 8 /23 /2019

Michael J. Koyak, P.E.,  
First Group Engineering, Inc.



Reviewed by:

*David Reamer*

Date: 9/3/2019

David Reamer  
Vincennes District Asset Engineer

Reviewed by:

*Duane D. Decker*

Date: 9/3/2019

Duane Decker  
District Scoping Manager

Reviewed by:

*Khalil I Dughhaish*

Date: 9/6/2019

Khalil Dughhaish  
District System Asset Manager

## **APPENDICES TABLE OF CONTENTS:**

APPENDIX A	VINCENNES DISTRICT MINI-SCOPE (2013)
APPENDIX B	FIELD CHECK MEETING MINUTES (2015) COORDINATION MEETING (2018)
APPENDIX C	RECOMMENDED ALTERNATIVE DRAWINGS: J-Turn Layout Truck Path & Dimensions Radio Avenue Modifications
APPENDIX D	CRASH SUMMARY Crash Analysis RoadHAT Reports: Index of Crash Frequency and Cost Benefit Cost Analysis
APPENDIX E	LOS SUMMARY - NETWORK
APPENDIX F	INTERSECTION PERFORMANCE SUMMARY TABLE
APPENDIX G	PRELIMINARY CONSTRUCTION COST ESTIMATE
APPENDIX H	QUEUING ANALYSIS (MOT)
APPENDIX I	TRAFFIC VOLUME MAPS 2020 No Build 2020 Radio Ave. R-in/R-out 2020 Hillsdale Rd. (west) & Radio Ave. R-in/R-out 2045 No Build 2045 Radio Ave. R-in/R-out
APPENDIX J	TRAFFIC FORECASTS (by INDOT)
APPENDIX K	RIGHT TURN LANE WARRANT
APPENDIX L	MAPS National Wetland Inventory USGS Quadrangle Urbanized Boundary
APPENDIX M	PHOTOGRAPHS
APPENDIX N	HCM TRAFFIC REPORTS

## **APPENDIX A**

# Call Application Report Project ( Mini Scope)

Date:	11/21/2013	District:	VINCENNES
DES:	1400005	Sub-District:	Evansville
Proposed FY:	2019	Asset Group:	SAFETY

## Project Location

Route:	US 41	City/Town:	Evansville	County:	Vanderburgh	NHS:	Yes
RP Start:	10.90	RP End:	11.1	AADT FY:	2011	AADT:	26042.0
Length:	0.2	# Lanes:	4	Lane Mi:	0.8	% Trucks:	11.0%
Func. Class:	Principal Arterial (Freeway/Expressway)			Area:	Urban	Number of Counties:	1
Bridge/ Culvert Length (FT):	NA	Bridge Area (SFT):	NA	Year Built:	NA		

Location Description: US 41 and Hillsdale Road Intersection, 2.04 miles North of SR 57

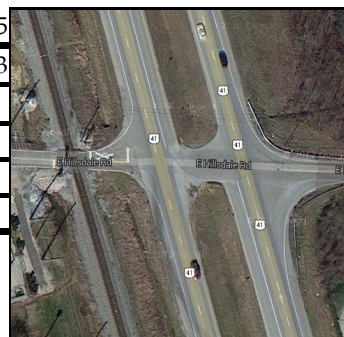
## Existing Conditions and Description of Problem

**WHAT IS THE CURRENT AND PROJECTED CONDITION AND WHY IS THIS A PROBLEM (INITIAL STATEMENT OF ESSENTIAL PROJECT NEED) AND CONSIDER DATE AND OTHER ISSUES TO THE PROBLEM (FOCUS ON PROBLEM):**

Due to customers requests in increase volume and accidents at the intersection of US 41 & Hillsdale, the District conducted a traffic study and analysis of the intersection. The traffic study found that the existing conditions warrant a signal in accordance to the Indiana Manual on Uniform Traffic Control Devices (IMUTCD).

TYPE OF LAST MAJOR TREATMENT: HMA PM Overlay      DATE: 2005

<b>PROJECT CONDITION RATINGS:</b>	<b>LOS:</b> NA	<b>Crash Rate:</b> 2.43
<b>Wearing Surface:</b> NA	<b>Deck:</b> NA	<b>Bridge/Culvert Super:</b> NA
<b>Bridge/Culvert Sub:</b> NA	<b>Bridge Scour:</b> NA	<b>Bridge Paint:</b> NA
<b>Type I Culverts/ pipes:</b> NA	<b>Channel:</b> NA	<b>Roadway:</b> NA
<b>IRI:</b> NA	<b>PCR:</b> NA	<b>RUT:</b> NA
		<b>Friction #:</b> NA



**INTENT/ PURPOSE OF PROJECT (INITIAL STATEMENT OF ESSENTIAL PROJECT PURPOSE:**

The purpose of the project is to improve the safety of the intersection of US 41 and Hillsdale Rd as well as improving the level of service for Hillsdale Rd approaches.

## Alternatives

**PRELIMINARY ALTERNATIVES THAT ARE CONTEMPLATED (ANALYSED) WITH COSTS:**

1. Install a traffic signal, install right-turn lanes along US 41, increase corner radii, increase the left-turn lane lengths along US 41. \$2,173,000 - Recommended
2. Same as (1), but add another lane on east approach. This would be much more costly than alternative (1) because the bridge on the east approach would have to be bought (not on INDOT R/W) and widened as well as relocate a water and sewage line on the south side of the bridge. Level of Service is not significantly improved with added lane east approach.
3. Same as (1), but add another lane on west approach. This would be much more costly than alternative (1) because of a power station on southwest corner of intersection and railroad on west approach. Level of Service is not significantly improved.

**CONSEQUENCES IF NO ACTION IS TAKEN (DO NOTHING ALTERNATIVE IS SELECTED):**

*If no improvement is made at this intersection, then the pattern of accidents will continue and the LOS for the east approach will continue to be an F.*

**SECONDARY CONSIDERATIONS OR GOALS WITH COSTS:**

*There are no secondary considerations besides those mentioned above.*

**Attach extra sheets as necessary to fully describe the alternatives.**

**Will Further Analysis/Assessment be required beyond this form?** NO



**Project Recommendations and Costs**

**QUANTIFIABLE PRIMARY GOAL(S) OF PROJECT (WHAT ARE WE PURCHASING SUCH AS CONDITION, SERVICE LIFE, LOS, OR CRF):**

GOAL: The goal of this project is to decrease the amount of accidents at the intersection. The signal should decrease the total number of accidents by 15% by installing a signal. Installing right-turn lanes along US 41 will decrease the accidents by 8%. Increasing the left-turn lane lengths will decrease the accidents by 40%.

Estimated Total Project Costs:		\$2,173,000.00	COMMENTS
In-House Design:	NO	COST: \$0.00	There are no anticipated in-house design costs.
R/W:	NO	COST: \$0.00	No new right-of-way should be needed.
PE:	YES	COST: \$174,000.00	There are anticipated PE costs.
GEOTECH:	YES	COST: \$52,000.00	Geotech design will be needed.
Hydraulics:	NO	COST: \$0.00	Hydraulic design should not be needed.
Maintenance of Traffic:	YES	COST: \$87,000.00	The roadway will remain open during construction.
Railroad:	YES	COST: \$100,000.00	Railroad coordination will be needed. A RR agreement is needed.
Environmental Study:	YES	COST: \$0.00	CE 1. Permits may be needed.
Utilities:	YES	COST: \$20,000.00	There are anticipated utility concerns.
Pavement:	YES	COST: \$0.00	A pavement design will be needed.
CN:	YES	COST: \$1,740,000.00	This is total construction cost.
Other Considerations:	NO	COST: \$0.00	There are no other anticipated costs associated with this.

**Other Projects within Limits**

DES:	0200849	FY:	2016	Work Type:	PM Overlay	Location:	RP 4.12 to RP 12.00
DES:		FY:		Work Type:		Location:	
DES:		FY:		Work Type:		Location:	

**Miscellaneous Notes**

ANTICIPATED NUMBER OF CONSTRUCTION SEASONS TO COMPLETE(1, 2 or 3 seasons):	1 FY
ANTICIPATED NUMBER OF YEARS TO COMPLETE DESIGN (1, 2 or 3 fiscal years):	1

**CALL HISTORY:** This was not submitted in a previous call.

**Attachments**

Pictures:	YES	Asset Team Scoring Sheet:	YES	Mobility History:	NO
Spreadsheets (calcs):	NO	Engineer Assessment:	NO		
Solution Schematic:	YES	Bridge/Culvert Inspection Report:	NO		
Cost Calculations:	YES	Accident History:	YES		
Location Map:	YES	Pathway Data:	NO		

**Additional Comments**

Other items relevant to the project not specifically listed elsewhere

NOTE: Appropriate environmental and assessment processes need to be followed.

**Report Prepared By and Approved By**

Name: <u>Brittney Smith / Damon Brown</u>	Title: <u>Scoping Engineer / Traffic Planning Engineer</u>
TSD: <u>Valerie Cockrum</u>	APPROVED ON: <u>12/31/2013</u>

NOTE: Any changes require a re-submittal of Call Application Report.

REVISED: <u>11/18/2013</u>	REVISED BY: <u>Andrew Fitzgerald, PTOE, PE</u>
----------------------------	--

## **APPENDIX B**



**Consulting Engineers**  
 5925 Lakeside Blvd.  
 Indianapolis, IN 46278  
 Phone (317) 290-9549  
 Fax (317) 290-9560

---

6949 Kennedy Avenue  
 Unit F  
 Hammond, IN 46323  
 Phone (219) 844-6752  
 Fax (219) 844-6795

**Engineering Assessment Field Check Minutes for  
 US 41 at Hillsdale Road  
 Intersection Improvements  
 Vanderburgh County, Indiana  
 Des. No. 1400005**

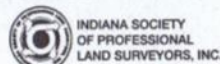
<b>Name of Attendee</b>	<b>Representing</b>	<b>Telephone Number</b>	<b>E-Mail</b>
Shawn Will	INDOT – Project Manager	(812) 895-7358	<a href="mailto:shwill@indot.in.gov">shwill@indot.in.gov</a>
David Farmer	INDOT – Railroad	(812) 895-7365	<a href="mailto:dfarmer@indot.in.gov">dfarmer@indot.in.gov</a>
Khalil Dughhaish	INDOT – Technical Services	(812) 895-7377	<a href="mailto:kdughhaish@indot.in.gov">kdughhaish@indot.in.gov</a>
Damon Brown	INDOT – Traffic Planning	(812) 895-7457	<a href="mailto:dabrown@indot.in.gov">dabrown@indot.in.gov</a>
Alex Loehrlein	INDOT - Utilities	(812) 453-7291	<a href="mailto:aloehrlein@indot.in.gov">aloehrlein@indot.in.gov</a>
John Stoll	Vanderburgh County Engineer	(812) 435-5773	<a href="mailto:jstoll@vanderburghgov.org">jstoll@vanderburghgov.org</a>
Jeff Hicks	First Group Engineering	317-216-7705 #208	<a href="mailto:JHicks@FirstGroupEngineering.com">JHicks@FirstGroupEngineering.com</a>
Jeff Brechbill	First Group Engineering	317-216-7705 #217	<a href="mailto:JBrechbill@FirstGroupEngineering.com">JBrechbill@FirstGroupEngineering.com</a>

The Engineering Assessment Field Check for this project was held at 10:00 AM Eastern Time on Wednesday, December 2, 2015 at the project site, which is located approximately 2.04 miles north of SR 57 in Vanderburgh County, Indiana. A summary of the major items discussed is shown below.

1. Jeff Brechbill gave a brief introduction, project history, and project overview.
  - Originally large US 41 corridor reconstruction project
  - Project cancelled in 2009
  - Became a spot improvement – intersection project
  - Engineering Assessment (mini-scope) – by INDOT 2013
  - Recommendations included southbound right turn lane, northbound right turn lane, extension of both northbound and southbound left turn lanes to provide deceleration
  - Traffic signals were found to be warranted
  - First Group is currently providing an Abbreviated Engineering Assessment to come up with specific scope of work, considering Open Roads potential cost savings

STREETS – HIGHWAYS – TRAFFIC – STRUCTURES – WATER – SEWER – SURVEYING – RIGHT-OF-WAY SERVICES

ACTIVE MEMBERSHIPS:





**Consulting Engineers**

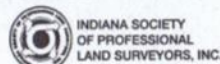
5925 Lakeside Blvd.  
Indianapolis, IN 46278  
Phone (317) 290-9549  
Fax (317) 290-9560

6949 Kennedy Avenue  
Unit F  
Hammond, IN 46323  
Phone (219) 844-6752  
Fax (219) 844-6795

2. INDOT and the Evansville MPO are requesting proposals for a corridor study of US 41 from the Lloyd Expressway to I-64, which includes this intersection. Any potential impacts to this project are unknown at this time. Due to the timing of the project, it was agreed that the results of the corridor study will not be incorporated into this Engineering Assessment.
3. A developer plans to construct apartment buildings on the property north of Hillsdale Rd. west of the railroad tracks. The developer is requesting that the property be re-zoned at a December 8<sup>th</sup> meeting. If re-zoning is approved, they plan to construct 300 units of 3-story apartments and possibly additional commercial development. If re-zoning is denied, they plan to construct 300 units of 2-story apartments instead.  
*[Darmstadt Town Council rejected the rezoning request for a 3-level apartment complex on December 8. The developer indicated that he still plans to construct over 300 units of 2-story apartments. The council is requiring the developer to conduct a traffic impact study.]*
4. It was stated that the apartment developer requested that improvements to accommodate increased traffic be added to this project. However, it was discussed that INDOT does not typically make improvements based on possible future developments. Jeff Brechbill stated that they will use the traffic counts and projections previously provided to them unless new traffic information is provided and requested by INDOT to be used instead.
5. Vanderburgh County has no plans to replace the existing bridge on Hillsdale Road just east of the intersection. Due to proximity of intersection, it would need to be widened or reconstructed to add turn lanes on the east approach. Damon stated that the addition of turn lanes on both the east and west Hillsdale Road approaches would not significantly improve the Level of Service (LOS) of the intersection with current traffic and projections based on current traffic. Therefore, no turn lanes will be added on Hillsdale Road.
6. It was agreed that no southbound right turn lane will be added. The survey shows no existing right-of-way on that side beyond the existing edge of pavement. The traffic counts for this movement are extremely low, it would require right-of-way or an easement from the railroad, and Open Roads considerations have influenced the decision not to provide the southbound turn lane.
7. Traffic signalization is recommended. As part of the original Mini-Scope, INDOT had performed a signal warrant analysis and determined that signals are warranted. Puck detection was requested at scope of work meeting.

STREETS – HIGHWAYS – TRAFFIC – STRUCTURES – WATER – SEWER – SURVEYING – RIGHT-OF-WAY SERVICES

ACTIVE MEMBERSHIPS:





**Consulting Engineers**

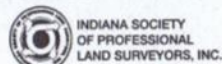
5925 Lakeside Blvd.  
Indianapolis, IN 46278  
Phone (317) 290-9549  
Fax (317) 290-9560

6949 Kennedy Avenue  
Unit F  
Hammond, IN 46323  
Phone (219) 844-6752  
Fax (219) 844-6795

8. Two viable Alternates appear to meet project objectives: 1) Remove adverse crown from the outside southbound travel lane and reconstruct Hillsdale Road to provide adequate Stopping Sight Distance over the train tracks. 2) Reconstruct US 41 southbound lanes to raise the profile grade to provide a smoother transition through the intersection. Both Alternates would require lowering the posted speed of Hillsdale Road to 30 mph.
9. John Stoll indicated that the County is willing to allow the lowering of the speed limit on Hillsdale Road west of US 41 to 30 mph. He said that he would ask their attorney, but does not anticipate that it will be a problem. The new speed limit signs could be placed as part of this project.
10. Conceptual roadway profiles for Hillsdale Road were shown to attendees for the two alternates. First Group mentioned that the railroad tracks and their proximity to US 41 will require Hillsdale Road to be raised west of the railroad tracks for both alternates. The driveway/access for the Electric Substation on the south side of Hillsdale Road on the west side of the railroad tracks would be closed and relocated to the west side of the Electric Substation due to the grade raise of Hillsdale Road in both alternates.
11. Since both alternates require a significant grade raise on Hillsdale Road west of the railroad tracks and have similar project limits, it is likely more cost-effective to choose the alternate that does not involve reconstruction of the US 41 southbound lanes.
12. An easement will be needed from CSX railroad for this project, as it is unlikely they would sell permanent right-of-way to INDOT.
13. Intersection lighting was discussed. It was decided that street lighting would be added to the proposed traffic signal poles to improve visibility at the intersection, as long as it can be provided within the project budget.
14. An R-cut was suggested as a possible third alternate by Damon Brown. The possibility was discussed, and one concern raised was the visibility of eastbound vehicles on Hillsdale Road on the east side of the railroad tracks. As a result, the profile of Hillsdale Road would still need to be raised to improve sight distance, so the overall cost of this would likely be considerably higher. This will be discussed in the Abbreviated Engineering Assessment.

STREETS – HIGHWAYS – TRAFFIC – STRUCTURES – WATER – SEWER – SURVEYING – RIGHT-OF-WAY SERVICES

ACTIVE MEMBERSHIPS:





**Consulting Engineers**

5925 Lakeside Blvd.  
Indianapolis, IN 46278  
Phone (317) 290-9549  
Fax (317) 290-9560

6949 Kennedy Avenue  
Unit F  
Hammond, IN 46323  
Phone (219) 844-6752  
Fax (219) 844-6795

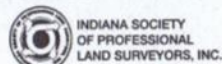
15. Northbound and southbound left turn lengths for US 41 were discussed. The mini-scope had indicated that they should be extended to provide adequate deceleration in accordance with the INDOT Design Manual. In considering Open Roads, it was decided that the AASHTO values for deceleration would be provided instead.
16. It was discussed and agreed upon that the northbound right turn lane will be extended to match into the existing northbound right turn lane just south of the gas station on the southeast corner of US 41 and Hillsdale Road.
17. INDOT has an upcoming paving project on US 41, and it may include a new guardrail end treatment on the southeast corner of US 41 at Hillsdale Road. First Group will review the plans to verify this.
18. INDOT utilities will look at sanitary sewer locations. Water may need to be relocated if it is located under pavement. It was noted that a waterline recently broke in this area.
19. It was decided that First Group will wait to hear the results of the zoning request and further direction before proceeding with finalizing the Abbreviated Engineering Assessment.

## END OF MINUTES

If any revisions are necessary please contact Jeff Brechbill at [JBrechbill@FirstGroupEngineering.com](mailto:JBrechbill@FirstGroupEngineering.com).

STREETS – HIGHWAYS – TRAFFIC – STRUCTURES – WATER – SEWER – SURVEYING – RIGHT-OF-WAY SERVICES

ACTIVE MEMBERSHIPS:







## MEETING MINUTES

Date: December 18, 2018

Project: Intersection Improvements, U.S. 41 at Hillsdale Road, Vanderburgh County, Indiana

**Des No: 1400005**

Engineering Assessment and Design

A conference call was held at 2:00 PM on Tuesday December 18, 2018 for the purpose of developing a path forward to complete the Engineering Assessment for which First Group Engineering (FGE) is currently under contract with INDOT and determine an overall project schedule.

Invitees to this meeting included:

Matthew Bullock, INDOT (Project Manager)  
Duane Decker, INDOT  
Alan Davis, INDOT  
Terry Bough, INDOT  
Khalil Dughaish, INDOT  
David Reamer, INDOT  
Jeff Brechbill, FGE  
Mike Koyak, FGE

The following items were discussed:

1. A supplemental agreement to the contract is necessary due to major changes in the planning for the US 41 corridor since the onset of the original contract.
2. FGE noted that a final Engineering Assessment has not been completed as a part of the current contract with INDOT. FGE has developed drawings for intersection improvements to install a traffic signal. The current drawings also address profile grade issues (west approach) related to the existing railroad crossing.
3. INDOT asked that FGE develop an Engineering Assessment that looks at alternative intersection designs to the Hillsdale Road intersection. The Engineering Assessment should evaluate the traffic impacts to the Radio Avenue intersection and the Old State Road intersection along U.S. 41 for each alternative design at Hillsdale Road.
4. The Engineering Assessment will include costs and benefits for each intersection alternative at Hillsdale Road.
5. INDOT will provide crash data for the project area.
6. The estimated costs (and justification for) for improvements to the Radio Avenue and/or Old State Road intersection(s), if any, should be estimated and included in the Engineering Assessment for each alternative design at Hillsdale Road.

7. INDOT indicated the MPO believes there may be more buy-in for alternative intersection designs than in the past.
8. INDOT would like to meet with representatives of the MPO and the County to discuss details of the alternative intersection designs at Hillsdale Road. FGE will prepare estimated costs of improvements for use at this meeting.
9. INDOT will provide traffic count data so that FGE can estimate impacts to U.S. 41 intersections at Radio Avenue and Old State Road resulting from traffic avoiding the Hillsdale Road intersection after an alternative intersection is constructed. FGE will make engineering judgments on traffic diversion from Hillsdale Road resulting from the installation of locally non-favored intersection treatments. INDOT will assist FGE in determining diversion rates.
10. The J-turn intersection is favored in the Corridor Study and is an alternative for the Engineering Assessment to examine, however; INDOT indicated that closing county roads is not preferred at his time.
11. INDOT asked that FGE examine if the location of the median cut for a J-turn will interfere with traffic operations at the existing Radio Avenue intersection.
12. INDOT asked that the Engineering Assessment include an alternative for a J-turn with a right-in/right-out (west approach) at the Hillsdale Avenue intersection. This alternative would eliminate the need for a median U-turn south of Hillsdale Road.
13. INDOT noted that the Engineering Assessment should include consideration for longer-term improvements at the Radio Avenue and Old State Road intersections that are consistent with the Corridor Study.
14. The Engineering Assessment/design project will be in 2 phases. The first phase is the Engineering Assessment. The second phase is design of the preferred alternative.
15. There is a desire to include public engagement beyond that of a public meeting as an element of the design contract.
16. INDOT will provide additional information to FGE for costs/benefits of traffic signals, J-turn and R-cut that has already been compiled by others for intersections within the project area.
17. FGE noted that a design fee cannot be accurately determined until a preferred alternative is decided.
18. FGE will obtain new utility locates and will conduct a site visit to verify, to the extent possible, the field information received.
19. The professional fees for the supplemental agreement will include conducting a Public Meeting. Recommendations will be presented at the Public Meeting. To expedite the project, the meeting will take place after the Engineering Assessment is completed and before design begins.





20. The project is currently programmed for a letting on 5/7/20. This results in Stage 3 plans being due by 12/13/19. INDOT indicated they discussed internally moving the project out 6 months (Nov. 2020 letting. Challenges to the schedule include railroad coordination and getting through the environmental process.

Regarding a revised project schedule:

- FGE said it would be 60 to 90 calendar days to complete the Engineering Assessment as described in this meeting. Note: FGE prefers the 90 days due adding an assessment of impacts to the Radio Avenue and the Old State Road intersections.
- FGE noted that it typically takes 60 calendar days to obtain a NTP from INDOT on contracts. INDOT indicated this process may possibly be streamlined on their end.
- FGE noted that railroad coordination could take up to 18 months depending on the alternative chosen and the need for a railroad agreement. INDOT will check internally with Michael Farmer to get his opinion on the time needed.

Revised Engineering Assessment NTP:	February 2019
Meeting w/ MPO & County:	March 2019
Revised Engineering Assessment Complete:	May 2019
Public Meeting:	June 2019

Design NTP:	TBD
Stage 3 Plans:	6/18/2020
Tracings:	8/2/2020
RFC:	9/2/2020
Letting:	11/10/2020

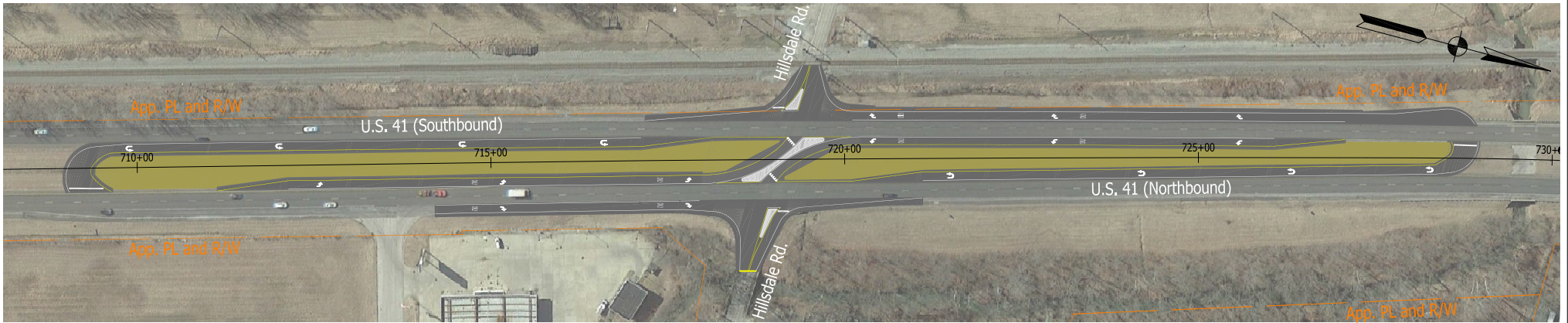
The above meeting notes represent the writer’s interpretation of discussions. If your interpretation varies significantly or if there are important omissions, please contact me and I will revise the record.

Sincerely,

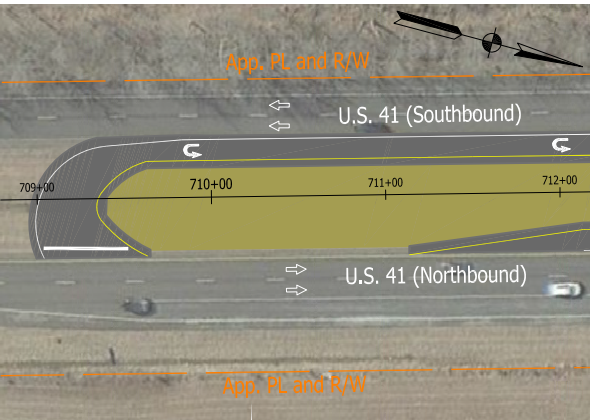
Michael Koyak, P.E.  
First Group Engineering, Inc.

Cc: All invitees (via email)

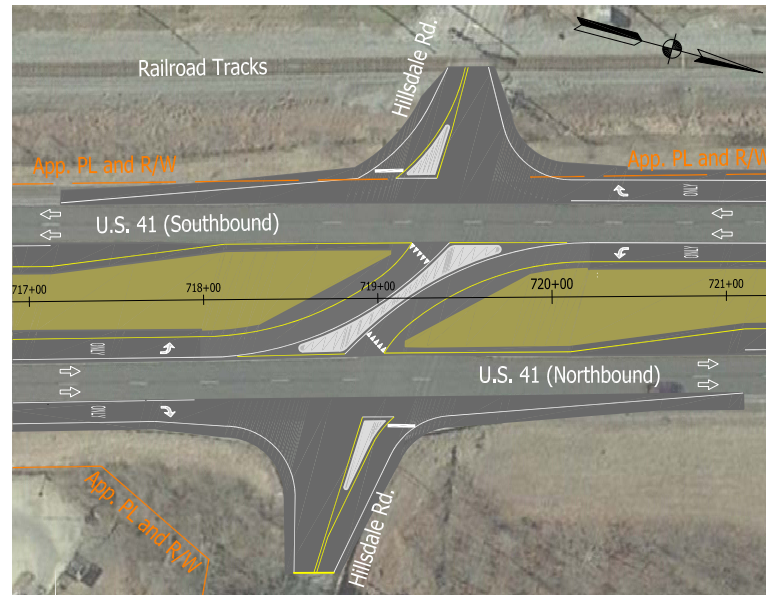
## **APPENDIX C**



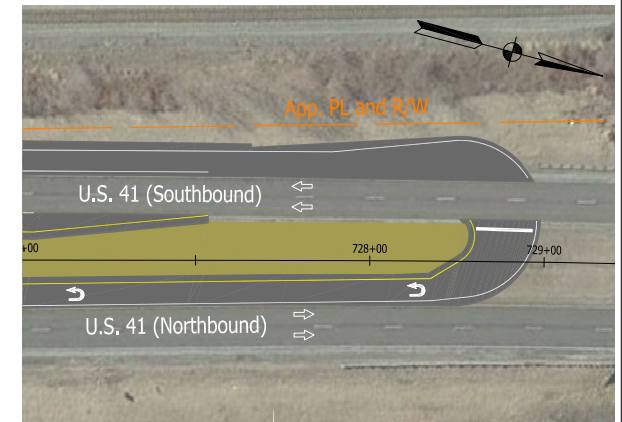
U.S. 41 and Hillsdale Rd. J-Turn Intersection Configuration



South End of J-Turn

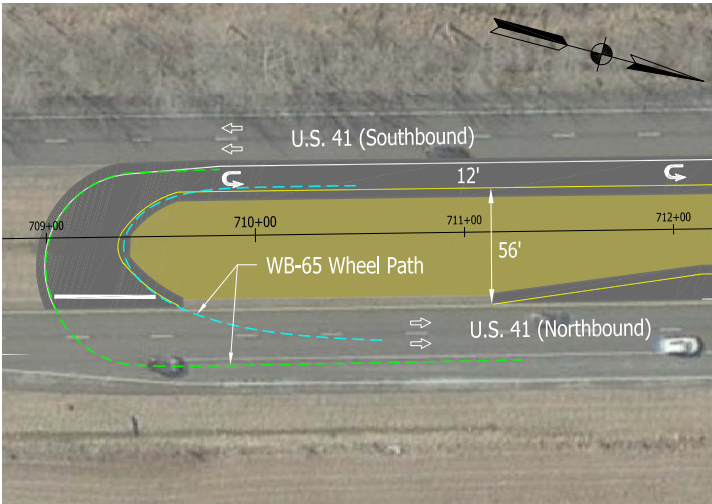
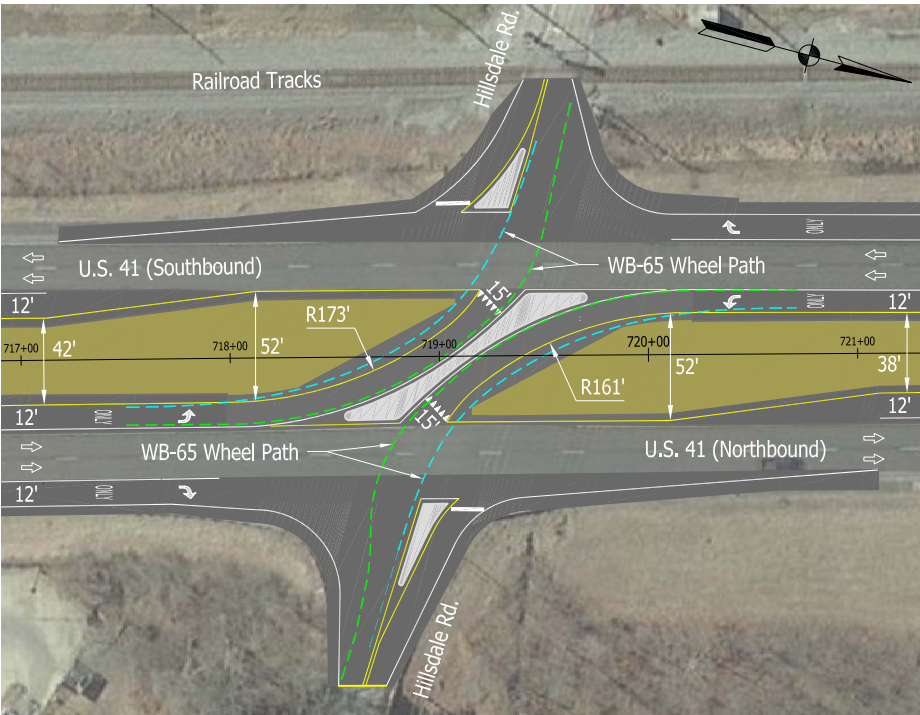
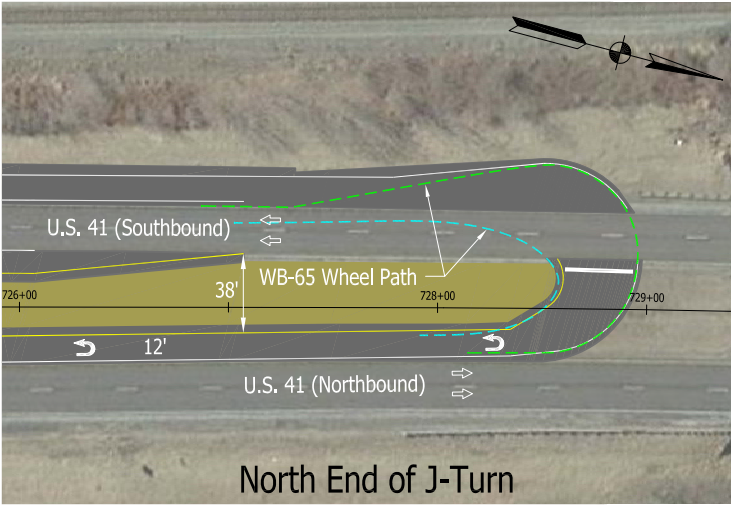


U.S. 41 and Hillsdale Rd. Intersection



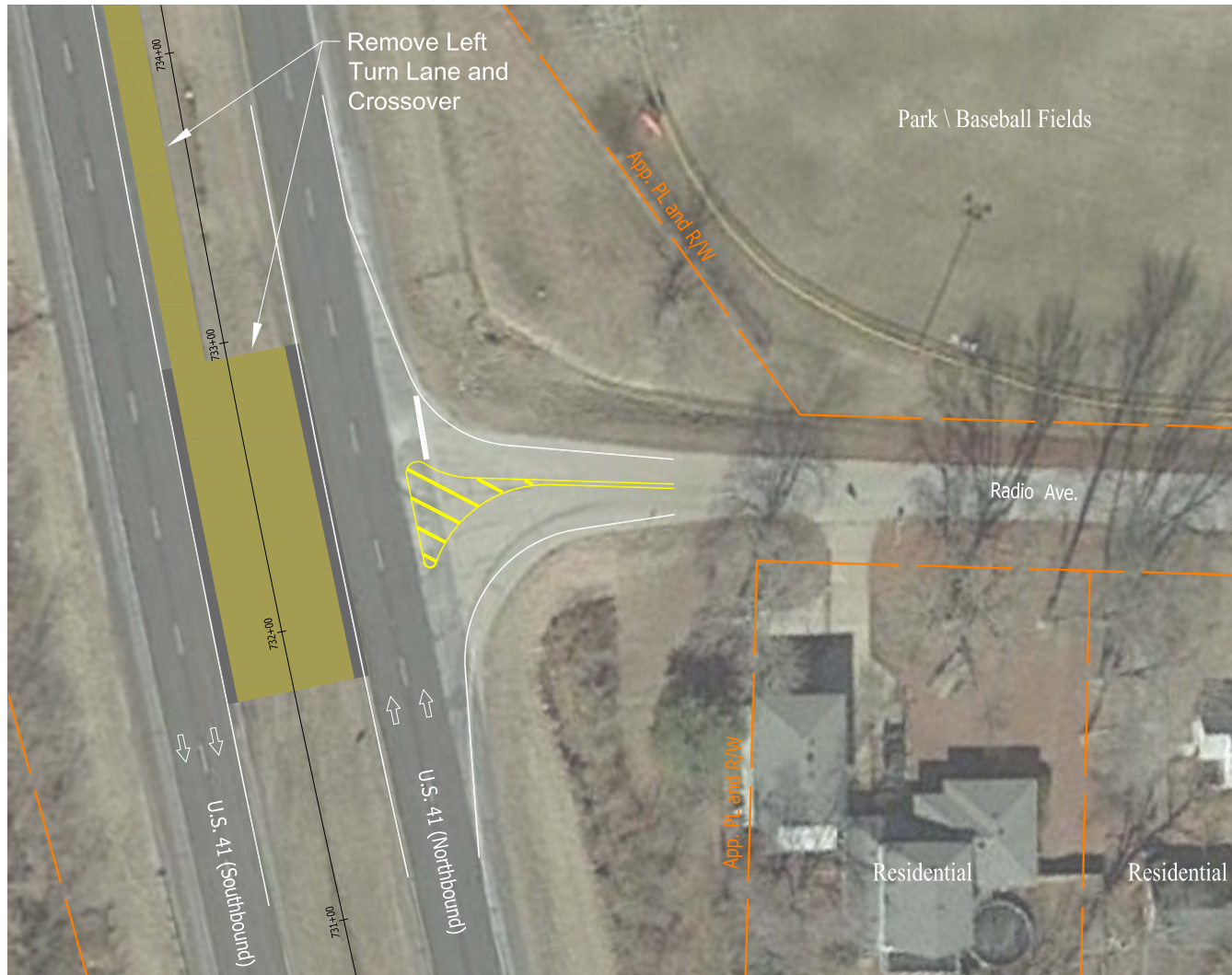
North End of J-Turn

**J-TURN LAYOUT  
U.S. 41 AT HILLSDALE ROAD  
VANDERBURGH COUNTY**



**J-TURN LAYOUT  
TRUCK TURN PATH AND DIMENSIONS  
U.S. 41 AT HILLSDALE ROAD  
VANDERBURGH COUNTY**





**REMOVE CROSSOVER AT RADIO AVE.  
U.S. 41 AT HILLSDALE ROAD  
VANDERBURGH COUNTY**

## **APPENDIX D**

Crash Analysis for US 41 and Hillsdale Rd, Evansville, Vanderburgh County  
 1/1/2016-12/31/2018

<b>1.) SEVERITY</b>	<b>2016</b>	<b>2017</b>	<b>2018</b>	<b>TOTALS</b>	
a. Property Damage .....	2	4	3	9	56%
b. Non-Incapacitating Injury .....	0	1	0	1	6%
c. Incapacitating Injury .....	0	2	3	5	31%
d. Fatality .....	1	0	0	1	6%
e. Unknown .....	0	0	0	0	0%
	---	---	---	---	100%
<b>TOTALS</b>	<b>3</b>	<b>7</b>	<b>6</b>	<b>16</b>	

<b>2.) COLLISION DIAGRAM</b>	<b>2016</b>	<b>2017</b>	<b>2018</b>	<b>TOTALS</b>	
a. Rear End Collision .....	0	1	0	1	6%
b. Head-On Collision .....	0	0	0	0	0%
c. Sideswipe .....	0	1	0	1	6%
d. Off Road Collision .....	0	0	2	2	13%
e. Right Angle Collision .....	1	4	4	9	56%
f. Lt. Turn Collision .....	2	1	0	3	19%
g. Rt. Turn Collision .....	0	0	0	0	0%
h. Animal .....	0	0	0	0	0%
i. Backing .....	0	0	0	0	0%
j. Object in Roadway.....	0	0	0	0	0%
k. Unknown.....	0	0	0	0	0%
	---	---	---	---	100%
<b>TOTALS</b>	<b>3</b>	<b>7</b>	<b>6</b>	<b>16</b>	

<b>3.) SURFACE CONDITION</b>	<b>2016</b>	<b>2017</b>	<b>2018</b>	<b>TOTALS</b>	
a. Dry .....	3	6	4	13	81%
b. Wet .....	0	1	2	3	19%

c. Snow/Ice/Slush .....	0	0	0	0	0%
d. Other.....	0	0	0	0	0%
e. Unknown.....	0	0	0	0	0%
	---	---	---	---	100%
<b>TOTALS</b>	<b>3</b>	<b>7</b>	<b>6</b>	<b>16</b>	

**4.) CONTRIBUTING**

<b>CIRCUMSTANCE</b>	<b>2016</b>	<b>2017</b>	<b>2018</b>	<b>TOTALS</b>	
a. Unsafe Speed .....	0	0	0	0	0%
b. Disregard Signal .....	0	0	0	0	0%
c. Failure to Yield R/W .....	2	4	4	10	63%
d. Brake Failure or Defective .....	0	0	0	0	0%
e. Following Too Closely .....	0	1	0	1	6%
f. Animal/Object in Roadway .....	0	0	0	0	0%
g. Cell Phone Usage .....	0	0	0	0	0%
h. Left of Center .....	0	0	0	0	0%
i. Distracted .....	0	0	0	0	0%
j. Improper Lane Change .....	0	0	0	0	0%
k. Alcohol .....	0	0	0	0	0%
l. Fatigue/Sleep.....	0	0	0	0	0%
m. Overcorrect / Oversteer .....	0	1	1	2	13%
n. Improper Turning .....	1	1	0	2	13%
o Other .....	0	0	1	1	6%
	---	---	---	---	100%
<b>TOTALS</b>	<b>3</b>	<b>7</b>	<b>6</b>	<b>16</b>	

**5.) MONTH**

	<b>2016</b>	<b>2017</b>	<b>2018</b>	<b>TOTALS</b>	
a. January (01).....	0	0	2	2	13%
b. February (02) .....	0	0	0	0	0%
c. March (03) .....	0	1	0	1	6%



d. April (04).....	0	2	1	3	19%
e. May (05).....	0	0	0	0	0%
f. June (06).....	2	1	1	4	25%
g. July (07).....	0	0	0	0	0%
h. August (08).....	1	1	0	2	13%
i. September (09).....	0	0	2	2	13%
j. October (10).....	0	0	0	0	0%
k. November (11).....	0	1	0	1	6%
l. December (12).....	0	1	0	1	6%
m. Unknown.....	0	0	0	0	0%
	---	---	---	---	100%
<b>TOTALS</b>	<b>3</b>	<b>7</b>	<b>6</b>	<b>16</b>	

6.) <b>DAY OF WEEK</b>	<b>2016</b>	<b>2017</b>	<b>2018</b>	<b>TOTALS</b>	
a. Sunday.....	1	2	1	4	25%
b. Monday.....	1	2	2	5	31%
c. Tuesday.....	0	0	0	0	0%
d. Wednesday.....	0	0	2	2	13%
e. Thursday.....	1	1	0	2	13%
f. Friday.....	0	2	0	2	13%
g. Saturday.....	0	0	1	1	6%
h. Unknown.....	0	0	0	0	0%
	---	---	---	---	100%
<b>TOTALS</b>	<b>3</b>	<b>7</b>	<b>6</b>	<b>16</b>	

7.) <b>HOUR OF DAY</b>	<b>2016</b>	<b>2017</b>	<b>2018</b>	<b>TOTALS</b>	
a. 6-7AM (0600).....	0	0	1	1	6%
b. 7-8AM (0700).....	0	0	1	1	6%

c. 8-9AM (0800).....	1	0	0	1	6%
d. 9-10AM (0900).....	0	0	1	1	6%
e. 10-11AM (1000).....	0	2	1	3	19%
f. 11A-12N (1100).....	0	0	0	0	0%
g. 12N-1P (1200).....	0	0	0	0	0%
h. 1-2PM (1300).....	0	1	1	2	13%
i. 2-3PM (1400).....	0	1	0	1	6%
j. 3-4PM (1500).....	0	2	1	3	19%
k. 4-5PM (1600).....	1	0	0	1	6%
l. 5-6PM (1700).....	1	0	0	1	6%
	---	---	---	---	94%
<b>Daylight Sub-Totals</b>	<b>3</b>	<b>6</b>	<b>6</b>	<b>15</b>	
m. 6-7PM (1800).....	0	1	0	1	6%
n. 7-8PM (1900).....	0	0	0	0	0%
o. 8-9PM (2000).....	0	0	0	0	0%
p. 9-10PM (2100).....	0	0	0	0	0%
q. 10-11PM (2200).....	0	0	0	0	0%
r. 11P-12M (2300).....	0	0	0	0	0%
s. 12M-1A (2400).....	0	0	0	0	0%
t. 1-2AM (0100).....	0	0	0	0	0%
u. 2-3AM (0200).....	0	0	0	0	0%
v. 3-4AM (0300).....	0	0	0	0	0%
w. 4-5AM (0400).....	0	0	0	0	0%
x. 5-6AM (0500).....	0	0	0	0	0%
y. Unknown.....	0	0	0	0	0%
	---	---	---	---	6%
<b>Nighttime Sub-Totals</b>	<b>0</b>	<b>1</b>	<b>0</b>	<b>1</b>	
	---	---	---	---	100%
<b>GRAND TOTALS</b>	<b>3</b>	<b>7</b>	<b>6</b>	<b>16</b>	

<b>8.) ROAD CHARACTER</b>	<b>2016</b>	<b>2017</b>	<b>2018</b>	<b>TOTALS</b>	
a. Straight/Level .....	3	7	6	16	100%
b. Straight/Grade .....	0	0	0	0	0%
c. Straight/Hillcrest .....	0	0	0	0	0%
d. Curve/Level .....	0	0	0	0	0%
e. Curve/Grade .....	0	0	0	0	0%
f. Curve/Hillcrest .....	0	0	0	0	0%
g. Unknown.....	0	0	0	0	0%
	---	---	---	---	100%
<b>TOTALS</b>	<b>3</b>	<b>7</b>	<b>6</b>	<b>16</b>	

<b>9.) DIRECTIONAL ANALYSIS / COLLISION INVOLVED</b>	<b>2016</b>	<b>2017</b>	<b>2018</b>	<b>TOTALS</b>	
a. WB w/ SBlt .....	0	0	0	0	
b. EB w/ NB .....	0	0	1	1	6%
c. WB w/ SB .....	0	0	2	2	13%
d. WBlt w/ SB .....	0	0	0	0	0%
e. SB w/ SB .....	0	0	0	0	0%
f. NB w/ Embankment, Ditch .....	0	0	1	1	6%
g. NB w/ WB .....	0	2	1	3	19%
h. NB U-Turn w/ SB .....	0	0	0	0	0%
i. EB w/ WBlt .....	0	0	0	0	0%
j. SB w/ Deer/Rock .....	0	0	0	0	0%
k. NB w/ Deer/Coyote .....	0	0	0	0	0%
l. NB w/ SBlt .....	0	0	0	0	0%
m. SB w/ Embankment, Ditch .....	0	0	1	1	6%
n. WB w/ WB .....	0	1	0	1	6%
o. WBrt w/ SB .....	0	0	0	0	0%
p. SB U-Turn w/ NBlt .....	0	0	0	0	0%

q. WB w/ NB .....	0	1	0	1	6%
r. WBlt w/SBlt .....	0	0	0	0	0%
s. WBlt w/EB .....	1	1	0	2	13%
t. SBlt w/ NB.....	1	0	0	1	6%
u. SB w/WB .....	1	1	0	2	13%
v. NB w/ NB .....	0	1	0	1	6%
	---	---	---	---	
<b>TOTALS</b>	<b>3</b>	<b>7</b>	<b>6</b>	<b>16</b>	<b>100%</b>

<b>10.) REFERENCED LOCATIONS</b>	<b>2016</b>	<b>2017</b>	<b>2018</b>	<b>TOTALS</b>	
a. At Intersection .....	2	6	3	11	69%
b. Zero to Hundred Feet East .....	0	0	0	0	0%
c. Zero to Hundred Feet North .....	0	0	2	2	13%
d. Zero to Hundred Feet South .....	0	1	0	1	6%
e. Zero to Hundred Feet West .....	1	0	1	2	13%
f. Hundred to Thousand Feet East .....	0	0	0	0	0%
g. Hundred to Thousand Feet North .....	0	0	0	0	0%
h. Hundred to Thousand Feet South .....	0	0	0	0	0%
i. Hundred to Thousand Feet West .....	0	0	0	0	0%
j. Between 1,000 and 1,500 Feet	0	0	0	0	0%
	---	---	---	---	100%
<b>TOTALS</b>	<b>3</b>	<b>7</b>	<b>6</b>	<b>16</b>	

Index of Crash Frequency and Cost - Form F1		Page 1/2
Location	Hillsdale Road	
	U.S. 41	
GIS		
Post		
Analyst		
Date	4/18/2019	
<b>INPUT</b>		
Road Facility Type	Unsignalized Rural State-Local Intersection	
Major Road AADT (veh/day)	24500	
T-intersection Indicator (1 if present, 0 otherwise)	0	
First Year with Crash Data (yyyy)	2016	
Last Year with Crash Data (yyyy)	2018	
Number of Crashes (crash/period)		
Fatal and Incapacitating Injury Crashes	7	
Non-Incapacitating and Possible Injury Crashes	2	
Property Damage Only Crashes	16	
Route or Road Type	Unsignalized Rural State-State Intersection	
Average Crash Costs (\$)		
Fatal and Incapacitating Injury Crashes	525500	
Non-Incapacitating and Possible Injury Crashes	33100	
Property Damage Only Crashes	5600	
Crash Cost Year (yyyy)	2013	
<b>OUTPUT</b>		
Expected Crash Frequency (crash/year)		
Fatal and Incapacitating Injury Crashes	0.084	
Non-Incapacitating and Possible Injury Crashes	0.51	
Property Damage Only Crashes	1.49	
All Crashes	2.08	
Index of Crash Frequency	<b>2.58</b>	
Index of Crash Cost	<b>2.56</b>	

Index of Crash Frequency and Cost - Form F1		Page 2/2
Location	Hillsdale Road	
	U.S. 41	
GIS		
Post		
Analyst		
Date	4/18/2019	
<b>Comments:</b> w/ deer strikes		

<b>Benefit Cost Analysis - Form F5</b>		Page 1/2
Location	Hillsdale Road	
	U.S. 41	
GIS		
Post		
Analyst		
Date	4/18/2019	
<b>INPUT</b>		
Number of Crashes (crash/period)		
Fatal and Incapacitating Injury Crashes		7
Non-Incapacitating and Possible Injury Crashes		2
Property Damage Only Crashes		16
<b>Countermeasure 1</b>		
Countermeasure Description	J-Turn	
Number of Target Crashes (crash/period)		
Fatal and Incapacitating Injury Crashes		4
Non-Incapacitating and Possible Injury Crashes		5
Property Damage Only Crashes		16
Crash Reduction Factors (%)		
Fatal and Incapacitating Injury Crashes		50
Non-Incapacitating and Possible Injury Crashes		50
Property Damage Only Crashes		50
Countermeasure Service Life (years)		20
Countermeasure Capital Cost (\$)		500000
Countermeasure Annual Maintenance Cost (\$)		10000
Countermeasure Salvage Value (\$)		100000
Route Type	Unsignalized Rural State-State Intersection	
Average Crash Costs (\$)		
Fatal and Incapacitating Injury Crashes		525500
Non-Incapacitating and Possible Injury Crashes		33100
Property Damage Only Crashes		5600
Crash Cost Year (yyyy)		2013
First Year with Crash Data (yyyy)		2016
Last Year with Crash Data (yyyy)		2018

<b>Benefit Cost Analysis - Form F5</b>		Page 2/2
Location	Hillsdale Road	
	U.S. 41	
GIS		
Post		
Analyst		

Date	4/18/2019
<b>Expected Crash Frequency (crash/year)</b>	
Fatal and Incapacitating Injury Crashes	0.084
Non-Incapacitating and Possible Injury Crashes	0.51
Property Damage Only Crashes	1.49
All Crashes	2.08
Present Year (yyyy)	2019
Implementation Year (yyyy)	2021
Inflation Rate (%)	2
Interest Rate (%)	4
Annual Traffic Growth Rate (%)	2
Total Traffic Growth Factor (%)	1.1
Future Traffic Growth Period (years)	20
<b>OUTPUT</b>	
Equivalent Annual Benefit (\$)	58,837
Equivalent Annual Cost (\$)	40,156
Present Worth Benefit (\$)	1,470,914
Present Worth Cost (\$)	1,003,899
Net Annual Benefit (\$)	<b>18,681</b>
Present Worth Net Benefit (\$)	<b>467,014</b>
Benefit-Cost Ratio	<b>1.47</b>
<b>Comments:</b> w/ deer strikes	

## **APPENDIX E**



**Intersection Modification Impacts  
Local Road Network**

August 2019

LOS Comparison for Modification Scenarios fat Network Intersections (2020 & 2045 AM Peak Hour - Critical Movements)													
Intersection	AM	No-Build (No Modifications)				With Radio Avenue R-in / R-out				With Hillsdale Rd. (west) & Radio Avenue R-in / R-out, and No South MUT			
		NB	SB	WB	EB	NB	SB	WB	EB	NB	SB	WB	EB
Hillsdale / US 41 J-Turn	2020	B	A	B	B	B	A	B	B	~	A	B	B
	2045	B	B	C	C	B	B	C	C				
Hillsdale / US 41 TWSC	2020	B	A	F	E	B	A	F	F				
	2045	B	B	^	F								
*Hillsdale / US 41 (EB removed) TWSC	2020	B	A	E	x								
	2045												
Radio / US 41 OWSC	2020	~	A	D	x	~	~	B	x				
	2045	~	B	E	x	~	~	B	x				
Old State Rd-Campbell / US 41 TWSC	2020	B	B	F	F	B	B	F	F	B	B	F	**F
	2045	B	B	^	F	B	B	^	^				
Campbell / Walnut TWSC	2020	A	x	A	~	A	x	A	~				
	2045	A	x	A	~	A	x	A	~				
Hillsdale / Walnut TWSC	2020	A	B	A	A	A	A	A	A				
	2045	B	B	A	A	B	B	A	A				
Hillsdale / Old State TWSC	2020	~	A	A	x	~	A	A	x				
	2045	~	A	B	x	~	A	B	x				
Radio / Walnut TWSC	2020	A	A	A	A	A	A	A	A				
	2045	A	A	A	A	A	A	A	A				

\*Tested once to compare removing the west approach under the TWSC condition. No significant difference in operations

\*\*A dedicated left turn lane was added at west approach for this computation. Does not improve LOS for critical WB or EB movements.

~Non-critical

^not computable

## **APPENDIX F**

US 41 at Hillsdale Road  
Intersection Performance Summary Table

August 2019

2020 (Radio R-in/R-out) - Delay: AM (PM)					
Type	EB	WB	NB	SB	ALL
TWSC	51.9 (317.0)	302.4 (1301.9)			
Signal	31.5 (22.0)	26.7 (23.2)	13.5 (7.2)	14.5 (7.4)	15.5 (8.3)
J-Turn (Main)	12.4 (13.2)	12.5 (14.9)			
J-Turn (north)		14.4 (14.9)			
J-Turn (south)	11.6 (14.2)				

2020 (Radio R-in/R-out) - LOS: AM (PM)					
Type	EB	WB	NB	SB	ALL
TWSC	F (F)	F (F)			
Signal	C (C)	C (C)	B (A)	B (A)	B (A)
J-Turn (Main)	B (B)	B (B)			
J-Turn (north)		B (B)			
J-Turn (south)	B (B)				

2045 (Radio R-in/R-out) - Delay: AM (PM)					
Type	EB	WB	NB	SB	ALL
TWSC	823.9 (432.8)	^ (^)			
Signal	20.3 (15.7)	23.9 (16.9)	4.0 (5.1)	4.4 (5.3)	6.2 (5.9)
J-Turn (Main)	14.3 (15.6)	15.9 (23.7)			
J-Turn (north)		18.6 (19.0)			
J-Turn (south)	12.8 (17.0)				

^ Not computable

2045 (Radio R-in/R-out) - LOS: AM (PM)					
Type	EB	WB	NB	SB	ALL
TWSC	F (F)	F+ (F+)			
Signal	C (B)	C (B)	A (A)	A (A)	A (A)
J-Turn (Main)	B (C)	C (C)			
J-Turn (north)		C (C)			
J-Turn (south)	B (C)				

US 41 at Hillsdale Road  
Intersection Performance Summary Table

August 2019

<b>COMPARE R-CUT TO J-TURN &amp; CONVENTIONAL SIGNAL</b>
--

Test the AM WB movement (heaviest conflicting directional movement)

2020 R-CUT - Delay (AM)					
Type	EB	WB	NB	SB	ALL
Stop (Main)		12.5			
*Travel1			60		
Signal (median)		27.1			
**Travel2				60	
Total Time					159.6
	Total WB =	39.6			

2020 J-Turn - Delay (AM)					
Type	EB	WB	NB	SB	ALL
Stop (Main)		12.5			
*Travel1			60		
Stop (median)		14.4			
**Travel2				60	
Total Time					146.9
	Total WB =	26.9			

Signal Intersection WB =	26.7	delay at approach
J-Turn WB =	26.9	delay at approaches
R-CUT WB =	39.6	delay at approaches

**R-CUT provides no added benefit for the critical movement (WB Hillsdale Road)**

\*Travel1 = estimated time from main intersection stop to median turn

\*\*Travel2 = estimated time from median turn to origination (in opposing lane)

## **APPENDIX G**

**US 41 / Hillsdale Road Intersection Improvements  
Preliminary Construction Cost Estimate - Engineering Assessment Stage**

Clearing Right-of-Way	LS	1	\$ 20,000.00	\$ 20,000.00
Field Office	MOS	12	\$ 1,759.13	\$ 21,109.56
Excavation, Common	CYS	7300	\$ 16.98	\$ 123,954.00
Erosion Control & Temp Seeding	LS	1	\$ 50,000.00	\$ 50,000.00
Borrow	CYS	1800	\$ 11.00	\$ 19,800.00
Underdrains, 6"	LFT	6000	\$ 5.76	\$ 34,560.00
Aggregate for Underdrains	CYS	325	\$ 44.31	\$ 14,400.75
Geotextiles for Underdrains	SYS	1300	\$ 3.00	\$ 3,900.00
Subgrade Treatment	SYS	7500	\$ 4.80	\$ 36,000.00
Structure Backfill	CYS	100	\$ 35.00	\$ 3,500.00
Compacted Aggregate	TON	200	\$ 40.00	\$ 8,000.00
Milling, Asphalt, 2.5 IN	SYS	15000	\$ 1.65	\$ 24,750.00
QC/QA-HMA Surface, 9.5 mm	TON	1300	\$ 65.00	\$ 84,500.00
QC/QA-HMA Intermediate, 12.5 mm	TON	870	\$ 65.00	\$ 56,550.00
QC/QA-HMA Base, 19.0 mm	TON	4900	\$ 68.00	\$ 333,200.00
Prime Coat	TON	8	\$ 531.57	\$ 4,252.56
Tack Coat	TON	5	\$ 526.65	\$ 2,633.25
HMA Wedge & Level	TON	175	\$ 85.00	\$ 14,875.00
Guardrail, W-Beam	LFT	1400	\$ 21.26	\$ 29,764.00
Guardrail Transition, Type TGB	EACH	2	\$ 2,485.23	\$ 4,970.46
Guardrail End Section, Type OS	EACH	2	\$ 2,924.58	\$ 5,849.16
Pipe, 15"	LFT	400	\$ 45.22	\$ 18,088.00
Pipe, 48" RCP (Extend)	Lft	40	\$ 205.00	\$ 8,200.00
Inlet, w/ Safety Grate (Median)	EACH	4	\$ 2,058.72	\$ 8,234.88
Pipe Anchor, 48"	EACH	2	\$ 2,000.00	\$ 4,000.00
Maintaining Traffic	LS	1	\$ 200,000.00	\$ 200,000.00
Signs	LS	1	\$ 175,000.00	\$ 175,000.00
Lighting	LS	1	\$ 150,000.00	\$ 150,000.00
Pavement Markings	LS	1	\$ 10,000.00	\$ 10,000.00
Directional Bore (15" pipe)	LFT	150	\$ 100.00	\$ 15,000.00
Construction Engineering	LS	1	\$ 29,400.00	\$ 29,400.00
Mobilization and Demobilization	LS	1	\$ 75,700.00	\$ 75,700.00

2018 unit price basis = \$ 1,590,191.62

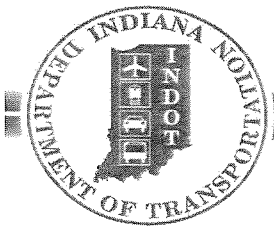
2018 w/ 20% contingency = \$ 1,908,230.00

2019 (4% inflation) = \$ 1,984,560.00

2020 (4% inflation) = \$ 2,063,940.00

2021 (4% inflation) = \$ 2,146,500.00

## **APPENDIX H**



## Report for IHCP Queuing Analysis

Prepared by: **Shawn Strange**  
Prepared for: **INDOT**

Position: **Engineer**  
Position: **District Traffic**

### Work Zone Information

---

Route: US 41  
Mile marker: MM XXX.XX  
Positive Direction: North  
  
Contract: X-XXXXX  
Work Year: 2021  
Work Month: JUN  
Work Type: Construction  
Work Description: Intersection J Turn

Permanent Speed Limit: 50 mph  
Work Zone Speed Limit: 50 mph  
Workzone Lane Width: 11 ft

Channelizing Devices: Typically Drums, Cones, or Barrier Wall.

Additional Information: Lane Drop for Roadside Work

### Queuing Formula

---

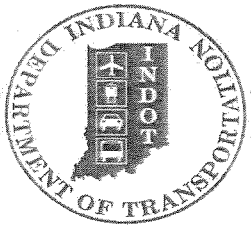
$$L = L_0 + \frac{(V - C) * 1 \text{ hr}}{(k * N)}$$

Where:

- L= Length of Queue for Specified Hour (mi)
- L<sub>0</sub>= Length of Queue in the Hour Prior to Specified Hour (mi)
- V= Hourly Volume (PCE/hr)
- C= Hourly Capacity (PCE/hr)
- k= Jam Density (PCE/mi/lane)
- N= Number of Storage Lanes Upstream of Restriction (lanes)

Jam Density (k)= 190 PCE/mi/ln  
N for SB= 1 ln  
N for NB= 1 ln





## Traffic Information

### Count Information

Count Station Number:	820260
Month, Year of Count:	SEP, 2018
Rural/Urban:	Rural

### Factors to Adjust Counts to Annual Value in Current Year

Annual Adj. Factor:	1.01
Monthly Adj. Factor:	0.98

### Factors to Project Annual Value in Current Year to Work Month and Year

Annual Adj. Factor:	1.020
Monthly Adj. Factor:	0.906

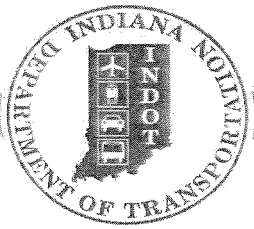
### Assumed Percent Reduction of Traffic Due to Detouring

#### Southbound

Monday-Thursday:	0%
Friday:	0%
Saturday-Sunday:	0%

#### Northbound

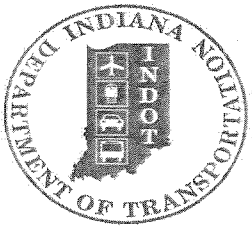
Monday-Thursday:	0%
Friday:	0%
Saturday-Sunday:	0%



## Traffic Volumes

Time	Southbound Volumes (PCE/hr)				Northbound Volumes (PCE/hr)			
	Weekday	Friday	Saturday	Sunday	Weekday	Friday	Saturday	Sunday
Midnight to 1 am	138	138	138	138	116	116	116	116
1 am to 2 am	159	159	159	159	120	120	120	120
2 am to 3 am	109	109	109	109	136	136	136	136
3 am to 4 am	134	134	134	134	111	111	111	111
4 am to 5 am	303	303	303	303	292	292	292	292
5 am to 6 am	812	812	812	812	937	937	937	937
6 am to 7 am	1306	1306	1306	1306	1011	1011	1011	1011
7 am to 8 am	1586	1586	1586	1586	977	977	977	977
8 am to 9 am	1088	1088	1088	1088	805	805	805	805
9 am to 10 am	946	946	946	946	737	737	737	737
10 am to 11 am	969	969	969	969	797	797	797	797
11 am to Noon	889	889	889	889	871	871	871	871
Noon to 1 pm	1021	1021	1021	1021	904	904	904	904
1 pm to 2 pm	974	974	974	974	888	888	888	888
2 pm to 3 pm	1098	1098	1098	1098	1001	1001	1001	1001
3 pm to 4 pm	1138	1138	1138	1138	1196	1196	1196	1196
4 pm to 5 pm	1132	1132	1132	1132	1201	1201	1201	1201
5 pm to 6 pm	1293	1293	1293	1293	1429	1429	1429	1429
6 pm to 7 pm	841	841	841	841	1058	1058	1058	1058
7 pm to 8 pm	742	742	742	742	642	642	642	642
8 pm to 9 pm	457	457	457	457	538	538	538	538
9 pm to 10 pm	336	336	336	336	405	405	405	405
10 pm to 11 pm	267	267	267	267	218	218	218	218
11 pm to Midnight	197	197	197	197	164	164	164	164

Note: All Traffic Volumes are in PCEs/hr as defined in the IHCP.



## Summary of Alternatives

### Southbound

Alternative 1 (Lane Drop for Daytime Work):

Within Policy Limits

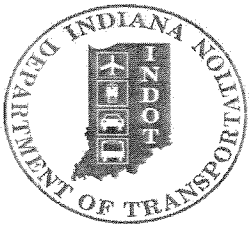
### Northbound

Alternative 1 (Lane Drop for Daytime Work):

Within Policy Limits

### Policy Limits Criteria:

- i) No queues of any length for > 6 continuous hours or 12 hours in a calendar day
- ii) No queues >0.5 mi for > 4 continuous hours
- iii) No queues >1.0 mi for > 2 continuous hours
- iv) No queues >1.5 mi



# Alternative 1

## Capacities for Southbound Traffic (PCE/hr)

Time	Monday	Tuesday	Wednesday	Thursday	Friday	Saturday	Sunday
Midnight to 1 am	1550	1550	1550	1550	1550	1550	1550
1 am to 2 am	1550	1550	1550	1550	1550	1550	1550
2 am to 3 am	1550	1550	1550	1550	1550	1550	1550
3 am to 4 am	1550	1550	1550	1550	1550	1550	1550
4 am to 5 am	1550	1550	1550	1550	1550	1550	1550
5 am to 6 am	1550	1550	1550	1550	1550	1550	1550
6 am to 7 am	1550	1550	1550	1550	1550	1550	1550
7 am to 8 am	1550	1550	1550	1550	1550	1550	1550
8 am to 9 am	1550	1550	1550	1550	1550	1550	1550
9 am to 10 am	1550	1550	1550	1550	1550	1550	1550
10 am to 11 am	1550	1550	1550	1550	1550	1550	1550
11 am to Noon	1550	1550	1550	1550	1550	1550	1550
Noon to 1 pm	1550	1550	1550	1550	1550	1550	1550
1 pm to 2 pm	1550	1550	1550	1550	1550	1550	1550
2 pm to 3 pm	1550	1550	1550	1550	1550	1550	1550
3 pm to 4 pm	1550	1550	1550	1550	1550	1550	1550
4 pm to 5 pm	1550	1550	1550	1550	1550	1550	1550
5 pm to 6 pm	1550	1550	1550	1550	1550	1550	1550
6 pm to 7 pm	1550	1550	1550	1550	1550	1550	1550
7 pm to 8 pm	1550	1550	1550	1550	1550	1550	1550
8 pm to 9 pm	1550	1550	1550	1550	1550	1550	1550
9 pm to 10 pm	1550	1550	1550	1550	1550	1550	1550
10 pm to 11 pm	1550	1550	1550	1550	1550	1550	1550
11 pm to Midnight	1550	1550	1550	1550	1550	1550	1550

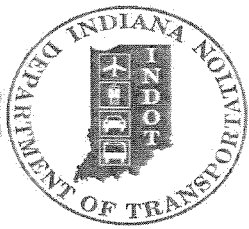
Legend:



Values that are less than the Recommended Non-work Capacity

Summary:

Recommended Non-work Capacity = 2075  
 Number of Hours per week of restrictions = 168 hr(s)



# Alternative 1

## Capacities for Northbound Traffic (PCE/hr)

Time	Monday	Tuesday	Wednesday	Thursday	Friday	Saturday	Sunday
Midnight to 1 am	1550	1550	1550	1550	1550	1550	1550
1 am to 2 am	1550	1550	1550	1550	1550	1550	1550
2 am to 3 am	1550	1550	1550	1550	1550	1550	1550
3 am to 4 am	1550	1550	1550	1550	1550	1550	1550
4 am to 5 am	1550	1550	1550	1550	1550	1550	1550
5 am to 6 am	1550	1550	1550	1550	1550	1550	1550
6 am to 7 am	1550	1550	1550	1550	1550	1550	1550
7 am to 8 am	1550	1550	1550	1550	1550	1550	1550
8 am to 9 am	1550	1550	1550	1550	1550	1550	1550
9 am to 10 am	1550	1550	1550	1550	1550	1550	1550
10 am to 11 am	1550	1550	1550	1550	1550	1550	1550
11 am to Noon	1550	1550	1550	1550	1550	1550	1550
Noon to 1 pm	1550	1550	1550	1550	1550	1550	1550
1 pm to 2 pm	1550	1550	1550	1550	1550	1550	1550
2 pm to 3 pm	1550	1550	1550	1550	1550	1550	1550
3 pm to 4 pm	1550	1550	1550	1550	1550	1550	1550
4 pm to 5 pm	1550	1550	1550	1550	1550	1550	1550
5 pm to 6 pm	1550	1550	1550	1550	1550	1550	1550
6 pm to 7 pm	1550	1550	1550	1550	1550	1550	1550
7 pm to 8 pm	1550	1550	1550	1550	1550	1550	1550
8 pm to 9 pm	1550	1550	1550	1550	1550	1550	1550
9 pm to 10 pm	1550	1550	1550	1550	1550	1550	1550
10 pm to 11 pm	1550	1550	1550	1550	1550	1550	1550
11 pm to Midnight	1550	1550	1550	1550	1550	1550	1550

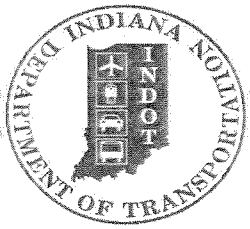
Legend:



Values that are less than the Recommended Non-work Capacity

Summary:

Recommended Non-work Capacity = 2075  
 Number of Hours per week of restrictions = 168 hr(s)

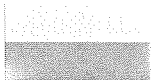


# Alternative 1

## Queuing for Southbound Traffic (mi)

Time	Monday	Tuesday	Wednesday	Thursday	Friday	Saturday	Sunday
Midnight to 1 am	0.0	0.0	0.0	0.0	0.0	0.0	0.0
1 am to 2 am	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2 am to 3 am	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3 am to 4 am	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4 am to 5 am	0.0	0.0	0.0	0.0	0.0	0.0	0.0
5 am to 6 am	0.0	0.0	0.0	0.0	0.0	0.0	0.0
6 am to 7 am	0.0	0.0	0.0	0.0	0.0	0.0	0.0
7 am to 8 am	0.2	0.2	0.2	0.2	0.2	0.2	0.2
8 am to 9 am	0.0	0.0	0.0	0.0	0.0	0.0	0.0
9 am to 10 am	0.0	0.0	0.0	0.0	0.0	0.0	0.0
10 am to 11 am	0.0	0.0	0.0	0.0	0.0	0.0	0.0
11 am to Noon	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Noon to 1 pm	0.0	0.0	0.0	0.0	0.0	0.0	0.0
1 pm to 2 pm	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2 pm to 3 pm	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3 pm to 4 pm	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4 pm to 5 pm	0.0	0.0	0.0	0.0	0.0	0.0	0.0
5 pm to 6 pm	0.0	0.0	0.0	0.0	0.0	0.0	0.0
6 pm to 7 pm	0.0	0.0	0.0	0.0	0.0	0.0	0.0
7 pm to 8 pm	0.0	0.0	0.0	0.0	0.0	0.0	0.0
8 pm to 9 pm	0.0	0.0	0.0	0.0	0.0	0.0	0.0
9 pm to 10 pm	0.0	0.0	0.0	0.0	0.0	0.0	0.0
10 pm to 11 pm	0.0	0.0	0.0	0.0	0.0	0.0	0.0
11 pm to Midnight	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Legend:

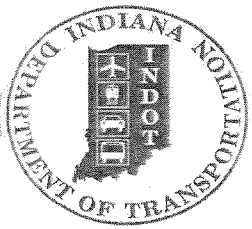


Queues greater than zero, but less than 1.5 miles

Queues greater than or equal to 1.5 miles

Summary:

Maximum Queue Length = 0.2 mi  
 Total queue length-hours = 1 mi-hrs  
 Total hours with queue = 7 hrs



# Alternative 1

## Queuing for Northbound Traffic (mi)

Time	Monday	Tuesday	Wednesday	Thursday	Friday	Saturday	Sunday
Midnight to 1 am	0.0	0.0	0.0	0.0	0.0	0.0	0.0
1 am to 2 am	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2 am to 3 am	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3 am to 4 am	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4 am to 5 am	0.0	0.0	0.0	0.0	0.0	0.0	0.0
5 am to 6 am	0.0	0.0	0.0	0.0	0.0	0.0	0.0
6 am to 7 am	0.0	0.0	0.0	0.0	0.0	0.0	0.0
7 am to 8 am	0.0	0.0	0.0	0.0	0.0	0.0	0.0
8 am to 9 am	0.0	0.0	0.0	0.0	0.0	0.0	0.0
9 am to 10 am	0.0	0.0	0.0	0.0	0.0	0.0	0.0
10 am to 11 am	0.0	0.0	0.0	0.0	0.0	0.0	0.0
11 am to Noon	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Noon to 1 pm	0.0	0.0	0.0	0.0	0.0	0.0	0.0
1 pm to 2 pm	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2 pm to 3 pm	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3 pm to 4 pm	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4 pm to 5 pm	0.0	0.0	0.0	0.0	0.0	0.0	0.0
5 pm to 6 pm	0.0	0.0	0.0	0.0	0.0	0.0	0.0
6 pm to 7 pm	0.0	0.0	0.0	0.0	0.0	0.0	0.0
7 pm to 8 pm	0.0	0.0	0.0	0.0	0.0	0.0	0.0
8 pm to 9 pm	0.0	0.0	0.0	0.0	0.0	0.0	0.0
9 pm to 10 pm	0.0	0.0	0.0	0.0	0.0	0.0	0.0
10 pm to 11 pm	0.0	0.0	0.0	0.0	0.0	0.0	0.0
11 pm to Midnight	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Legend:



Queues greater than zero, but less than 1.5 miles

Queues greater than or equal to 1.5 miles

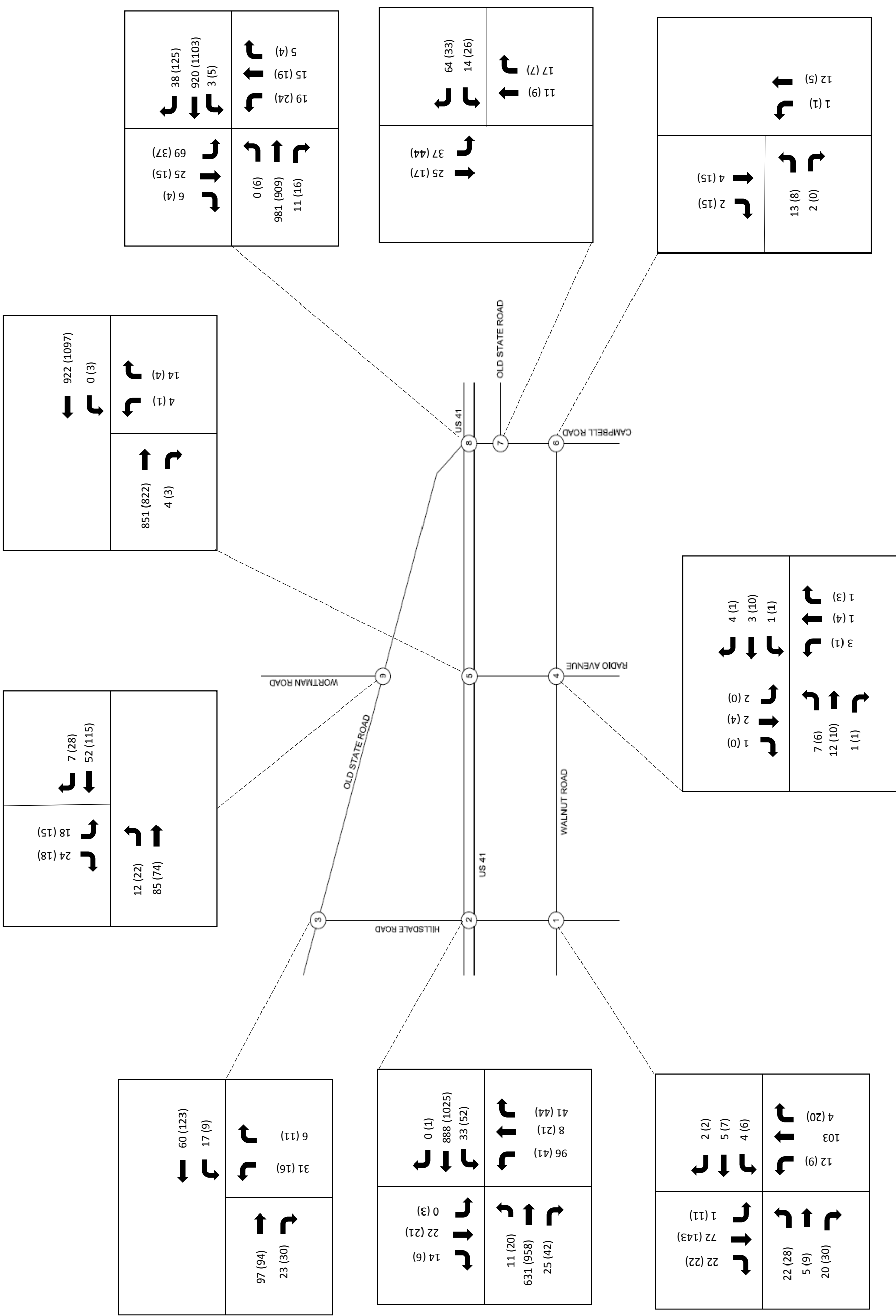
Summary:

Maximum Queue Length = 0.0 mi  
 Total queue length-hours = 0 mi-hrs  
 Total hours with queue = 0 hrs

## **APPENDIX I**

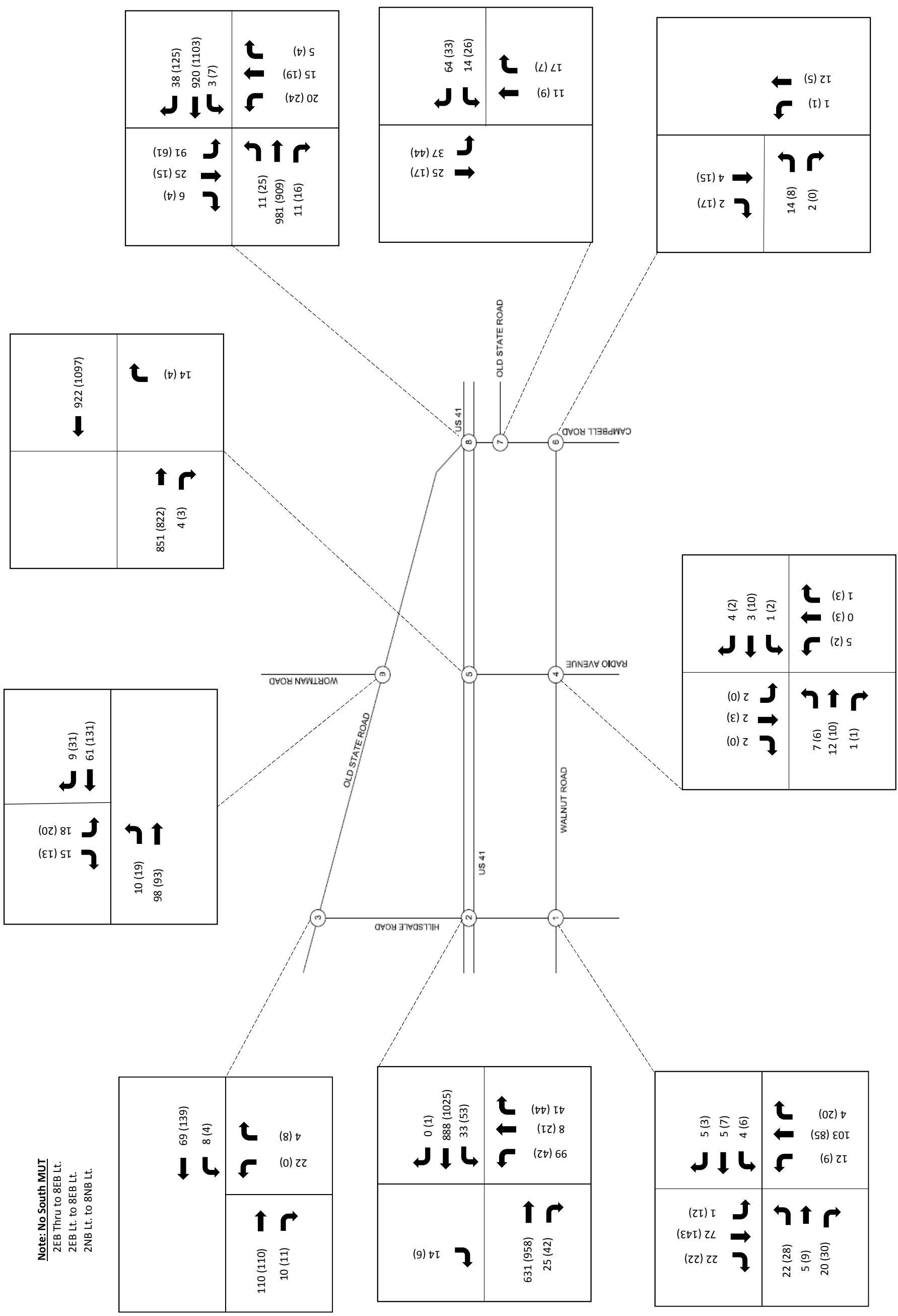


Peak Hour Traffic  
AM (PM)



Peak Hour Traffic  
AM (PM)

2020 R-in/R-out  
@ Hillsdale Rd. (west)  
@ Radio Ave.



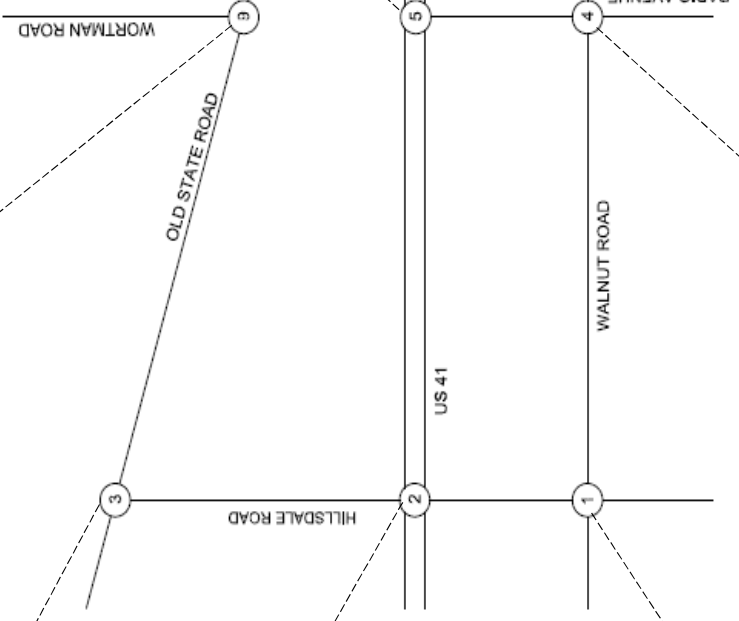
**Note: No South MUT**  
2EB Thru to 8EB Lt.  
2EB Lt. to 8EB Lt.  
2NB Lt. to 8NB Lt.

110 (110) 10 (11)	22 (0) 4 (8)	69 (139) 8 (4)
----------------------	-----------------	-------------------

631 (958) 25 (42)	99 (42) 8 (21) 41 (44)	888 (1025) 33 (53) 0 (1)
----------------------	------------------------------	--------------------------------

22 (22) 72 (143) 1 (12)	12 (9) 103 (85) 4 (20)	22 (28) 5 (9) 20 (30)
-------------------------------	------------------------------	-----------------------------

15 (13) 18 (20)	10 (19) 98 (93)	9 (31) 61 (131)
--------------------	--------------------	--------------------



851 (822) 4 (3)	922 (1097)
--------------------	------------

11 (25) 981 (909) 11 (16)	20 (24) 15 (19) 5 (4)	38 (125) 920 (1103) 3 (7)
---------------------------------	-----------------------------	---------------------------------

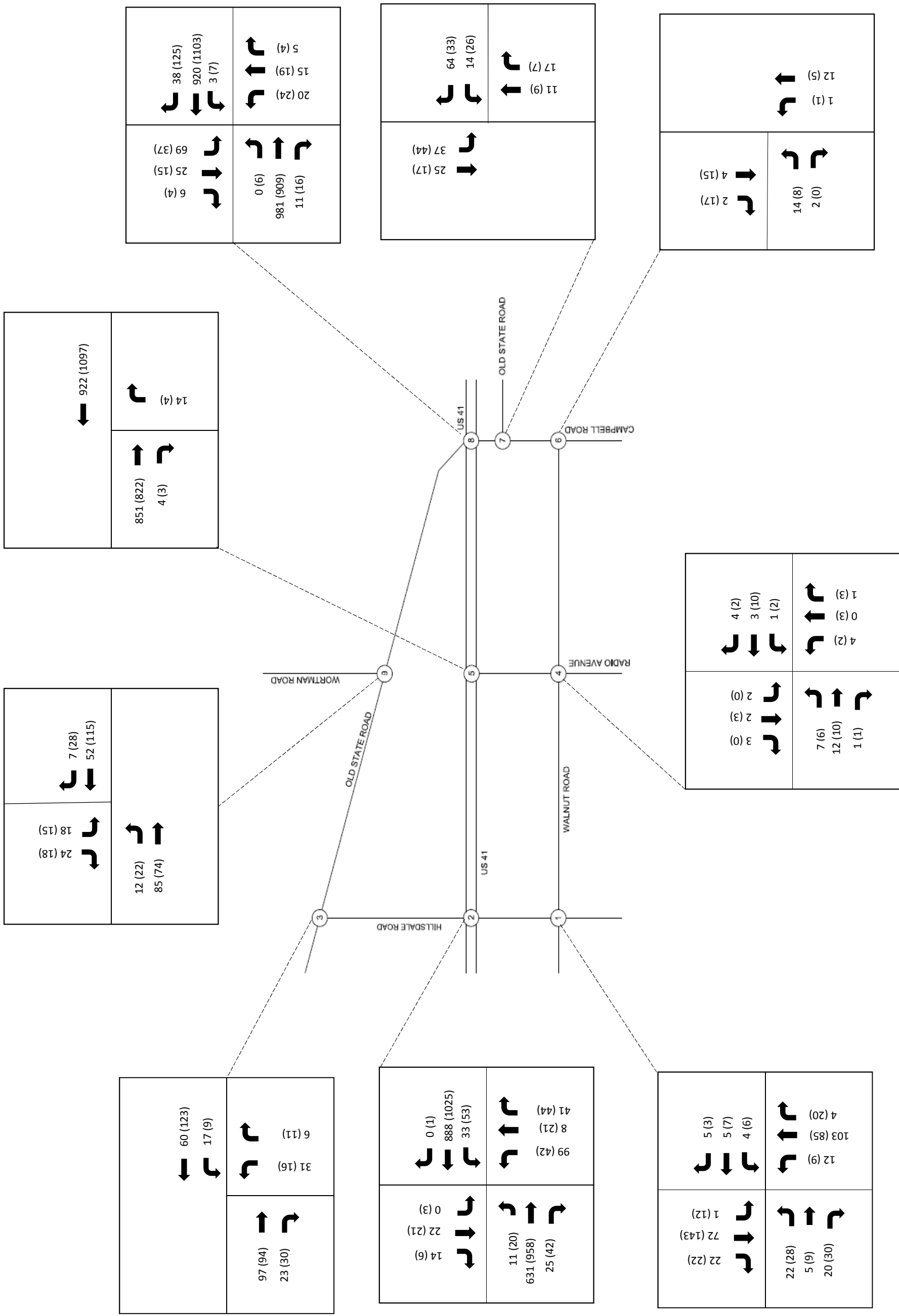
25 (17) 37 (44)	11 (9) 17 (7)	64 (33) 14 (26)
--------------------	------------------	--------------------

2 (17) 4 (15)	14 (8) 2 (0)	1 (1) 12 (5)
------------------	-----------------	-----------------

2 (0) 2 (3) 2 (0)	7 (6) 12 (10) 1 (1)	5 (2) 0 (3) 1 (3)
-------------------------	---------------------------	-------------------------

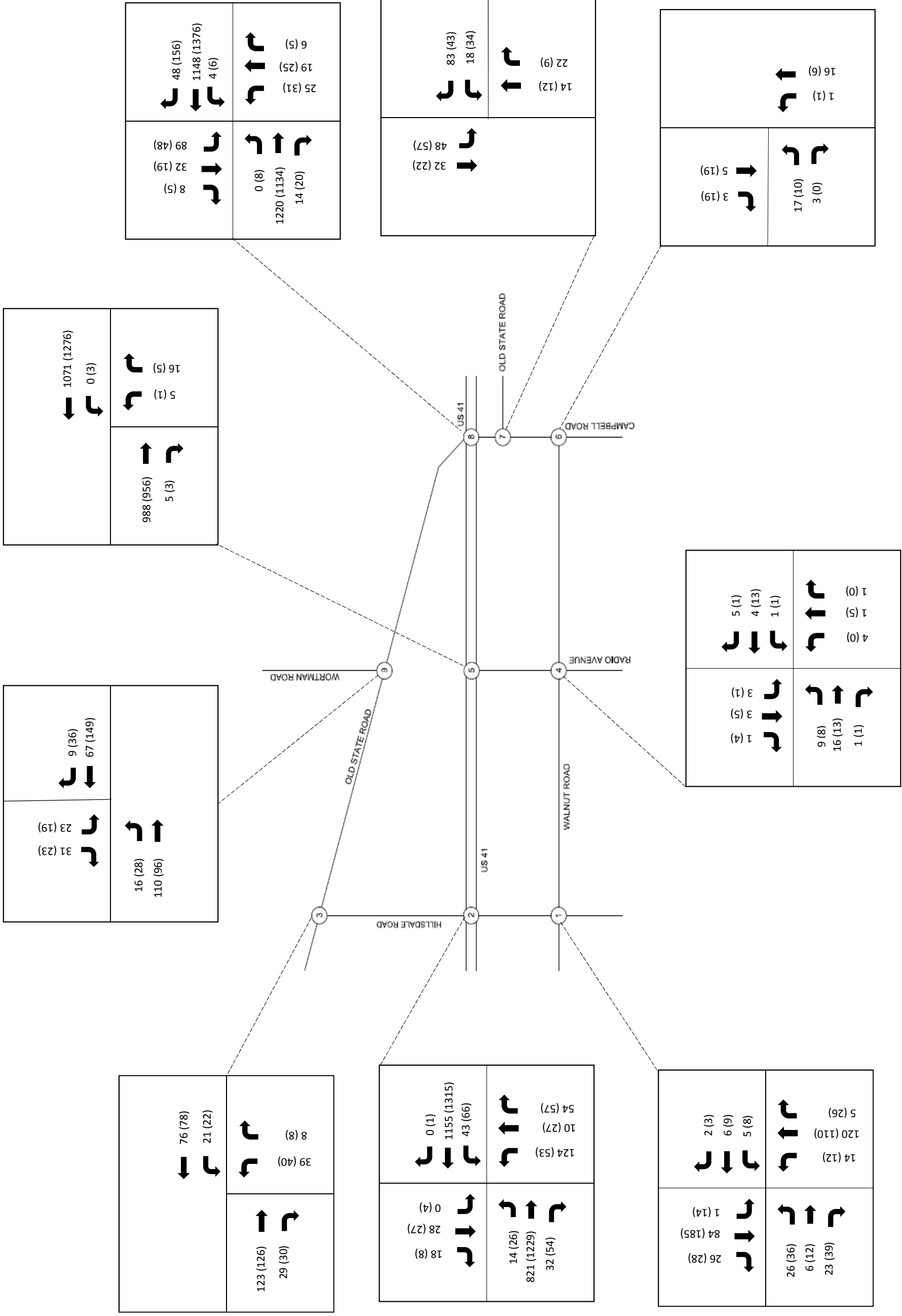
Peak Hour Traffic  
AM (PM)

2020 R-in / R-out  
@ Radio Ave.



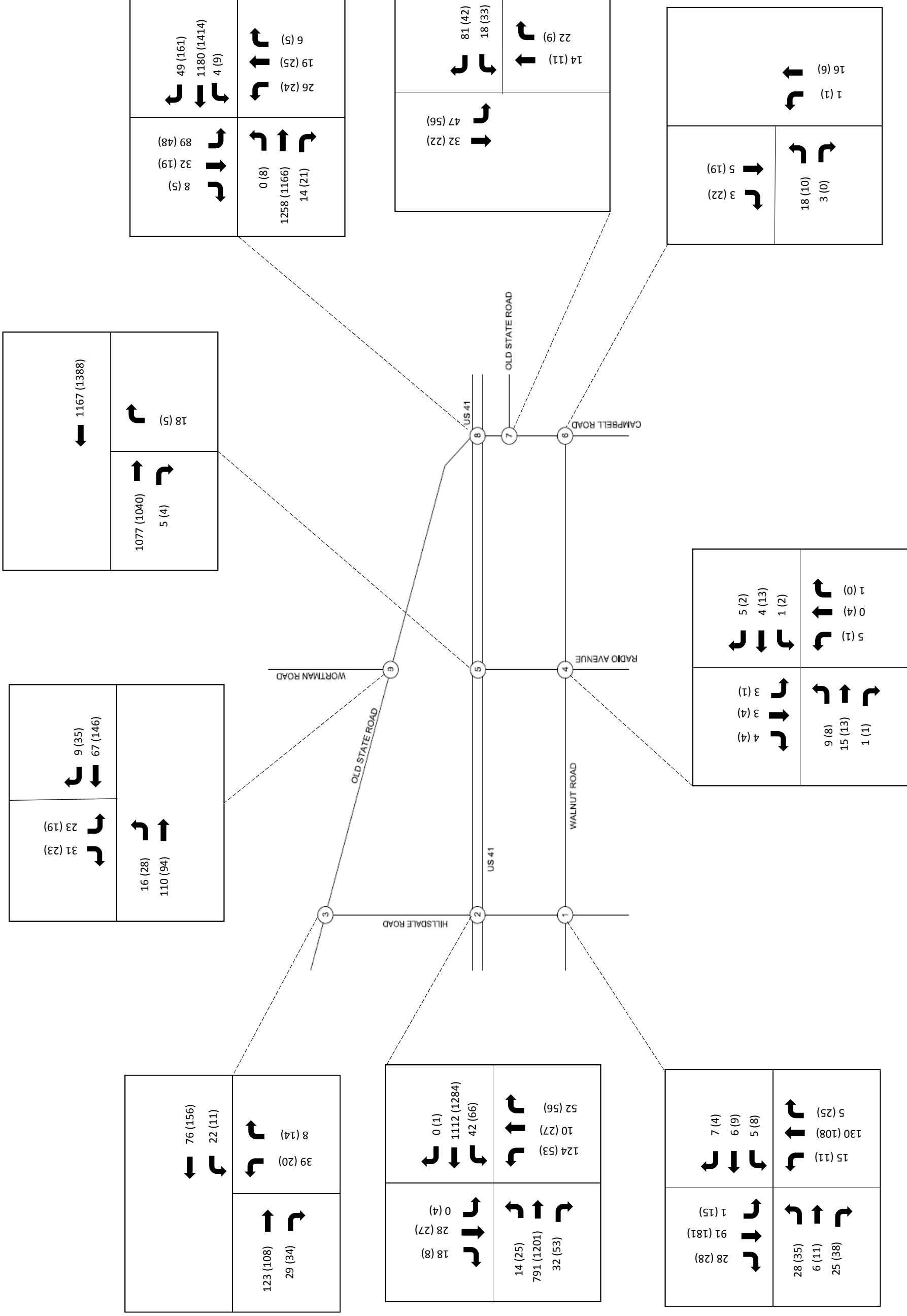
Peak Hour Traffic  
AM (PM)

2045 No Build



Peak Hour Traffic  
AM (PM)

2045 R-in / R-out  
@ Radio Ave.



## **APPENDIX J**



**PROJECT TRAFFIC FORECAST REPORT**

**DES No.:** 1400005

US-41 At Hillsdale Road, 2.04 miles N of SR 57

From RP 10+90 to RP 11+1

Vanderburgh County

**Prepared For**

Matthew Bullock

**On**

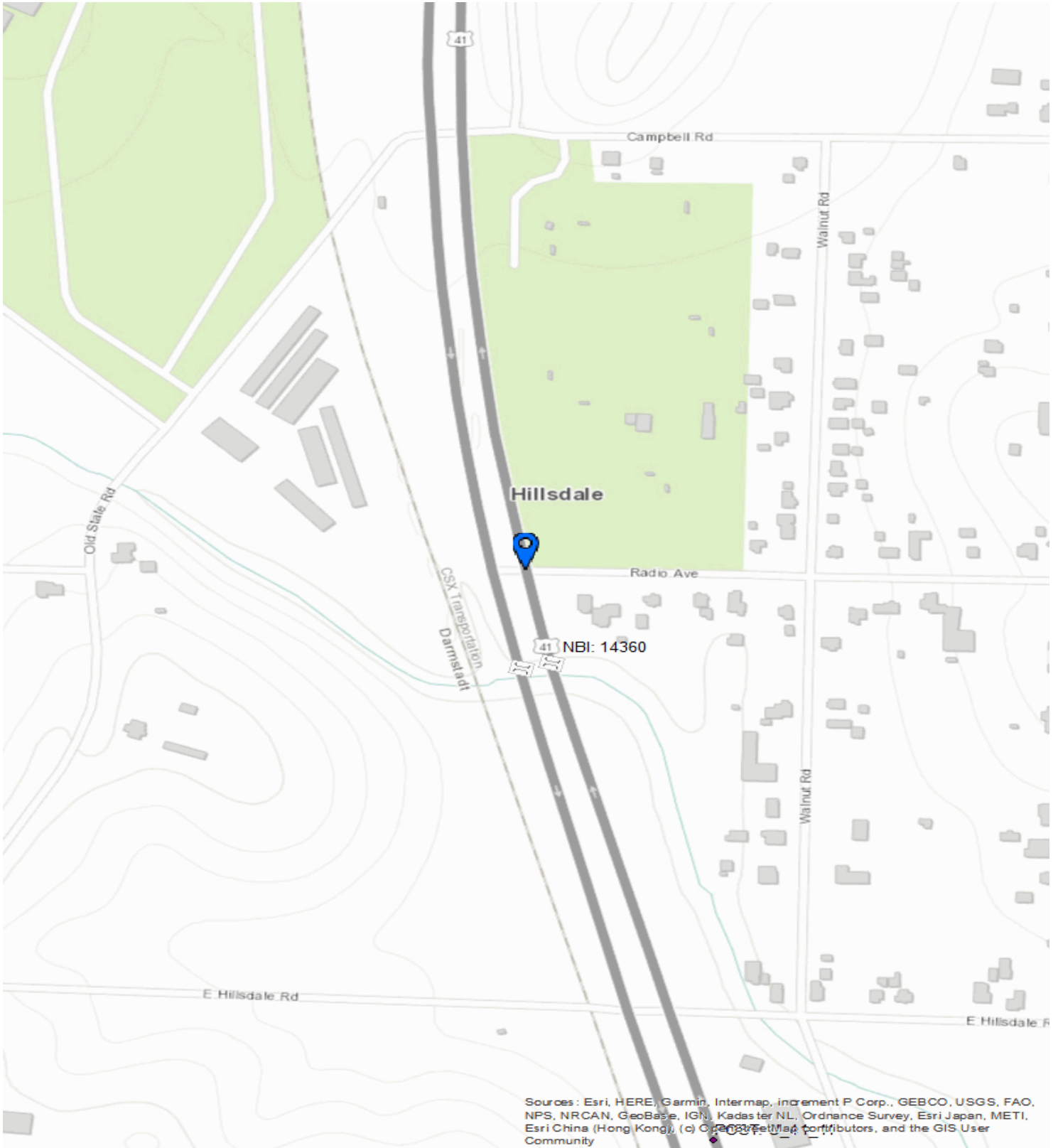
01/02/2019

**By**

INDOT, Office of Traffic Statistics  
Technical Planning Support & Programming Division  
Gregory A. Katter, PE, Supervisor  
100 N. Senate Ave, N955  
Indianapolis, Indiana 46204  
[INDOTTrafficForecasts@indot.IN.gov](mailto:INDOTTrafficForecasts@indot.IN.gov)



PROJECT TRAFFIC FORECAST REPORT







PROJECT TRAFFIC FORECAST REPORT

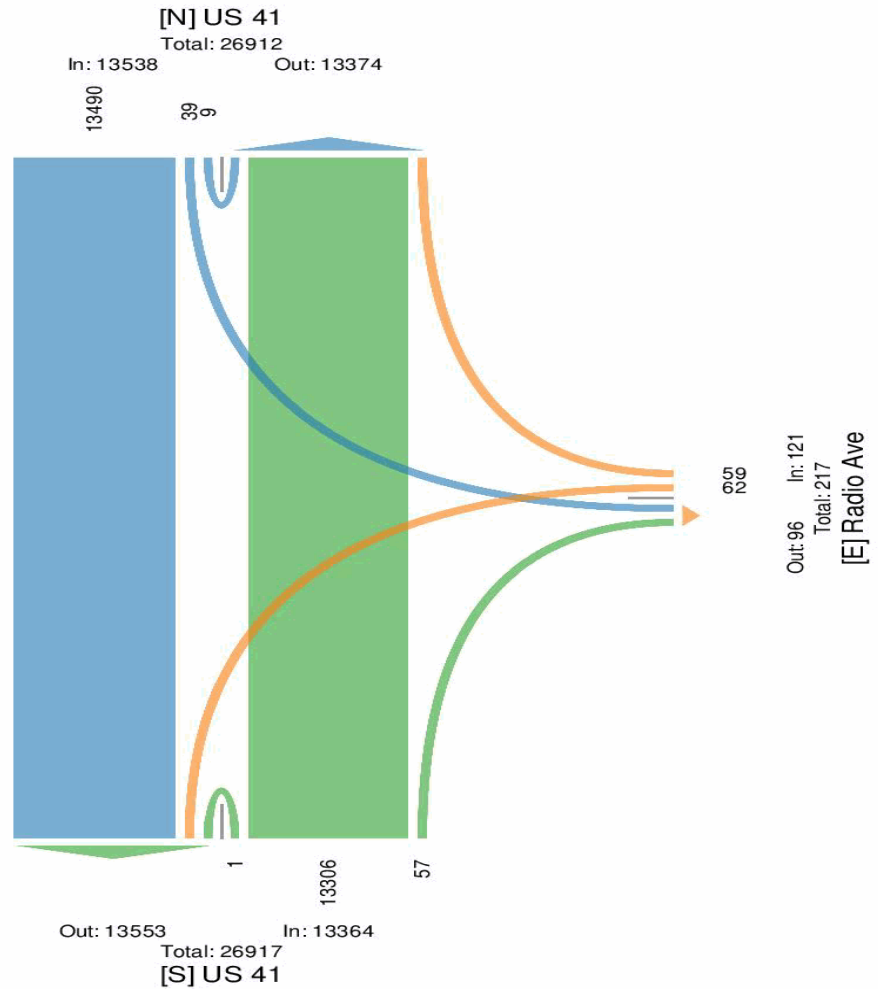
Intersection: 1

EMPO/INDOT US 41 @ Radio Ave - TMC

Thu May 2, 2019
Full Length (2 PM-2 PM (+1))
All Classes (Motorcycles, Cars, Light Goods Vehicles, Single-Unit Trucks, Articulated Trucks, Buses)
All Movements
ID: 650491, Location: 38.083135, -87.556062



Provided by: Indiana DOT
100 N. Senate Ave.,
Indianapolis, IN, 46204, US





**PROJECT TRAFFIC FORECAST REPORT**

**Location:** At Hillsdale Road, 2.04 miles N of SR 57

The table below contains the projected Annual Average Daily Traffic (AADT) in each requested year for each approach and movement.

The per year growth rate used for each approach is indicated in the table below. It is applied as a straight line growth.

For the purpose of this report a commercial vehicle would fall into FHWA Scheme F Classes 4 through 13. They are identified by MioVision as either an Articulated Truck, a Bus, or a Single-Unit Truck.

Daily Movement Forecast										
Approach Road Name	Approach Direction	Movement	Total	Count Year AADT	Growth Rate	Construction Year AADT 2020	Intermediate Year 1 AADT 2025	Intermediate Year 2 AADT 2030	Design Year AADT 2045	Commercial Percentage
RADIO AV	East	Right	59	52	0.00%	52	52	52	52	1.70%
RADIO AV	East	Left	62	55	0.00%	55	55	55	55	1.61%
RADIO AV	East	U-Turn	0	0	0.00%	0	0	0	0	0.00%
RADIO AV	East	Total	121	107	0.00%	107	107	107	107	0.00%
US 41	North	Thru	13,490	11,939	0.00%	11,939	11,939	11,939	11,939	13.05%
US 41	North	Left	39	35	0.00%	35	35	35	35	0.00%
US 41	North	U-Turn	9	8	0.00%	8	8	8	8	0.00%
US 41	North	Total	13,538	11,981	0.00%	11,981	11,981	11,981	11,981	0.00%
US 41	South	Right	57	50	0.00%	50	50	50	50	3.51%
US 41	South	Thru	13,306	11,776	0.00%	11,776	11,776	11,776	11,776	12.69%
US 41	South	U-Turn	1	1	0.00%	1	1	1	1	0.00%
US 41	South	Total	13,364	11,827	0.00%	11,827	11,827	11,827	11,827	0.00%

**Growth Rate Notes**



**PROJECT TRAFFIC FORECAST REPORT**

**Location:** At Hillsdale Road, 2.04 miles N of SR 57

The table below contains the projected traffic volumes in each requested year for approach and movement during the morning and afternoon peak hour.

The morning and afternoon peak hours are those 60 minute periods during which the most vehicles pass through the intersection.

AM PM Peak Movement Forecast										
Approach Direction	Growth Rate	Movement	Interval	Total Vehicles	Commercial % AADT	Count Year AADT	Construction Year AADT 2020	Intermediate Year 1 AADT 2025	Intermediate Year 2 AADT 2030	Design Year AADT 2045
East	0.00%	Left	6:45 AM	5	0.00%	4	4	4	4	4
East	0.00%	Right	6:45 AM	16	0.00%	14	14	14	14	14
East	0.00%	U-Turn	6:45 AM	0	0.00%	0	0	0	0	0
North	0.00%	Left	6:45 AM	0	0.00%	0	0	0	0	0
North	0.00%	Thru	6:45 AM	1042	13.53%	922	922	922	922	922
North	0.00%	U-Turn	6:45 AM	0	0.00%	0	0	0	0	0
South	0.00%	Thru	6:45 AM	962	11.12%	851	851	851	851	851
South	0.00%	Right	6:45 AM	4	0.00%	4	4	4	4	4
South	0.00%	U-Turn	6:45 AM	0	0.00%	0	0	0	0	0
East	0.00%	Left	3:45 PM	1	0.00%	1	1	1	1	1
East	0.00%	Right	3:45 PM	4	25.00%	4	4	4	4	4
East	0.00%	U-Turn	3:45 PM	0	0.00%	0	0	0	0	0
North	0.00%	Left	3:45 PM	3	0.00%	3	3	3	3	3
North	0.00%	Thru	3:45 PM	1239	7.91%	1,097	1,097	1,097	1,097	1,097
North	0.00%	U-Turn	3:45 PM	1	0.00%	1	1	1	1	1
South	0.00%	Thru	3:45 PM	929	9.15%	822	822	822	822	822
South	0.00%	Right	3:45 PM	3	0.00%	3	3	3	3	3
South	0.00%	U-Turn	3:45 PM	0	0.00%	0	0	0	0	0

It should be recognized by users of this forecast that the base year AADT has an accuracy of plus or minus 10%. It should also be understood that while this report may include forecasts with up to six apparent significant figures, the accuracy should not be interpreted as being greater than two significant figures. It is the responsibility of designers to exercise professional judgement when using this data to influence decisions.



**PROJECT TRAFFIC FORECAST REPORT**

**DES No.:** 1400005

US-41 At Hillsdale Road, 2.04 miles N of SR 57

From RP 10+90 to RP 11+1

Vanderburgh County

**Prepared For**

Matthew Bullock

**On**

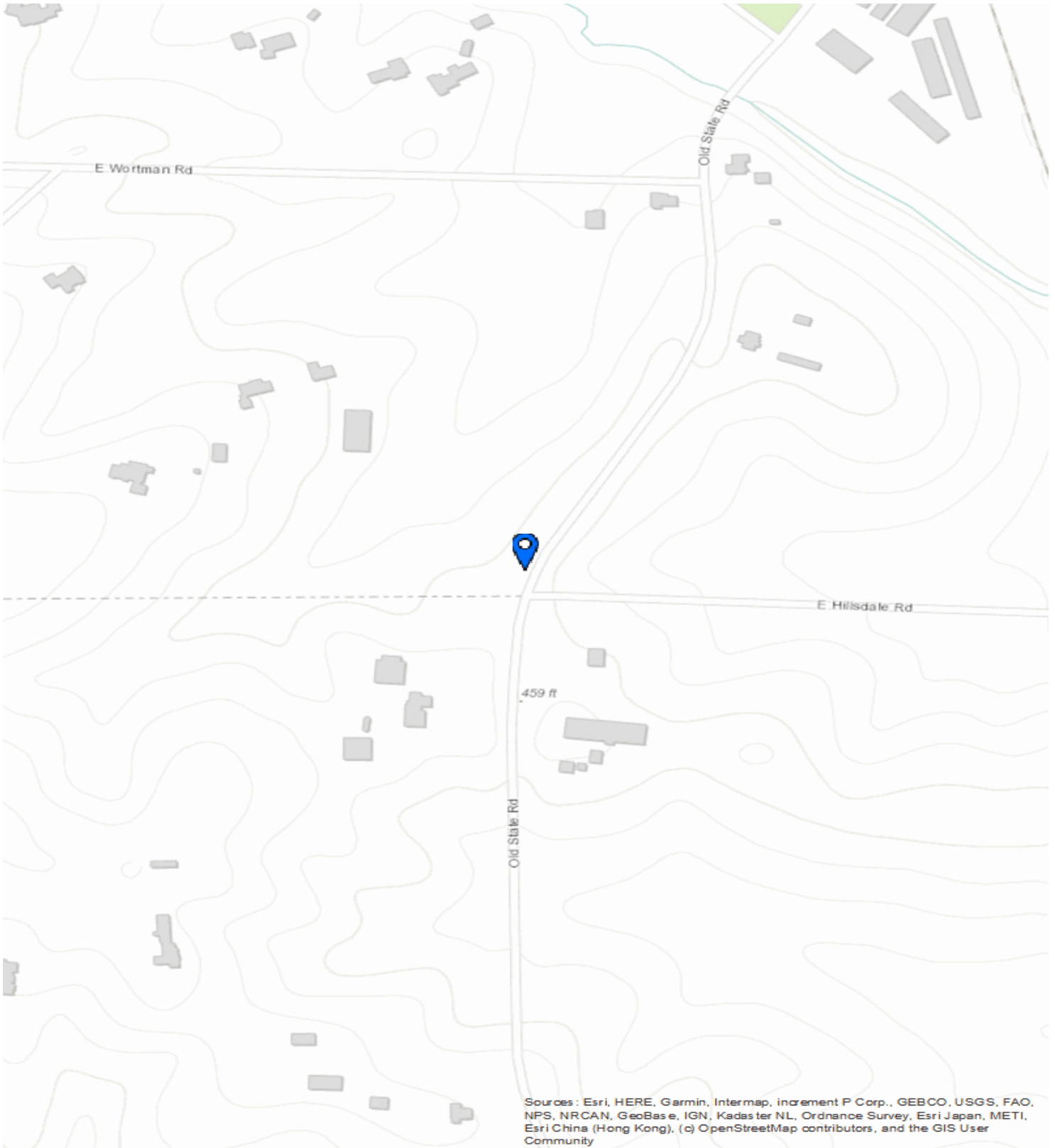
01/02/2019

**By**

INDOT, Office of Traffic Statistics  
Technical Planning Support & Programming Division  
Gregory A. Katter, PE, Supervisor  
100 N. Senate Ave, N955  
Indianapolis, Indiana 46204  
[INDOTTrafficForecasts@indot.IN.gov](mailto:INDOTTrafficForecasts@indot.IN.gov)



PROJECT TRAFFIC FORECAST REPORT



Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), (c) OpenStreetMap contributors, and the GIS User Community



PROJECT TRAFFIC FORECAST REPORT

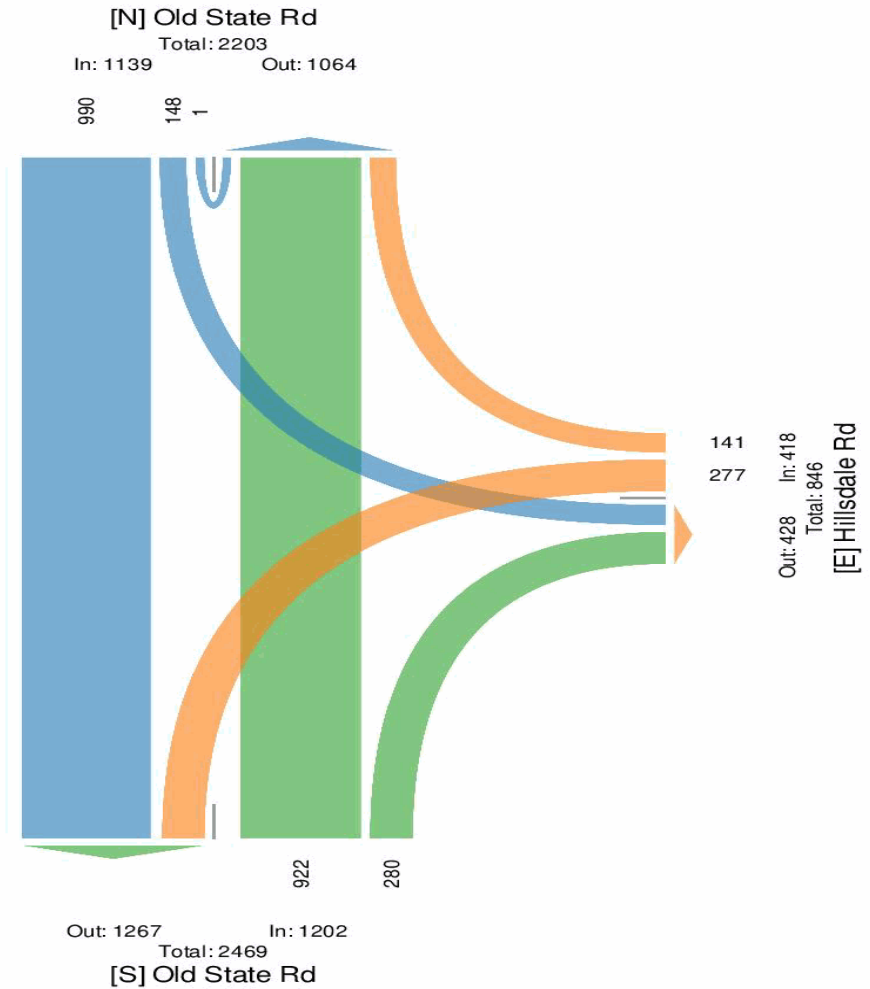
Intersection: 1

EMPO/INDOT Old State Rd @ Hillsdale Rd - TMC

Wed May 1, 2019
Full Length (12 PM-12 PM (+1))
All Classes (Motorcycles, Cars, Light Goods Vehicles, Single-Unit Trucks, Articulated Trucks, Buses)
All Movements
ID: 650505, Location: 38.079693, -87.560941



Provided by: Indiana DOT
100 N. Senate Ave.,
Indianapolis, IN, 46204, US





**PROJECT TRAFFIC FORECAST REPORT**

**Location:** At Hillsdale Road, 2.04 miles N of SR 57

The table below contains the projected Annual Average Daily Traffic (AADT) in each requested year for each approach and movement.

The per year growth rate used for each approach is indicated in the table below. It is applied as a straight line growth.

For the purpose of this report a commercial vehicle would fall into FHWA Scheme F Classes 4 through 13. They are identified by MioVision as either an Articulated Truck, a Bus, or a Single-Unit Truck.

Daily Movement Forecast										
Approach Road Name	Approach Direction	Movement	Total	Count Year AADT	Growth Rate	Construction Year AADT 2020	Intermediate Year 1 AADT 2025	Intermediate Year 2 AADT 2030	Design Year AADT 2045	Commercial Percentage
HILLSDALE RD	East	Right	141	128	0.00%	128	128	128	128	4.26%
HILLSDALE RD	East	Left	277	252	0.00%	252	252	252	252	1.08%
HILLSDALE RD	East	U-Turn	0	0	0.00%	0	0	0	0	0.00%
HILLSDALE RD	East	Total	418	380	0.00%	380	380	380	380	0.00%
OLD STATE RD SB	North	Thru	990	900	0.00%	900	900	900	900	1.72%
OLD STATE RD SB	North	Left	148	135	0.00%	135	135	135	135	4.73%
OLD STATE RD SB	North	U-Turn	1	1	0.00%	1	1	1	1	0.00%
OLD STATE RD SB	North	Total	1,139	1,035	0.00%	1,035	1,035	1,035	1,035	0.00%
OLD SR	South	Right	280	255	0.00%	255	255	255	255	2.86%
OLD SR	South	Thru	922	838	0.00%	838	838	838	838	1.84%
OLD SR	South	U-Turn	0	0	0.00%	0	0	0	0	0.00%
OLD SR	South	Total	1,202	1,093	0.00%	1,093	1,093	1,093	1,093	0.00%

**Growth Rate Notes**



## PROJECT TRAFFIC FORECAST REPORT

**Location:** At Hillsdale Road, 2.04 miles N of SR 57

The table below contains the projected traffic volumes in each requested year for approach and movement during the morning and afternoon peak hour.

The morning and afternoon peak hours are those 60 minute periods during which the most vehicles pass through the intersection.

AM PM Peak Movement Forecast										
Approach Direction	Growth Rate	Movement	Interval	Total Vehicles	Commercial % AADT	Count Year AADT	Construction Year AADT 2020	Intermediate Year 1 AADT 2025	Intermediate Year 2 AADT 2030	Design Year AADT 2045
East	0.00%	Left	6:45 AM	34	0.00%	31	31	31	31	31
East	0.00%	Right	6:45 AM	7	0.00%	6	6	6	6	6
East	0.00%	U-Turn	6:45 AM	0	0.00%	0	0	0	0	0
North	0.00%	Left	6:45 AM	19	0.00%	17	17	17	17	17
North	0.00%	Thru	6:45 AM	66	4.55%	60	60	60	60	60
North	0.00%	U-Turn	6:45 AM	0	0.00%	0	0	0	0	0
South	0.00%	Thru	6:45 AM	97	1.03%	88	88	88	88	88
South	0.00%	Right	6:45 AM	23	0.00%	21	21	21	21	21
South	0.00%	U-Turn	6:45 AM	0	0.00%	0	0	0	0	0
East	0.00%	Left	4:30 PM	18	0.00%	16	16	16	16	16
East	0.00%	Right	4:30 PM	12	0.00%	11	11	11	11	11
East	0.00%	U-Turn	4:30 PM	0	0.00%	0	0	0	0	0
North	0.00%	Left	4:30 PM	10	0.00%	9	9	9	9	9
North	0.00%	Thru	4:30 PM	135	0.00%	123	123	123	123	123
North	0.00%	U-Turn	4:30 PM	0	0.00%	0	0	0	0	0
South	0.00%	Thru	4:30 PM	94	0.00%	85	85	85	85	85
South	0.00%	Right	4:30 PM	30	0.00%	27	27	27	27	27
South	0.00%	U-Turn	4:30 PM	0	0.00%	0	0	0	0	0

It should be recognized by users of this forecast that the base year AADT has an accuracy of plus or minus 10%. It should also be understood that while this report may include forecasts with up to six apparent significant figures, the accuracy should not be interpreted as being greater than two significant figures. It is the responsibility of designers to exercise professional judgement when using this data to influence decisions.





**PROJECT TRAFFIC FORECAST REPORT**

**DES No.:** 1400005

US-41 At Hillsdale Road, 2.04 miles N of SR 57

From RP 10+90 to RP 11+1

Vanderburgh County

**Prepared For**

Matthew Bullock

**On**

01/02/2019

**By**

INDOT, Office of Traffic Statistics  
Technical Planning Support & Programming Division  
Gregory A. Katter, PE, Supervisor  
100 N. Senate Ave, N955  
Indianapolis, Indiana 46204  
[INDOTTrafficForecasts@indot.IN.gov](mailto:INDOTTrafficForecasts@indot.IN.gov)



PROJECT TRAFFIC FORECAST REPORT

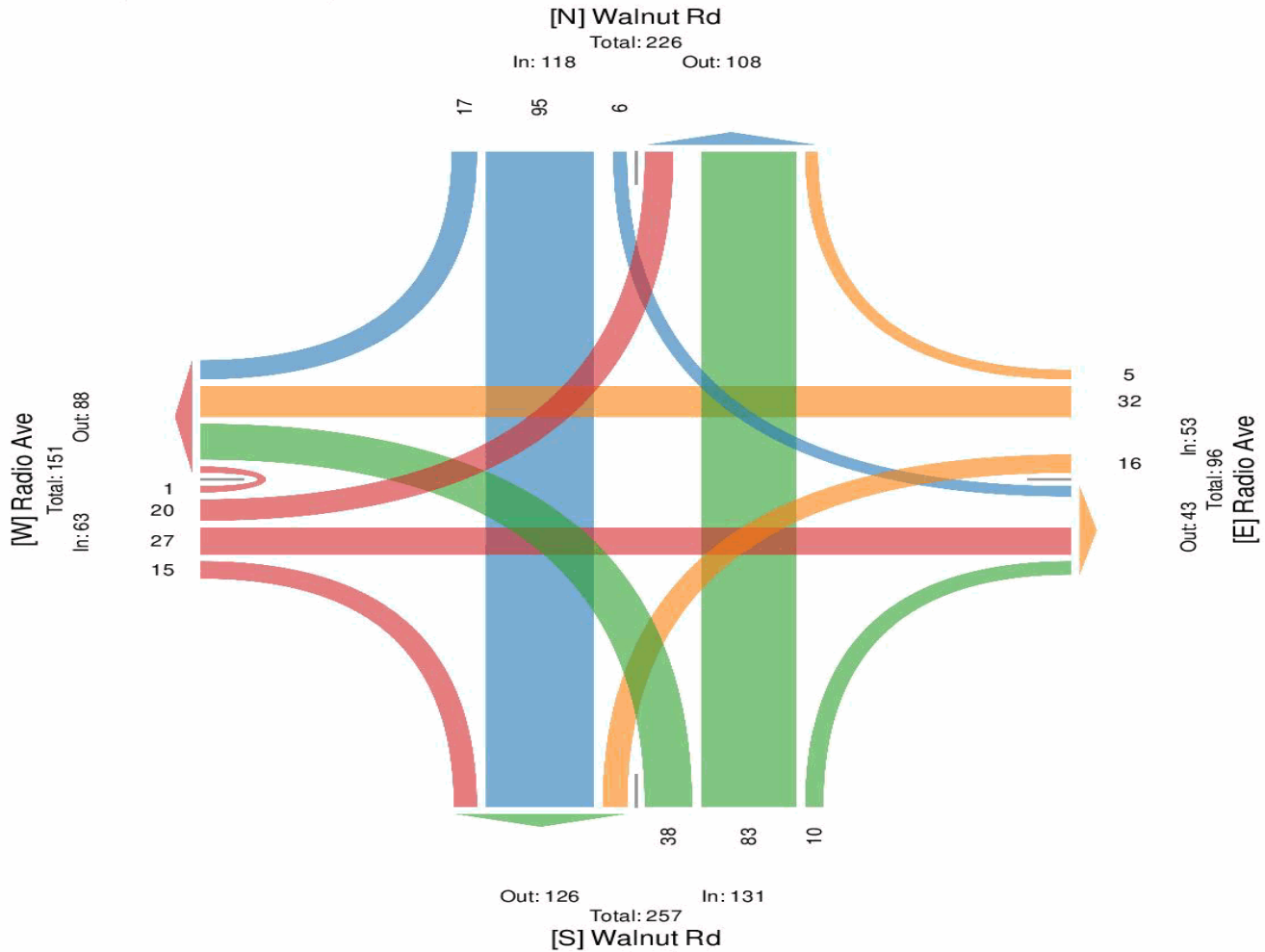
Intersection: 1

EMPO/INDOT Radio Ave @ Walnut Rd - TMC

Wed May 1, 2019
Full Length (12 PM-12 PM (+1))
All Classes (Motorcycles, Cars, Light Goods Vehicles, Single-Unit Trucks, Articulated Trucks, Buses)
All Movements
ID: 650497, Location: 38.083017, -87.553573



Provided by: Indiana DOT
100 N. Senate Ave.,
Indianapolis, IN, 46204, US





**PROJECT TRAFFIC FORECAST REPORT**

**Location:** At Hillsdale Road, 2.04 miles N of SR 57

The table below contains the projected Annual Average Daily Traffic (AADT) in each requested year for each approach and movement.

The per year growth rate used for each approach is indicated in the table below. It is applied as a straight line growth.

For the purpose of this report a commercial vehicle would fall into FHWA Scheme F Classes 4 through 13. They are identified by MioVision as either an Articulated Truck, a Bus, or a Single-Unit Truck.

Daily Movement Forecast										
Approach Road Name	Approach Direction	Movement	Total	Count Year AADT	Growth Rate	Construction Year AADT 2020	Intermediate Year 1 AADT 2025	Intermediate Year 2 AADT 2030	Design Year AADT 2045	Commercial Percentage
RADIO AV	East	Right	5	5	0.00%	5	5	5	5	40.00%
RADIO AV	East	Thru	32	29	0.00%	29	29	29	29	3.13%
RADIO AV	East	Left	16	15	0.00%	15	15	15	15	6.25%
RADIO AV	East	U-Turn	0	0	0.00%	0	0	0	0	0.00%
RADIO AV	East	Total	53	48	0.00%	48	48	48	48	0.00%
WALNUT RD	North	Right	17	15	0.00%	15	15	15	15	0.00%
WALNUT RD	North	Thru	95	87	0.00%	87	87	87	87	2.11%
WALNUT RD	North	Left	6	5	0.00%	5	5	5	5	0.00%
WALNUT RD	North	U-Turn	0	0	0.00%	0	0	0	0	0.00%
WALNUT RD	North	Total	118	107	0.00%	107	107	107	107	0.00%
WALNUT RD	South	Right	10	9	0.00%	9	9	9	9	30.00%
WALNUT RD	South	Thru	83	76	0.00%	76	76	76	76	1.21%
WALNUT RD	South	Left	38	35	0.00%	35	35	35	35	2.63%
WALNUT RD	South	U-Turn	0	0	0.00%	0	0	0	0	0.00%
WALNUT RD	South	Total	131	119	0.00%	119	119	119	119	0.00%
RADIO AV	West	Right	15	14	0.00%	14	14	14	14	0.00%
RADIO AV	West	Thru	27	25	0.00%	25	25	25	25	3.70%
RADIO AV	West	Left	20	18	0.00%	18	18	18	18	5.00%
RADIO AV	West	U-Turn	1	1	0.00%	1	1	1	1	0.00%
RADIO AV	West	Total	63	57	0.00%	57	57	57	57	0.00%

**Growth Rate Notes**



**PROJECT TRAFFIC FORECAST REPORT**

**Location:** At Hillsdale Road, 2.04 miles N of SR 57

The table below contains the projected traffic volumes in each requested year for approach and movement during the morning and afternoon peak hour.

The morning and afternoon peak hours are those 60 minute periods during which the most vehicles pass through the intersection.

AM PM Peak Movement Forecast										
Approach Direction	Growth Rate	Movement	Interval	Total Vehicles	Commercial % AADT	Count Year AADT	Construction Year AADT 2020	Intermediate Year 1 AADT 2025	Intermediate Year 2 AADT 2030	Design Year AADT 2045
East	0.00%	Left	6:45 AM	2	0.00%	2	2	2	2	2
East	0.00%	Thru	6:45 AM	2	50.00%	2	2	2	2	2
East	0.00%	Right	6:45 AM	1	0.00%	1	1	1	1	1
East	0.00%	U-Turn	6:45 AM	0	0.00%	0	0	0	0	0
North	0.00%	Left	6:45 AM	1	0.00%	1	1	1	1	1
North	0.00%	Thru	6:45 AM	3	0.00%	3	3	3	3	3
North	0.00%	Right	6:45 AM	4	0.00%	4	4	4	4	4
North	0.00%	U-Turn	6:45 AM	0	0.00%	0	0	0	0	0
South	0.00%	Left	6:45 AM	8	0.00%	7	7	7	7	7
South	0.00%	Thru	6:45 AM	13	0.00%	12	12	12	12	12
South	0.00%	Right	6:45 AM	1	100.00%	1	1	1	1	1
South	0.00%	U-Turn	6:45 AM	0	0.00%	0	0	0	0	0
West	0.00%	Left	6:45 AM	3	0.00%	3	3	3	3	3
West	0.00%	Thru	6:45 AM	1	0.00%	1	1	1	1	1
West	0.00%	Right	6:45 AM	1	0.00%	1	1	1	1	1
West	0.00%	U-Turn	6:45 AM	0	0.00%	0	0	0	0	0
East	0.00%	Left	5:15 PM	0	0.00%	0	0	0	0	0
East	0.00%	Thru	5:15 PM	4	0.00%	4	4	4	4	4
East	0.00%	Right	5:15 PM	0	0.00%	0	0	0	0	0
East	0.00%	U-Turn	5:15 PM	0	0.00%	0	0	0	0	0
North	0.00%	Left	5:15 PM	1	0.00%	1	1	1	1	1
North	0.00%	Thru	5:15 PM	11	0.00%	10	10	10	10	10
North	0.00%	Right	5:15 PM	1	0.00%	1	1	1	1	1
North	0.00%	U-Turn	5:15 PM	0	0.00%	0	0	0	0	0
South	0.00%	Left	5:15 PM	7	0.00%	6	6	6	6	6
South	0.00%	Thru	5:15 PM	11	0.00%	10	10	10	10	10
South	0.00%	Right	5:15 PM	1	0.00%	1	1	1	1	1
South	0.00%	U-Turn	5:15 PM	0	0.00%	0	0	0	0	0
West	0.00%	Left	5:15 PM	1	0.00%	1	1	1	1	1
West	0.00%	Thru	5:15 PM	4	0.00%	4	4	4	4	4
West	0.00%	Right	5:15 PM	3	0.00%	3	3	3	3	3
West	0.00%	U-Turn	5:15 PM	0	0.00%	0	0	0	0	0

It should be recognized by users of this forecast that the base year AADT has an accuracy of plus or minus 10%. It should also be understood that while this report may include forecasts with up to six apparent significant figures, the accuracy should not be interpreted as being greater than two significant figures. It is the responsibility of designers to exercise professional judgement when using this data to influence decisions.



**PROJECT TRAFFIC FORECAST REPORT**

**DES No.:** 1400005

US-41 At Hillsdale Road, 2.04 miles N of SR 57

From RP 10+90 to RP 11+1

Vanderburgh County

**Prepared For**

Matthew Bullock

**On**

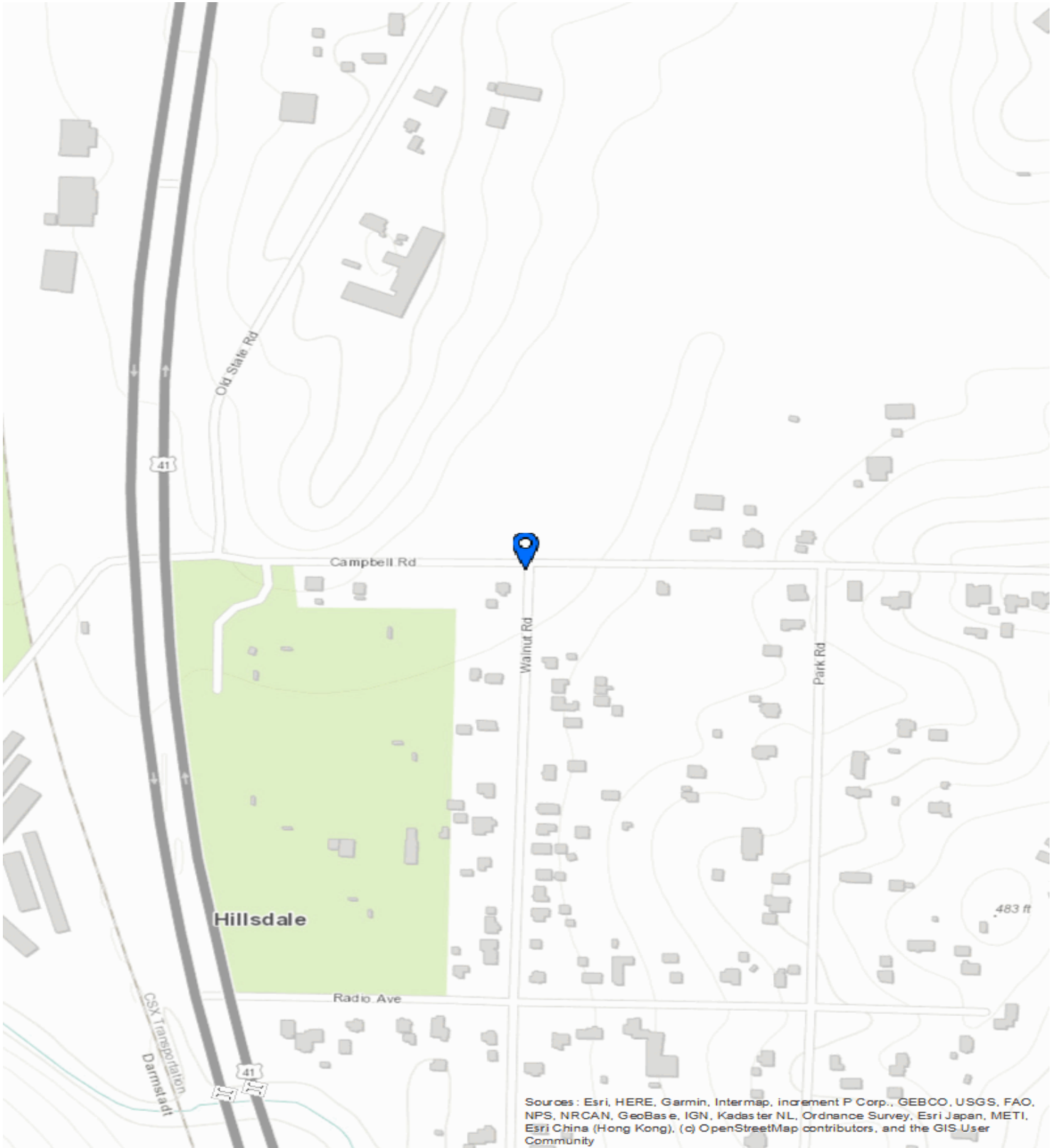
01/02/2019

**By**

INDOT, Office of Traffic Statistics  
Technical Planning Support & Programming Division  
Gregory A. Katter, PE, Supervisor  
100 N. Senate Ave, N955  
Indianapolis, Indiana 46204  
[INDOTTrafficForecasts@indot.IN.gov](mailto:INDOTTrafficForecasts@indot.IN.gov)



PROJECT TRAFFIC FORECAST REPORT



Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), (c) OpenStreetMap contributors, and the GIS User Community



PROJECT TRAFFIC FORECAST REPORT

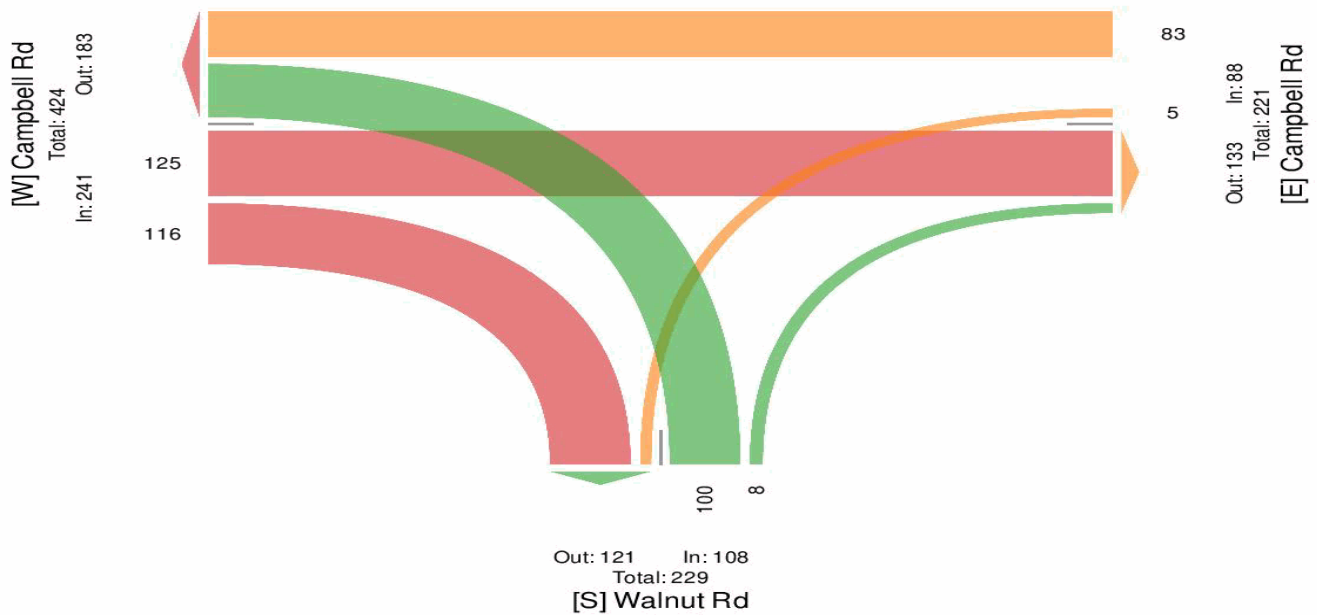
Intersection: 1

EMPO/INDOT Campbell Rd @ Walnut Rd - TMC

Wed May 1, 2019
Full Length (12 PM-12 PM (+1))
All Classes (Motorcycles, Cars, Light Goods Vehicles, Single-Unit Trucks, Articulated Trucks, Buses)
All Movements
ID: 650500, Location: 38.08659, -87.553434



Provided by: Indiana DOT
100 N. Senate Ave.,
Indianapolis, IN, 46204, US





**PROJECT TRAFFIC FORECAST REPORT**

**Location:** At Hillsdale Road, 2.04 miles N of SR 57

The table below contains the projected Annual Average Daily Traffic (AADT) in each requested year for each approach and movement.

The per year growth rate used for each approach is indicated in the table below. It is applied as a straight line growth.

For the purpose of this report a commercial vehicle would fall into FHWA Scheme F Classes 4 through 13. They are identified by MioVision as either an Articulated Truck, a Bus, or a Single-Unit Truck.

Daily Movement Forecast										
Approach Road Name	Approach Direction	Movement	Total	Count Year AADT	Growth Rate	Construction Year AADT 2020	Intermediate Year 1 AADT 2025	Intermediate Year 2 AADT 2030	Design Year AADT 2045	Commercial Percentage
CAMPBELL RD	East	Thru	83	76	0.01%	76	76	76	76	6.02%
CAMPBELL RD	East	Left	5	5	0.01%	5	5	5	5	0.00%
CAMPBELL RD	East	U-Turn	0	0	0.01%	0	0	0	0	0.00%
CAMPBELL RD	East	Total	88	80	0.01%	80	80	80	80	0.00%
WALNUT RD	South	Right	8	7	0.01%	7	7	7	7	12.50%
WALNUT RD	South	Left	100	91	0.01%	91	91	91	91	2.00%
WALNUT RD	South	U-Turn	0	0	0.01%	0	0	0	0	0.00%
WALNUT RD	South	Total	108	98	0.01%	98	98	98	99	0.00%
CAMPBELL RD	West	Right	116	106	0.01%	106	106	106	106	1.72%
CAMPBELL RD	West	Thru	125	114	0.01%	114	114	114	114	2.40%
CAMPBELL RD	West	U-Turn	0	0	0.01%	0	0	0	0	0.00%
CAMPBELL RD	West	Total	241	220	0.01%	220	220	220	220	0.00%

**Growth Rate Notes**





**PROJECT TRAFFIC FORECAST REPORT**

**Location:** At Hillsdale Road, 2.04 miles N of SR 57

The table below contains the projected traffic volumes in each requested year for approach and movement during the morning and afternoon peak hour.

The morning and afternoon peak hours are those 60 minute periods during which the most vehicles pass through the intersection.

AM PM Peak Movement Forecast										
Approach Direction	Growth Rate	Movement	Interval	Total Vehicles	Commercial % AADT	Count Year AADT	Construction Year AADT 2020	Intermediate Year 1 AADT 2025	Intermediate Year 2 AADT 2030	Design Year AADT 2045
East	0.01%	Left	6:45 AM	1	0.00%	1	1	1	1	1
East	0.01%	Thru	6:45 AM	13	7.69%	12	12	12	12	12
East	0.01%	U-Turn	6:45 AM	0	0.00%	0	0	0	0	0
South	0.01%	Left	6:45 AM	14	0.00%	13	13	13	13	13
South	0.01%	Right	6:45 AM	2	0.00%	2	2	2	2	2
South	0.01%	U-Turn	6:45 AM	0	0.00%	0	0	0	0	0
West	0.01%	Thru	6:45 AM	4	0.00%	4	4	4	4	4
West	0.01%	Right	6:45 AM	2	0.00%	2	2	2	2	2
West	0.01%	U-Turn	6:45 AM	0	0.00%	0	0	0	0	0
East	0.01%	Left	5:00 PM	1	0.00%	1	1	1	1	1
East	0.01%	Thru	5:00 PM	5	0.00%	5	5	5	5	5
East	0.01%	U-Turn	5:00 PM	0	0.00%	0	0	0	0	0
South	0.01%	Left	5:00 PM	9	0.00%	8	8	8	8	8
South	0.01%	Right	5:00 PM	0	0.00%	0	0	0	0	0
South	0.01%	U-Turn	5:00 PM	0	0.00%	0	0	0	0	0
West	0.01%	Thru	5:00 PM	17	0.00%	15	15	15	15	15
West	0.01%	Right	5:00 PM	17	0.00%	15	15	15	15	15
West	0.01%	U-Turn	5:00 PM	0	0.00%	0	0	0	0	0

It should be recognized by users of this forecast that the base year AADT has an accuracy of plus or minus 10%. It should also be understood that while this report may include forecasts with up to six apparent significant figures, the accuracy should not be interpreted as being greater than two significant figures. It is the responsibility of designers to exercise professional judgement when using this data to influence decisions.



**PROJECT TRAFFIC FORECAST REPORT**

**DES No.:** 1400005

US-41 At Hillsdale Road, 2.04 miles N of SR 57

From RP 10+90 to RP 11+1

Vanderburgh County

**Prepared For**

Matthew Bullock

**On**

01/02/2019

**By**

INDOT, Office of Traffic Statistics  
Technical Planning Support & Programming Division  
Gregory A. Katter, PE, Supervisor  
100 N. Senate Ave, N955  
Indianapolis, Indiana 46204  
[INDOTTrafficForecasts@indot.IN.gov](mailto:INDOTTrafficForecasts@indot.IN.gov)



PROJECT TRAFFIC FORECAST REPORT

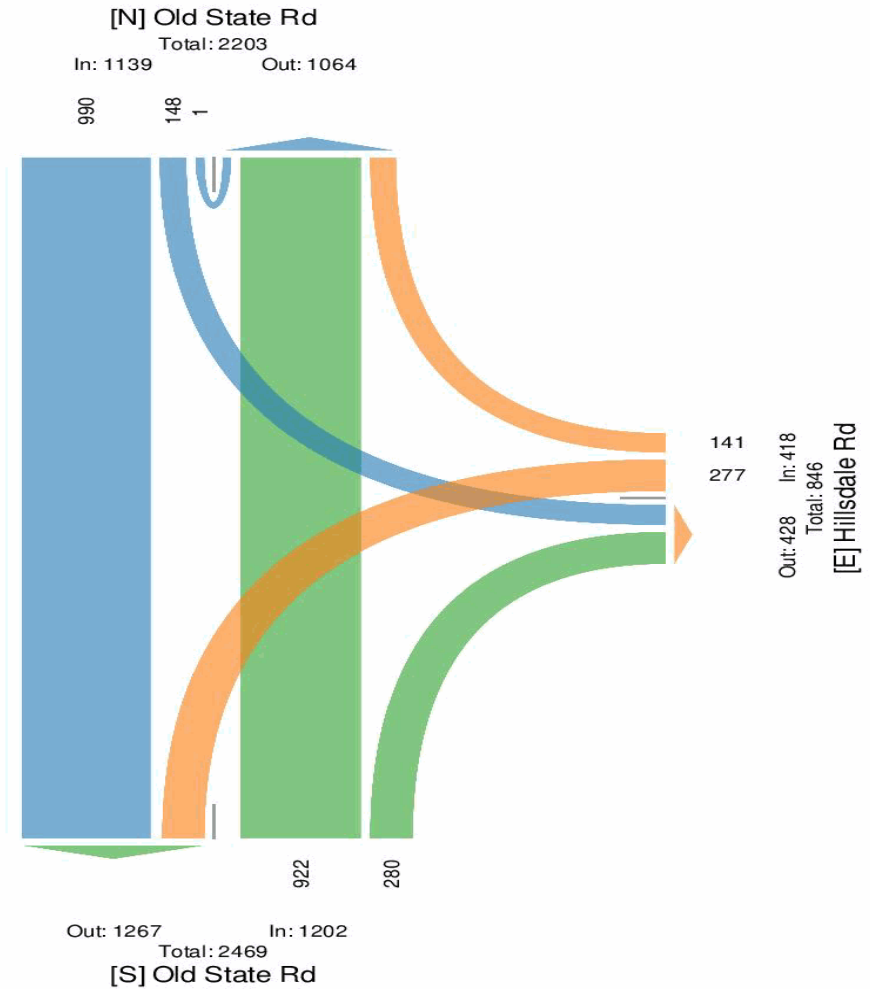
Intersection: 1

EMPO/INDOT Old State Rd @ Hillsdale Rd - TMC

Wed May 1, 2019
Full Length (12 PM-12 PM (+1))
All Classes (Motorcycles, Cars, Light Goods Vehicles, Single-Unit Trucks, Articulated Trucks, Buses)
All Movements
ID: 650505, Location: 38.079693, -87.560941



Provided by: Indiana DOT
100 N. Senate Ave.,
Indianapolis, IN, 46204, US





**PROJECT TRAFFIC FORECAST REPORT**

**Location:** At Hillsdale Road, 2.04 miles N of SR 57

The table below contains the projected Annual Average Daily Traffic (AADT) in each requested year for each approach and movement.

The per year growth rate used for each approach is indicated in the table below. It is applied as a straight line growth.

For the purpose of this report a commercial vehicle would fall into FHWA Scheme F Classes 4 through 13. They are identified by MioVision as either an Articulated Truck, a Bus, or a Single-Unit Truck.

Daily Movement Forecast										
Approach Road Name	Approach Direction	Movement	Total	Count Year AADT	Growth Rate	Construction Year AADT 2020	Intermediate Year 1 AADT 2025	Intermediate Year 2 AADT 2030	Design Year AADT 2045	Commercial Percentage
HILLSDALE RD	East	Right	141	128	0.00%	128	128	128	128	4.26%
HILLSDALE RD	East	Left	277	252	0.00%	252	252	252	252	1.08%
HILLSDALE RD	East	U-Turn	0	0	0.00%	0	0	0	0	0.00%
HILLSDALE RD	East	Total	418	381	0.00%	381	381	381	381	0.00%
OLD STATE RD	North	Thru	990	900	0.00%	900	900	900	900	1.72%
OLD STATE RD	North	Left	148	135	0.00%	135	135	135	135	4.73%
OLD STATE RD	North	U-Turn	1	1	0.00%	1	1	1	1	0.00%
OLD STATE RD	North	Total	1,139	1,035	0.00%	1,035	1,035	1,035	1,035	0.00%
OLD SR SB	South	Right	280	280	0.00%	280	280	280	280	2.86%
OLD SR SB	South	Thru	922	922	0.00%	922	922	922	922	1.84%
OLD SR SB	South	U-Turn	0	0	0.00%	0	0	0	0	0.00%
OLD SR SB	South	Total	1,202	1,202	0.00%	1,202	1,202	1,202	1,202	0.00%

**Growth Rate Notes**



**PROJECT TRAFFIC FORECAST REPORT**

**Location:** At Hillsdale Road, 2.04 miles N of SR 57

The table below contains the projected traffic volumes in each requested year for approach and movement during the morning and afternoon peak hour.

The morning and afternoon peak hours are those 60 minute periods during which the most vehicles pass through the intersection.

AM PM Peak Movement Forecast										
Approach Direction	Growth Rate	Movement	Interval	Total Vehicles	Commercial % AADT	Count Year AADT	Construction Year AADT 2020	Intermediate Year 1 AADT 2025	Intermediate Year 2 AADT 2030	Design Year AADT 2045
East	0.00%	Left	6:45 AM	34	0.00%	31	31	31	31	31
East	0.00%	Right	6:45 AM	7	0.00%	6	6	6	6	6
East	0.00%	U-Turn	6:45 AM	0	0.00%	0	0	0	0	0
North	0.00%	Left	6:45 AM	19	0.00%	17	17	17	17	17
North	0.00%	Thru	6:45 AM	66	4.55%	60	60	60	60	60
North	0.00%	U-Turn	6:45 AM	0	0.00%	0	0	0	0	0
South	0.00%	Thru	6:45 AM	97	1.03%	97	97	97	97	97
South	0.00%	Right	6:45 AM	23	0.00%	23	23	23	23	23
South	0.00%	U-Turn	6:45 AM	0	0.00%	0	0	0	0	0
East	0.00%	Left	4:30 PM	18	0.00%	16	16	16	16	16
East	0.00%	Right	4:30 PM	12	0.00%	11	11	11	11	11
East	0.00%	U-Turn	4:30 PM	0	0.00%	0	0	0	0	0
North	0.00%	Left	4:30 PM	10	0.00%	9	9	9	9	9
North	0.00%	Thru	4:30 PM	135	0.00%	123	123	123	123	123
North	0.00%	U-Turn	4:30 PM	0	0.00%	0	0	0	0	0
South	0.00%	Thru	4:30 PM	94	0.00%	94	94	94	94	94
South	0.00%	Right	4:30 PM	30	0.00%	30	30	30	30	30
South	0.00%	U-Turn	4:30 PM	0	0.00%	0	0	0	0	0

It should be recognized by users of this forecast that the base year AADT has an accuracy of plus or minus 10%. It should also be understood that while this report may include forecasts with up to six apparent significant figures, the accuracy should not be interpreted as being greater than two significant figures. It is the responsibility of designers to exercise professional judgement when using this data to influence decisions.



**PROJECT TRAFFIC FORECAST REPORT**

**DES No.:** 1400005

US-41 At Hillsdale Road, 2.04 miles N of SR 57

From RP 10+90 to RP 11+1

Vanderburgh County

**Prepared For**

Matthew Bullock

**On**

01/02/2019

**By**

INDOT, Office of Traffic Statistics  
Technical Planning Support & Programming Division  
Gregory A. Katter, PE, Supervisor  
100 N. Senate Ave, N955  
Indianapolis, Indiana 46204  
[INDOTTrafficForecasts@indot.IN.gov](mailto:INDOTTrafficForecasts@indot.IN.gov)



PROJECT TRAFFIC FORECAST REPORT





PROJECT TRAFFIC FORECAST REPORT

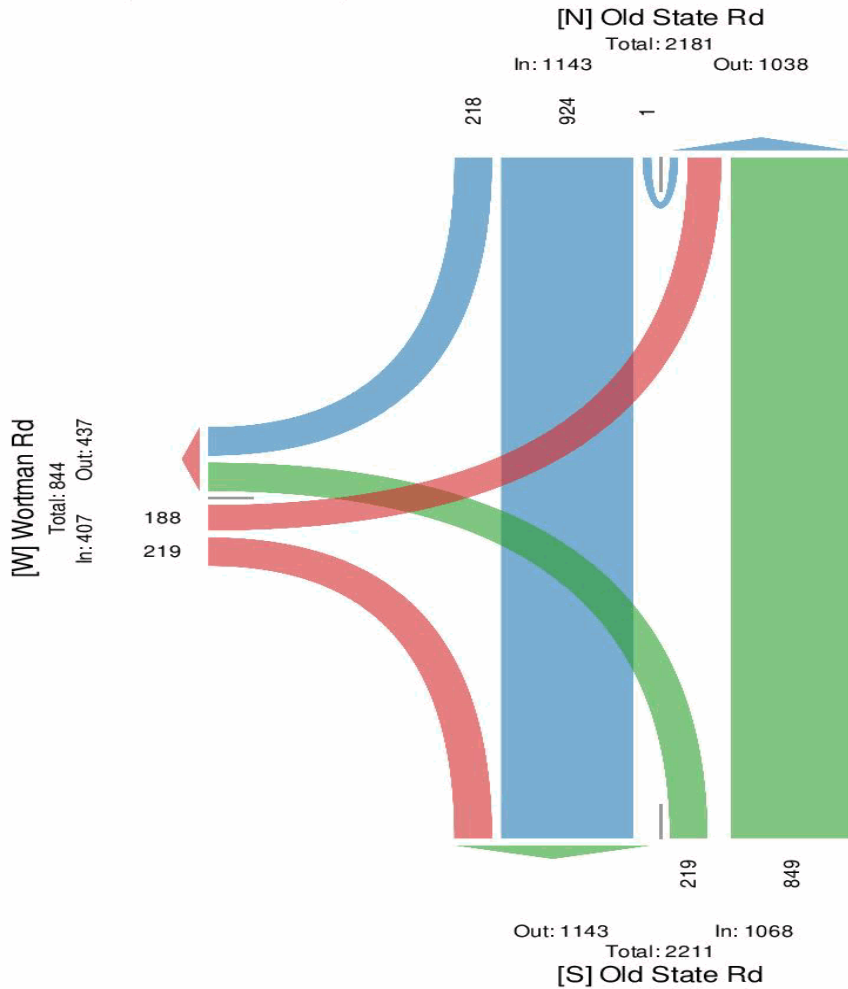
Intersection: 1

EMPO/INDOT Old State Rd @ Wortman Rd - TMC

Wed May 1, 2019
Full Length (12 PM-12 PM (+1))
All Classes (Motorcycles, Cars, Light Goods Vehicles, Single-Unit Trucks, Articulated Trucks, Buses)
All Movements
ID: 650503, Location: 38.083096, -87.559539



Provided by: Indiana DOT
100 N. Senate Ave.,
Indianapolis, IN, 46204, US







**PROJECT TRAFFIC FORECAST REPORT**

**Location:** At Hillsdale Road, 2.04 miles N of SR 57

The table below contains the projected Annual Average Daily Traffic (AADT) in each requested year for each approach and movement.

The per year growth rate used for each approach is indicated in the table below. It is applied as a straight line growth.

For the purpose of this report a commercial vehicle would fall into FHWA Scheme F Classes 4 through 13. They are identified by MioVision as either an Articulated Truck, a Bus, or a Single-Unit Truck.

Daily Movement Forecast										
Approach Road Name	Approach Direction	Movement	Total	Count Year AADT	Growth Rate	Construction Year AADT 2020	Intermediate Year 1 AADT 2025	Intermediate Year 2 AADT 2030	Design Year AADT 2045	Commercial Percentage
OLD STATE RD	North	Right	218	198	0.00%	198	198	198	198	7.34%
OLD STATE RD	North	Thru	924	840	0.00%	840	840	840	840	1.52%
OLD STATE RD	North	U-Turn	1	1	0.00%	1	1	1	1	0.00%
OLD STATE RD	North	Total	1,143	1,039	0.00%	1,039	1,039	1,039	1,039	0.00%
OLD STATE RD	South	Thru	849	772	0.00%	772	772	772	772	1.89%
OLD STATE RD	South	Left	219	199	0.00%	199	199	199	199	1.37%
OLD STATE RD	South	U-Turn	0	0	0.00%	0	0	0	0	0.00%
OLD STATE RD	South	Total	1,068	971	0.00%	971	971	971	971	0.00%
WORTHMAN RD	West	Right	219	199	0.00%	199	199	199	199	5.02%
WORTHMAN RD	West	Left	188	171	0.00%	171	171	171	171	6.92%
WORTHMAN RD	West	U-Turn	0	0	0.00%	0	0	0	0	0.00%
WORTHMAN RD	West	Total	407	370	0.00%	370	370	370	370	0.00%

**Growth Rate Notes**



**PROJECT TRAFFIC FORECAST REPORT**

**Location:** At Hillsdale Road, 2.04 miles N of SR 57

The table below contains the projected traffic volumes in each requested year for approach and movement during the morning and afternoon peak hour.

The morning and afternoon peak hours are those 60 minute periods during which the most vehicles pass through the intersection.

AM PM Peak Movement Forecast										
Approach Direction	Growth Rate	Movement	Interval	Total Vehicles	Commercial % AADT	Count Year AADT	Construction Year AADT 2020	Intermediate Year 1 AADT 2025	Intermediate Year 2 AADT 2030	Design Year AADT 2045
North	0.00%	Thru	6:45 AM	57	5.26%	52	52	52	52	52
North	0.00%	Right	6:45 AM	8	25.00%	7	7	7	7	7
North	0.00%	U-Turn	6:45 AM	0	0.00%	0	0	0	0	0
South	0.00%	Left	6:45 AM	13	0.00%	12	12	12	12	12
South	0.00%	Thru	6:45 AM	93	1.08%	85	85	85	85	85
South	0.00%	U-Turn	6:45 AM	0	0.00%	0	0	0	0	0
West	0.00%	Left	6:45 AM	20	5.00%	18	18	18	18	18
West	0.00%	Right	6:45 AM	26	0.00%	24	24	24	24	24
West	0.00%	U-Turn	6:45 AM	0	0.00%	0	0	0	0	0
North	0.00%	Thru	4:30 PM	127	0.00%	115	115	115	115	115
North	0.00%	Right	4:30 PM	31	0.00%	28	28	28	28	28
North	0.00%	U-Turn	4:30 PM	0	0.00%	0	0	0	0	0
South	0.00%	Left	4:30 PM	24	0.00%	22	22	22	22	22
South	0.00%	Thru	4:30 PM	81	0.00%	74	74	74	74	74
South	0.00%	U-Turn	4:30 PM	0	0.00%	0	0	0	0	0
West	0.00%	Left	4:30 PM	16	0.00%	15	15	15	15	15
West	0.00%	Right	4:30 PM	20	0.00%	18	18	18	18	18
West	0.00%	U-Turn	4:30 PM	0	0.00%	0	0	0	0	0

It should be recognized by users of this forecast that the base year AADT has an accuracy of plus or minus 10%. It should also be understood that while this report may include forecasts with up to six apparent significant figures, the accuracy should not be interpreted as being greater than two significant figures. It is the responsibility of designers to exercise professional judgement when using this data to influence decisions.



PROJECT TRAFFIC FORECAST REPORT

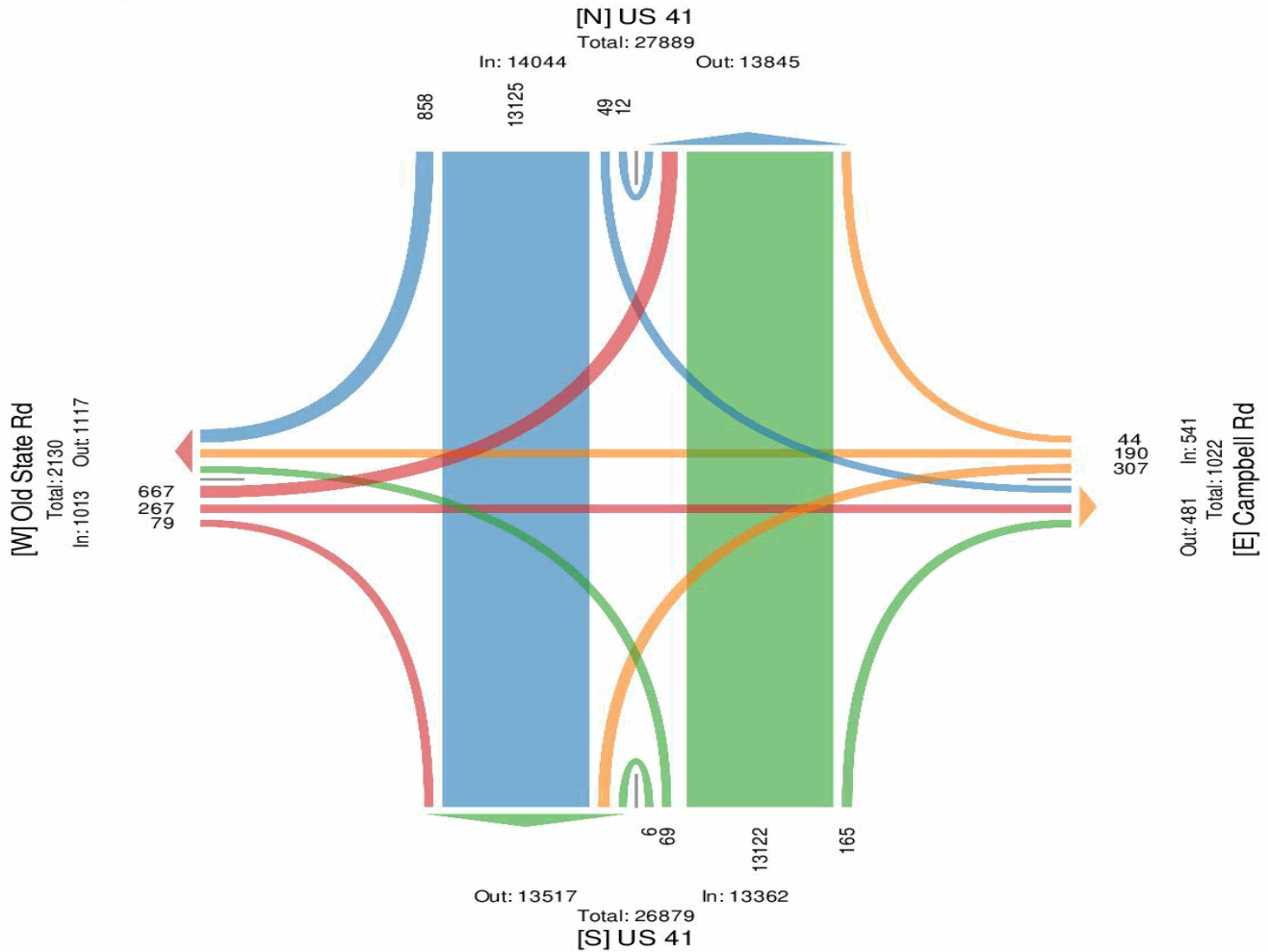
Intersection: 2

EMPO/INDOT US 41 @ Campbell Rd - TMC

Thu May 2, 2019
Full Length (2 PM-2 PM (+1))
All Classes (Motorcycles, Cars, Light Goods Vehicles, Single-Unit Trucks, Articulated Trucks, Buses)
All Movements
ID: 650494, Location: 38.086613, -87.556556



Provided by: Indiana DOT
100 N. Senate Ave.,
Indianapolis, IN, 46204, US





**PROJECT TRAFFIC FORECAST REPORT**

**Location:** At Hillsdale Road, 2.04 miles N of SR 57

The table below contains the projected Annual Average Daily Traffic (AADT) in each requested year for each approach and movement.

The per year growth rate used for each approach is indicated in the table below. It is applied as a straight line growth.

For the purpose of this report a commercial vehicle would fall into FHWA Scheme F Classes 4 through 13. They are identified by MioVision as either an Articulated Truck, a Bus, or a Single-Unit Truck.

Daily Movement Forecast										
Approach Road Name	Approach Direction	Movement	Total	Count Year AADT	Growth Rate	Construction Year AADT 2020	Intermediate Year 1 AADT 2025	Intermediate Year 2 AADT 2030	Design Year AADT 2045	Commercial Percentage
OLD STATE RD	East	Right	44	39	0.01%	39	39	39	39	0.00%
OLD STATE RD	East	Thru	190	168	0.01%	168	168	168	169	1.05%
OLD STATE RD	East	Left	307	272	0.01%	272	272	272	272	0.65%
OLD STATE RD	East	U-Turn	0	0	0.01%	0	0	0	0	0.00%
OLD STATE RD	East	Total	541	479	0.01%	479	479	479	480	0.00%
US 41	North	Right	858	759	1.00%	767	805	843	957	2.68%
US 41	North	Thru	13,125	11,616	1.00%	11,732	12,313	12,893	14,636	13.11%
US 41	North	Left	49	43	1.00%	44	46	48	55	2.04%
US 41	North	U-Turn	12	11	1.00%	11	11	12	13	8.33%
US 41	North	Total	14,044	12,429	1.00%	12,553	13,175	13,796	15,660	0.00%
US 41 SB	South	Right	165	165	1.00%	167	175	183	208	4.85%
US 41 SB	South	Thru	13,122	13,122	1.00%	13,253	13,909	14,565	16,534	12.65%
US 41 SB	South	Left	69	69	1.00%	70	73	77	87	2.90%
US 41 SB	South	U-Turn	6	6	1.00%	6	6	7	8	16.67%
US 41 SB	South	Total	13,362	13,362	1.00%	13,496	14,164	14,832	16,836	0.00%
OLD STATE RD	West	Right	79	70	0.01%	70	70	70	70	1.27%
OLD STATE RD	West	Thru	267	236	0.01%	236	236	237	237	1.87%
OLD STATE RD	West	Left	667	590	0.01%	590	591	591	592	1.65%
OLD STATE RD	West	U-Turn	0	0	0.01%	0	0	0	0	0.00%
OLD STATE RD	West	Total	1,013	897	0.01%	897	897	897	899	0.00%

**Growth Rate Notes**



**PROJECT TRAFFIC FORECAST REPORT**

**Location:** At Hillsdale Road, 2.04 miles N of SR 57

The table below contains the projected traffic volumes in each requested year for approach and movement during the morning and afternoon peak hour.

The morning and afternoon peak hours are those 60 minute periods during which the most vehicles pass through the intersection.

AM PM Peak Movement Forecast										
Approach Direction	Growth Rate	Movement	Interval	Total Vehicles	Commercial % AADT	Count Year AADT	Construction Year AADT 2020	Intermediate Year 1 AADT 2025	Intermediate Year 2 AADT 2030	Design Year AADT 2045
East	0.01%	Left	6:45 AM	21	0.00%	19	19	19	19	19
East	0.01%	Thru	6:45 AM	17	0.00%	15	15	15	15	15
East	0.01%	Right	6:45 AM	6	0.00%	5	5	5	5	5
East	0.01%	U-Turn	6:45 AM	0	0.00%	0	0	0	0	0
North	1.00%	Left	6:45 AM	3	0.00%	3	3	3	3	4
North	1.00%	Thru	6:45 AM	1029	13.99%	911	920	966	1,011	1,148
North	1.00%	Right	6:45 AM	43	9.30%	38	38	40	42	48
North	1.00%	U-Turn	6:45 AM	1	0.00%	1	1	1	1	1
South	1.00%	Left	6:45 AM	0	0.00%	0	0	0	0	0
South	1.00%	Thru	6:45 AM	971	10.92%	971	981	1,029	1,078	1,223
South	1.00%	Right	6:45 AM	11	0.00%	11	11	12	12	14
South	1.00%	U-Turn	6:45 AM	0	0.00%	0	0	0	0	0
West	0.01%	Left	6:45 AM	78	3.85%	69	69	69	69	69
West	0.01%	Thru	6:45 AM	28	0.00%	25	25	25	25	25
West	0.01%	Right	6:45 AM	7	0.00%	6	6	6	6	6
West	0.01%	U-Turn	6:45 AM	0	0.00%	0	0	0	0	0
East	0.01%	Left	4:00 PM	27	0.00%	24	24	24	24	24
East	0.01%	Thru	4:00 PM	21	0.00%	19	19	19	19	19
East	0.01%	Right	4:00 PM	5	0.00%	4	4	4	4	4
East	0.01%	U-Turn	4:00 PM	0	0.00%	0	0	0	0	0
North	1.00%	Left	4:00 PM	6	0.00%	5	5	5	6	6
North	1.00%	Thru	4:00 PM	1234	7.29%	1,092	1,103	1,158	1,212	1,376
North	1.00%	Right	4:00 PM	140	0.00%	124	125	131	138	156
North	1.00%	U-Turn	4:00 PM	1	0.00%	1	1	1	1	1
South	1.00%	Left	4:00 PM	6	0.00%	6	6	6	7	8
South	1.00%	Thru	4:00 PM	900	9.00%	900	909	954	999	1,134
South	1.00%	Right	4:00 PM	16	6.25%	16	16	17	18	20
South	1.00%	U-Turn	4:00 PM	0	0.00%	0	0	0	0	0
West	0.01%	Left	4:00 PM	42	0.00%	37	37	37	37	37
West	0.01%	Thru	4:00 PM	17	0.00%	15	15	15	15	15
West	0.01%	Right	4:00 PM	4	0.00%	4	4	4	4	4
West	0.01%	U-Turn	4:00 PM	0	0.00%	0	0	0	0	0

It should be recognized by users of this forecast that the base year AADT has an accuracy of plus or minus 10%. It should also be understood that while this report may include forecasts with up to six apparent significant figures, the accuracy should not be interpreted as being greater than two significant figures. It is the responsibility of designers to exercise professional judgement when using this data to influence decisions.



**PROJECT TRAFFIC FORECAST REPORT**

**DES No.:** 1400005

US-41 US 41 at Hillsdale Rd

From RP 10+90 to RP 11+1

Vanderburgh County

**Prepared For**

Matthew Bullock

**On**

06/03/2019

**By**

INDOT, Office of Traffic Statistics  
Technical Planning Support & Programming Division  
Gregory A. Katter, PE, Supervisor  
100 N. Senate Ave, N955  
Indianapolis, Indiana 46204  
[INDOTTrafficForecasts@indot.IN.gov](mailto:INDOTTrafficForecasts@indot.IN.gov)



PROJECT TRAFFIC FORECAST REPORT

Intersection: 1

VD US 41 and HILLSDALE RD. EVANSVILLW VANDER... - TMC

Mon Jan 25, 2016

Full Length (12 PM-6 PM, 6 AM-12 PM)

All Classes (Lig hts, Articulated Trucks, Buses and Single-Unit Trucks)

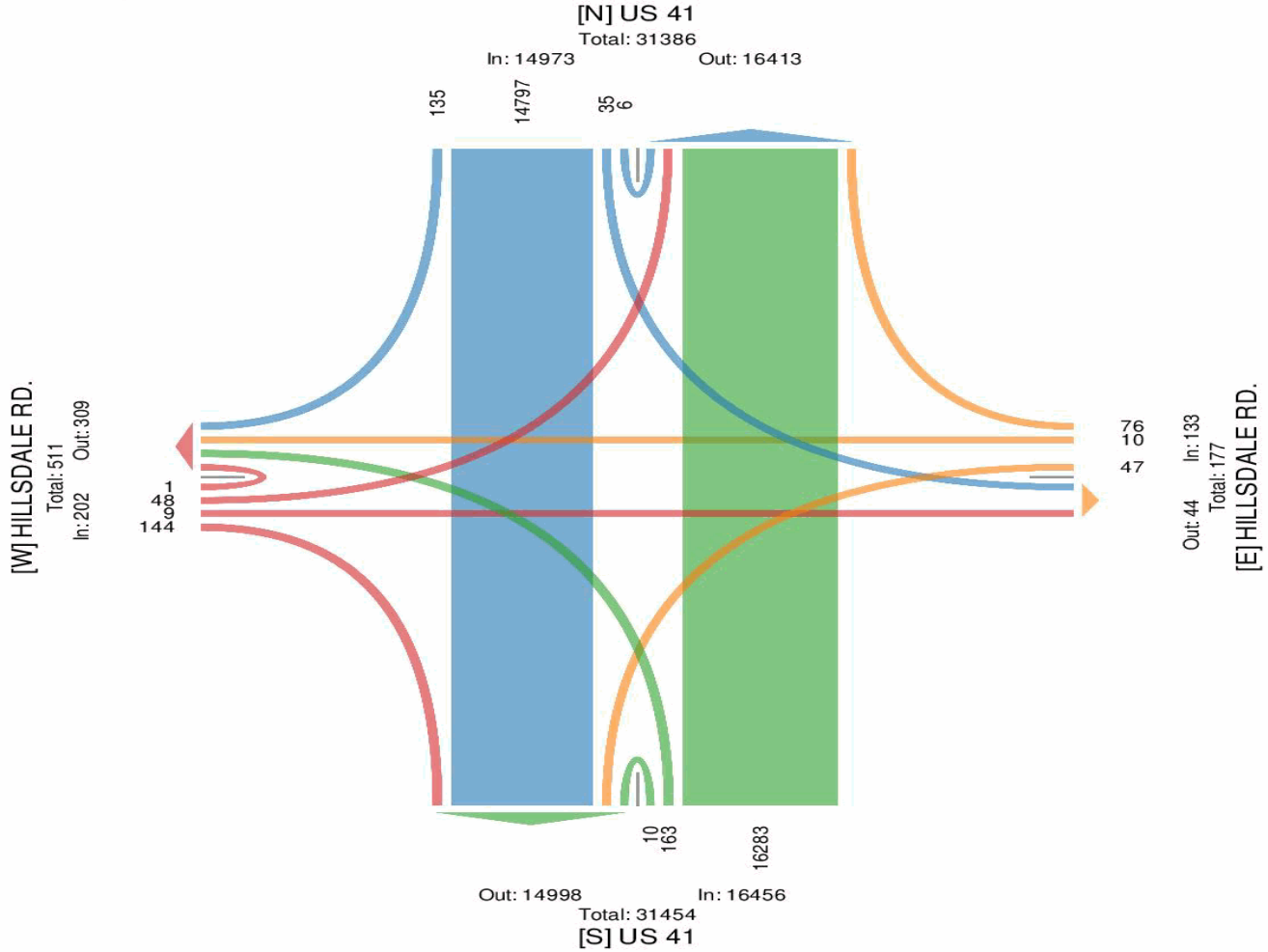
All Movements

ID: 288172, Location: 38.079492, -87.554662



Provided by: Indiana DOT

100 N. Senate Ave., Indianapolis, IN, 46204, US





## PROJECT TRAFFIC FORECAST REPORT

**Location:** US 41 at Hillsdale Rd

The table below contains the projected Annual Average Daily Traffic (AADT) in each requested year for each approach and movement.

The per year growth rate used for each approach is indicated in the table below. It is applied as a straight line growth.

For the purpose of this report a commercial vehicle would fall into FHWA Scheme F Classes 4 through 13. They are identified by MioVision as either an Articulated Truck, a Bus, or a Single-Unit Truck.

Daily Movement Forecast										
Approach Road Name	Approach Direction	Movement	Total	Count Year AADT	Growth Rate	Construction Year AADT 2020	Intermediate Year 1 AADT 2025	Intermediate Year 2 AADT 2030	Design Year AADT 2045	Commercial Percentage
HILLSDALE RD	East	Right	76	82	1.00%	85	90	94	106	0.00%
HILLSDALE RD	East	Thru	10	11	1.00%	11	12	12	14	0.00%
HILLSDALE RD	East	Left	47	51	1.00%	53	55	58	66	0.00%
HILLSDALE RD	East	Total	133	144	1.00%	150	157	164	185	0.00%
US 41	North	Right	135	146	1.00%	152	159	166	188	0.74%
US 41	North	Thru	14,797	15,996	1.00%	16,635	17,435	18,235	20,634	2.86%
US 41	North	Left	35	38	1.00%	39	41	43	49	0.00%
US 41	North	U-Turn	6	6	1.00%	7	7	7	8	0.00%
US 41	North	Total	14,973	16,186	1.00%	16,833	17,643	18,452	20,880	0.00%
US 41 SB	South	Left	163	176	1.00%	183	192	201	227	0.00%
US 41 SB	South	U-Turn	10	11	1.00%	11	12	12	14	0.00%
US 41 SB	South	Total	16,456	17,789	1.00%	18,500	19,390	20,279	22,948	0.00%
US 41 SB	South	Thru	16,283	17,602	1.00%	18,306	19,186	20,066	22,706	2.01%
HILLSDALE RD	West	Right	144	156	1.00%	162	170	177	201	0.00%
HILLSDALE RD	West	Thru	9	10	1.00%	10	11	11	13	0.00%
HILLSDALE RD	West	Left	48	52	1.00%	54	57	59	67	0.00%
HILLSDALE RD	West	U-Turn	1	1	1.00%	1	1	1	1	0.00%
HILLSDALE RD	West	Total	202	218	1.00%	227	238	249	282	0.00%

**Growth Rate Notes**





**PROJECT TRAFFIC FORECAST REPORT**

**Location:** US 41 at Hillsdale Rd

The table below contains the projected traffic volumes in each requested year for approach and movement during the morning and afternoon peak hour.

The morning and afternoon peak hours are those 60 minute periods during which the most vehicles pass through the intersection.

AM PM Peak Movement Forecast										
Approach Direction	Growth Rate	Movement	Interval	Total Vehicles	Commercial % AADT	Count Year AADT	Construction Year AADT 2020	Intermediate Year 1 AADT 2025	Intermediate Year 2 AADT 2030	Design Year AADT 2045
East	1.00%	Left	8:00 AM	2	0.00%	2	2	2	2	3
East	1.00%	Thru	8:00 AM	1	0.00%	1	1	1	1	1
East	1.00%	Right	8:00 AM	12	0.00%	13	14	14	15	17
North	1.00%	Left	8:00 AM	3	0.00%	3	3	3	3	4
North	1.00%	Thru	8:00 AM	775	6.32%	838	872	913	955	1,081
North	1.00%	Right	8:00 AM	8	0.00%	9	9	10	10	12
North	1.00%	U-Turn	8:00 AM	0	0.00%	0	0	0	0	0
South	1.00%	Left	8:00 AM	21	0.00%	23	24	25	26	30
South	1.00%	Thru	8:00 AM	2756	0.76%	2,979	3,098	3,247	3,396	3,843
South	1.00%	U-Turn	8:00 AM	0	0.00%	0	0	0	0	0
West	1.00%	Left	8:00 AM	9	0.00%	10	10	11	11	13
West	1.00%	Thru	8:00 AM	1	0.00%	1	1	1	1	1
West	1.00%	Right	8:00 AM	7	0.00%	8	8	9	9	10
West	1.00%	U-Turn	8:00 AM	0	0.00%	0	0	0	0	0
East	1.00%	Left	4:30 PM	5	0.00%	5	5	5	6	6
East	1.00%	Thru	4:30 PM	0	0.00%	0	0	0	0	0
East	1.00%	Right	4:30 PM	4	0.00%	4	4	4	5	5
North	1.00%	Left	4:30 PM	3	0.00%	3	3	3	3	4
North	1.00%	Thru	4:30 PM	2314	0.56%	2,501	2,601	2,726	2,851	3,226
North	1.00%	Right	4:30 PM	14	0.00%	15	16	16	17	19
North	1.00%	U-Turn	4:30 PM	2	0.00%	2	2	2	2	3
South	1.00%	Left	4:30 PM	22	0.00%	24	25	26	27	31
South	1.00%	Thru	4:30 PM	1351	2.00%	1,460	1,518	1,591	1,664	1,883
South	1.00%	U-Turn	4:30 PM	0	0.00%	0	0	0	0	0
West	1.00%	Left	4:30 PM	3	0.00%	3	3	3	3	4
West	1.00%	Thru	4:30 PM	1	0.00%	1	1	1	1	1
West	1.00%	Right	4:30 PM	17	0.00%	18	19	20	21	23
West	1.00%	U-Turn	4:30 PM	0	0.00%	0	0	0	0	0

It should be recognized by users of this forecast that the base year AADT has an accuracy of plus or minus 10%. It should also be understood that while this report may include forecasts with up to six apparent significant figures, the accuracy should not be interpreted as being greater than two significant figures. It is the responsibility of designers to exercise professional judgement when using this data to influence decisions.



**PROJECT TRAFFIC FORECAST REPORT**

**DES No.:** 1400005

US-41 Hillsdale Rd and Walnut Road

From RP 11+90 to RP 10+1

Vanderburgh County

**Prepared For**

Matthew Bullock

**On**

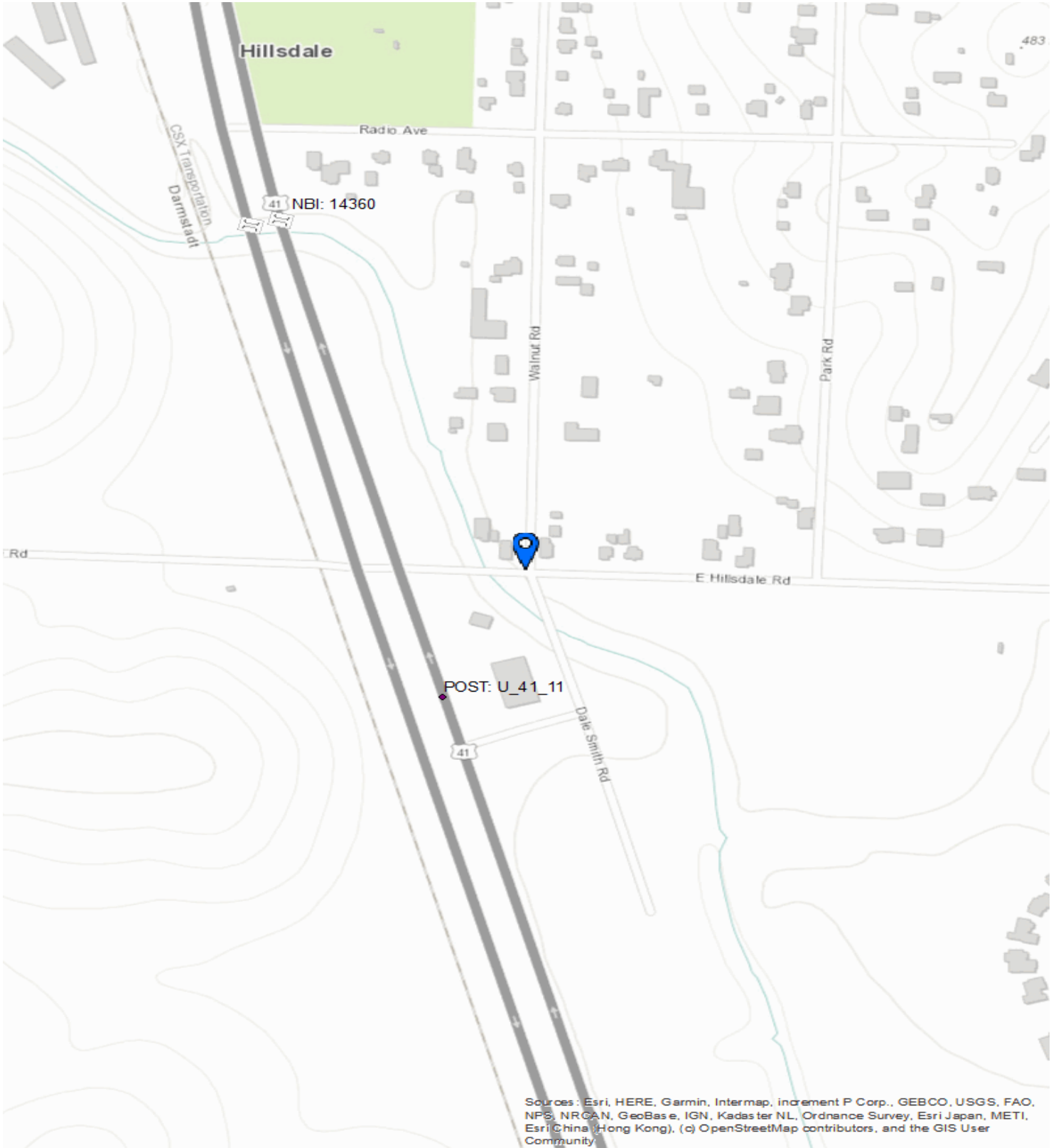
06/03/2019

**By**

INDOT, Office of Traffic Statistics  
Technical Planning Support & Programming Division  
Gregory A. Katter, PE, Supervisor  
100 N. Senate Ave, N955  
Indianapolis, Indiana 46204  
INDOTTrafficForecasts@indot.IN.gov



PROJECT TRAFFIC FORECAST REPORT



Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), (c) OpenStreetMap contributors, and the GIS User Community



PROJECT TRAFFIC FORECAST REPORT

Intersection: 1

EMPO/INDOT 82S001, 82S002, 82S003, 82S004 - TMC

Tue Jun 11, 2019

Full Length (6 AM-6 AM(+1))

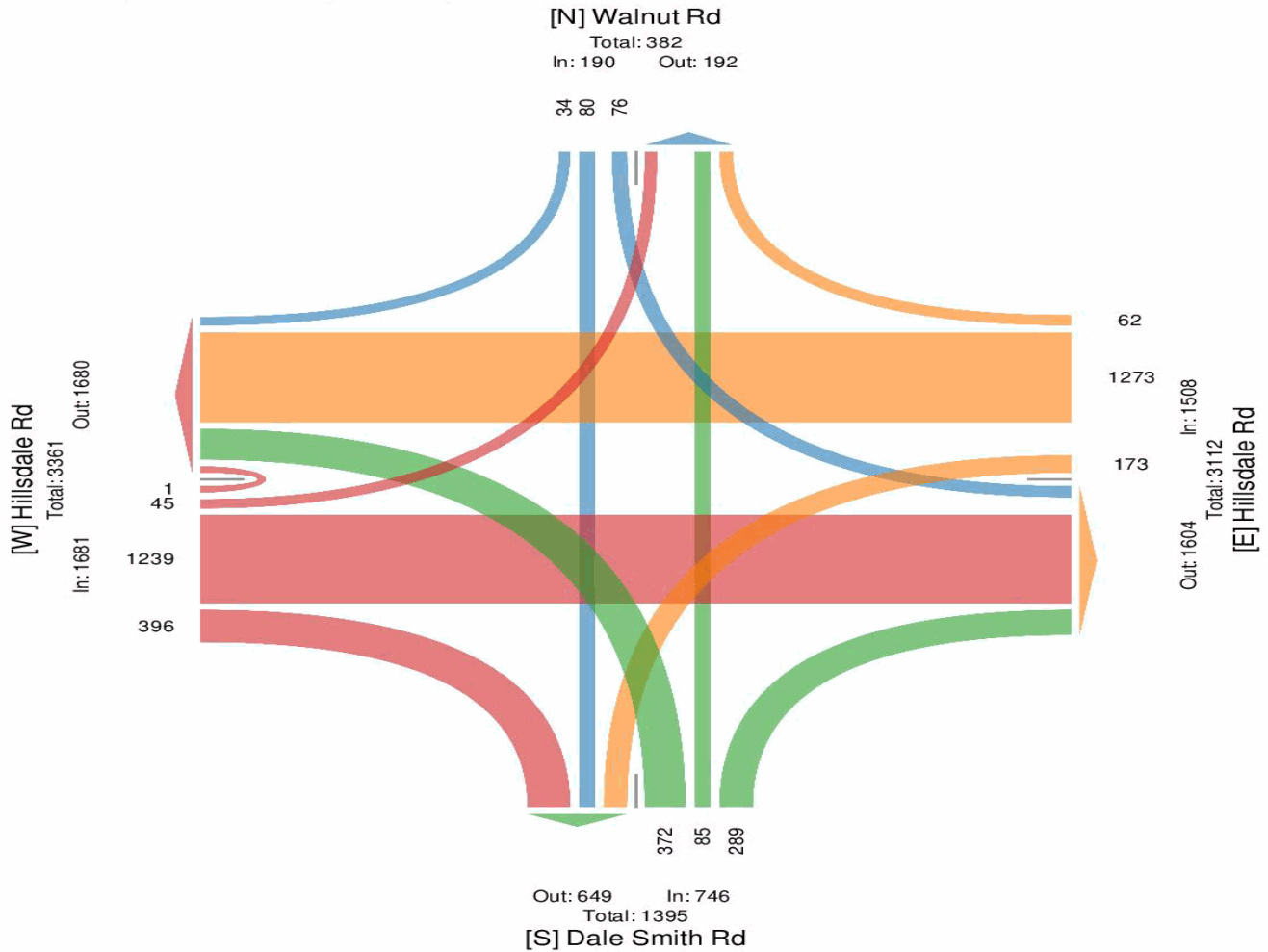
All Classes (Motorcycles, Cars, Light Goods Vehicles, Single-Unit Trucks, Articulated Trucks, Buses)

All Movements

ID: 667438, Location: 38.079445, -87.553619, Site Code: 82S001



Provided by: Indiana DOT  
100 N. Senate Ave.,  
Indianapolis, IN, 46204, US





**PROJECT TRAFFIC FORECAST REPORT**

**Location:** Hillsdale Rd and Walnut Road

The table below contains the projected Annual Average Daily Traffic (AADT) in each requested year for each approach and movement.

The per year growth rate used for each approach is indicated in the table below. It is applied as a straight line growth.

For the purpose of this report a commercial vehicle would fall into FHWA Scheme F Classes 4 through 13. They are identified by MioVision as either an Articulated Truck, a Bus, or a Single-Unit Truck.

Daily Movement Forecast										
Approach Road Name	Approach Direction	Movement	Total	Count Year AADT	Growth Rate	Construction Year AADT 2020	Intermediate Year 1 AADT 2025	Intermediate Year 2 AADT 2030	Design Year AADT 2045	Commercial Percentage
HILLSDALE RD	East	Right	62	58	0.00%	58	58	58	58	1.61%
HILLSDALE RD	East	Thru	1,273	1,183	0.00%	1,183	1,183	1,183	1,183	1.65%
HILLSDALE RD	East	Left	173	161	0.00%	161	161	161	161	1.73%
HILLSDALE RD	East	U-Turn	0	0	0.00%	0	0	0	0	0.00%
HILLSDALE RD	East	Total	1,508	1,401	0.00%	1,401	1,401	1,401	1,401	0.00%
WALNUT RD	North	Right	34	32	0.00%	32	32	32	32	2.94%
WALNUT RD	North	Thru	80	74	0.00%	74	74	74	74	3.75%
WALNUT RD	North	Left	76	71	0.00%	71	71	71	71	0.00%
WALNUT RD	North	U-Turn	0	0	0.00%	0	0	0	0	0.00%
WALNUT RD	North	Total	190	177	0.00%	177	177	177	177	0.00%
WALNUT RD	South	Right	289	268	0.00%	268	268	268	268	2.08%
WALNUT RD	South	Thru	85	79	0.00%	79	79	79	79	3.53%
WALNUT RD	South	Left	372	346	0.00%	346	346	346	346	2.42%
WALNUT RD	South	U-Turn	0	0	0.00%	0	0	0	0	0.00%
WALNUT RD	South	Total	746	693	0.00%	693	693	693	693	0.00%
HILLSDALE RD	West	Right	396	368	0.00%	368	368	368	368	3.28%
HILLSDALE RD	West	Thru	1,239	1,151	0.00%	1,151	1,151	1,151	1,151	1.29%
HILLSDALE RD	West	Left	45	42	0.00%	42	42	42	42	4.44%
HILLSDALE RD	West	U-Turn	1	1	0.00%	1	1	1	1	0.00%
HILLSDALE RD	West	Total	1,681	1,562	0.00%	1,562	1,562	1,562	1,562	0.00%

**Growth Rate Notes**



**PROJECT TRAFFIC FORECAST REPORT**

**Location:** Hillsdale Rd and Walnut Road

The table below contains the projected traffic volumes in each requested year for approach and movement during the morning and afternoon peak hour.

The morning and afternoon peak hours are those 60 minute periods during which the most vehicles pass through the intersection.

AM PM Peak Movement Forecast										
Approach Direction	Growth Rate	Movement	Interval	Total Vehicles	Commercial % AADT	Count Year AADT	Construction Year AADT 2020	Intermediate Year 1 AADT 2025	Intermediate Year 2 AADT 2030	Design Year AADT 2045
East	0.00%	Left	7:00 AM	13	0.00%	12	12	12	12	12
East	0.00%	Thru	7:00 AM	111	0.00%	103	103	103	103	103
East	0.00%	Right	7:00 AM	4	0.00%	4	4	4	4	4
East	0.00%	U-Turn	7:00 AM	0	0.00%	0	0	0	0	0
North	0.00%	Left	7:00 AM	4	0.00%	4	4	4	4	4
North	0.00%	Thru	7:00 AM	5	0.00%	5	5	5	5	5
North	0.00%	Right	7:00 AM	2	0.00%	2	2	2	2	2
North	0.00%	U-Turn	7:00 AM	0	0.00%	0	0	0	0	0
South	0.00%	Left	7:00 AM	24	0.00%	22	22	22	22	22
South	0.00%	Thru	7:00 AM	5	0.00%	5	5	5	5	5
South	0.00%	Right	7:00 AM	21	9.52%	20	20	20	20	20
South	0.00%	U-Turn	7:00 AM	0	0.00%	0	0	0	0	0
West	0.00%	Left	7:00 AM	1	0.00%	1	1	1	1	1
West	0.00%	Thru	7:00 AM	77	1.30%	72	72	72	72	72
West	0.00%	Right	7:00 AM	24	8.33%	22	22	22	22	22
West	0.00%	U-Turn	7:00 AM	0	0.00%	0	0	0	0	0
East	0.00%	Left	4:45 PM	10	0.00%	9	9	9	9	9
East	0.00%	Thru	4:45 PM	92	1.09%	85	85	85	85	85
East	0.00%	Right	4:45 PM	22	4.55%	20	20	20	20	20
East	0.00%	U-Turn	4:45 PM	0	0.00%	0	0	0	0	0
North	0.00%	Left	4:45 PM	6	0.00%	6	6	6	6	6
North	0.00%	Thru	4:45 PM	7	0.00%	7	7	7	7	7
North	0.00%	Right	4:45 PM	2	0.00%	2	2	2	2	2
North	0.00%	U-Turn	4:45 PM	0	0.00%	0	0	0	0	0
South	0.00%	Left	4:45 PM	30	0.00%	28	28	28	28	28
South	0.00%	Thru	4:45 PM	10	0.00%	9	9	9	9	9
South	0.00%	Right	4:45 PM	32	0.00%	30	30	30	30	30
South	0.00%	U-Turn	4:45 PM	0	0.00%	0	0	0	0	0
West	0.00%	Left	4:45 PM	12	0.00%	11	11	11	11	11
West	0.00%	Thru	4:45 PM	154	0.00%	143	143	143	143	143
West	0.00%	Right	4:45 PM	24	0.00%	22	22	22	22	22
West	0.00%	U-Turn	4:45 PM	0	0.00%	0	0	0	0	0

It should be recognized by users of this forecast that the base year AADT has an accuracy of plus or minus 10%. It should also be understood that while this report may include forecasts with up to six apparent significant figures, the accuracy should not be interpreted as being greater than two significant figures. It is the responsibility of designers to exercise professional judgement when using this data to influence decisions.



**PROJECT TRAFFIC FORECAST REPORT**

**DES No.:** 1400005

US-41 Campbell Rd and Old State Rd

From RP 11+90 to RP 10+1

Vanderburgh County

**Prepared For**

Matthew Bullock

**On**

06/03/2019

**By**

INDOT, Office of Traffic Statistics  
Technical Planning Support & Programming Division  
Gregory A. Katter, PE, Supervisor  
100 N. Senate Ave, N955  
Indianapolis, Indiana 46204  
[INDOTTrafficForecasts@indot.IN.gov](mailto:INDOTTrafficForecasts@indot.IN.gov)





PROJECT TRAFFIC FORECAST REPORT



Sources: Esri, HERE, Garmin, Intermap, increment P Corp., GEBCO, USGS, FAO, NPS, NRCAN, GeoBase, IGN, Kadaster NL, Ordnance Survey, Esri Japan, METI, Esri China (Hong Kong), (c) OpenStreetMap contributors, and the GIS User Community





PROJECT TRAFFIC FORECAST REPORT

Intersection: 1

EMPO/INDOT 82S005, 82S006, 82S007 - TMC

Tue Jun 11, 2019

Full Length (6 AM-6 AM(+1))

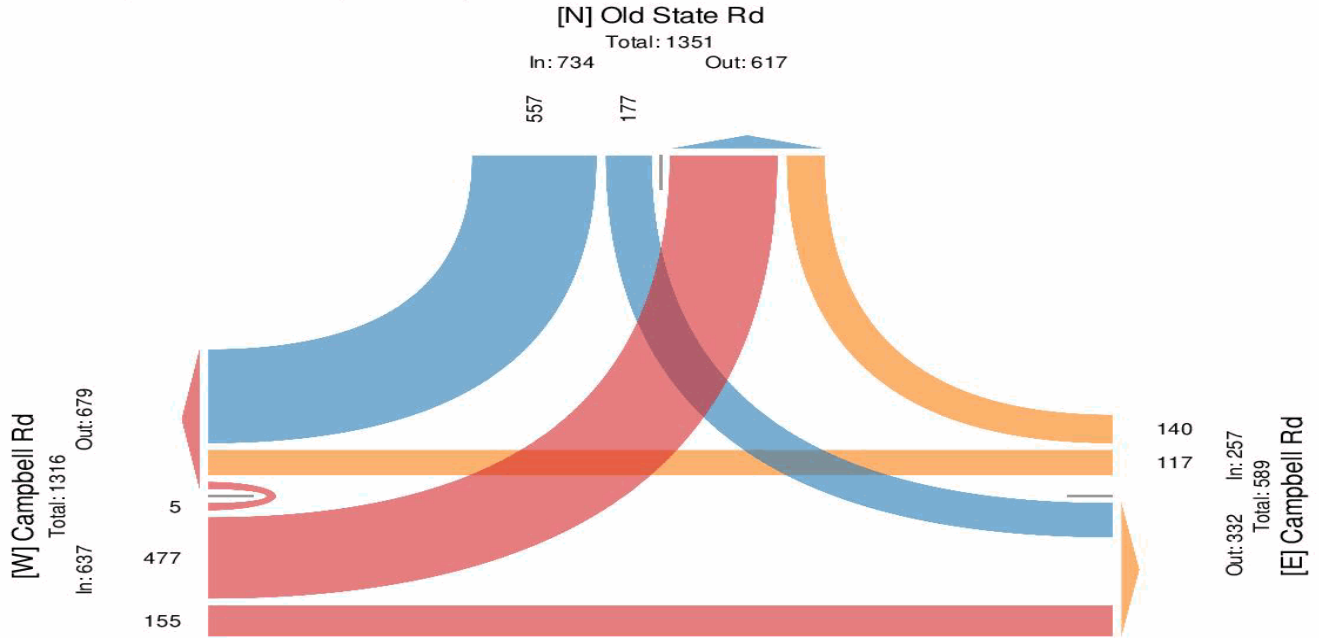
All Classes (Motorcycles, Cars, Light Goods Vehicles, Single-Unit Trucks, Articulated Trucks, Buses)

All Movements

ID: 667442, Location: 38.086617, -87.556025, Site Code: 82S005



Provided by: Indiana DOT  
100 N. Senate Ave.,  
Indianapolis, IN, 46204, US





**PROJECT TRAFFIC FORECAST REPORT**

**Location:** Campbell Rd and Old State Rd

The table below contains the projected Annual Average Daily Traffic (AADT) in each requested year for each approach and movement.

The per year growth rate used for each approach is indicated in the table below. It is applied as a straight line growth.

For the purpose of this report a commercial vehicle would fall into FHWA Scheme F Classes 4 through 13. They are identified by MioVision as either an Articulated Truck, a Bus, or a Single-Unit Truck.

Daily Movement Forecast										
Approach Road Name	Approach Direction	Movement	Total	Count Year AADT	Growth Rate	Construction Year AADT 2020	Intermediate Year 1 AADT 2025	Intermediate Year 2 AADT 2035	Design Year AADT 2045	Commercial Percentage
CAMPBELL RD	East	Right	140	130	0.00%	130	130	130	130	2.86%
CAMPBELL RD	East	Thru	117	109	0.00%	109	109	109	109	2.56%
CAMPBELL RD	East	U-Turn	0	0	0.00%	0	0	0	0	0.00%
CAMPBELL RD	East	Total	257	239	0.00%	239	239	239	239	0.00%
OLD STATE RD SB	North	Right	557	517	0.00%	517	517	517	517	0.90%
OLD STATE RD SB	North	Left	177	164	0.00%	164	164	164	164	1.70%
OLD STATE RD SB	North	U-Turn	0	0	0.00%	0	0	0	0	0.00%
OLD STATE RD SB	North	Total	734	682	0.00%	682	682	682	682	0.00%
OLD STATE RD SB	West	Thru	155	144	0.00%	144	144	144	144	1.29%
OLD STATE RD SB	West	Left	477	443	0.00%	443	443	443	443	0.84%
OLD STATE RD SB	West	U-Turn	5	5	0.00%	5	5	5	5	0.00%
OLD STATE RD SB	West	Total	637	592	0.00%	592	592	592	592	0.00%

**Growth Rate Notes**



**PROJECT TRAFFIC FORECAST REPORT**

**Location:** Campbell Rd and Old State Rd

The table below contains the projected traffic volumes in each requested year for approach and movement during the morning and afternoon peak hour.

The morning and afternoon peak hours are those 60 minute periods during which the most vehicles pass through the intersection.

AM PM Peak Movement Forecast										
Approach Direction	Growth Rate	Movement	Interval	Total Vehicles	Commercial % AADT	Count Year AADT	Construction Year AADT 2020	Intermediate Year 1 AADT 2025	Intermediate Year 2 AADT 2035	Design Year AADT 2045
East	0.00%	Thru	9:15 AM	12	0.00%	11	11	11	11	11
East	0.00%	Right	9:15 AM	18	0.00%	17	17	17	17	17
East	0.00%	U-Turn	9:15 AM	0	0.00%	0	0	0	0	0
North	0.00%	Left	9:15 AM	15	6.67%	14	14	14	14	14
North	0.00%	Right	9:15 AM	69	4.35%	64	64	64	64	64
North	0.00%	U-Turn	9:15 AM	0	0.00%	0	0	0	0	0
West	0.00%	Left	9:15 AM	40	2.50%	37	37	37	37	37
West	0.00%	Thru	9:15 AM	27	0.00%	25	25	25	25	25
West	0.00%	U-Turn	9:15 AM	0	0.00%	0	0	0	0	0
East	0.00%	Thru	5:00 PM	10	0.00%	9	9	9	9	9
East	0.00%	Right	5:00 PM	7	0.00%	7	7	7	7	7
East	0.00%	U-Turn	5:00 PM	0	0.00%	0	0	0	0	0
North	0.00%	Left	5:00 PM	28	0.00%	26	26	26	26	26
North	0.00%	Right	5:00 PM	35	0.00%	33	33	33	33	33
North	0.00%	U-Turn	5:00 PM	0	0.00%	0	0	0	0	0
West	0.00%	Left	5:00 PM	47	0.00%	44	44	44	44	44
West	0.00%	Thru	5:00 PM	18	0.00%	17	17	17	17	17
West	0.00%	U-Turn	5:00 PM	3	0.00%	3	3	3	3	3

It should be recognized by users of this forecast that the base year AADT has an accuracy of plus or minus 10%. It should also be understood that while this report may include forecasts with up to six apparent significant figures, the accuracy should not be interpreted as being greater than two significant figures. It is the responsibility of designers to exercise professional judgement when using this data to influence decisions.

**EMPO/INDOT US 41 @ Hillsdale Rd - TMC**

Tue Jul 16, 2019

Full Length (6 AM-6 AM (+1))

All Classes (Motorcycles, Cars, Light Goods Vehicles, Single-Unit Trucks, Articulated Trucks, Buses)

All Movements

ID: 679143, Location: 38.079492, -87.554662



Provided by: Indiana DOT  
100 N. Senate Ave.,  
Indianapolis, IN, 46204, US

Leg Direction	Hillsdale Rd Westbound				US 41 Northbound				Hillsdale Rd Eastbound				Int
	R	T	U	App	R	T	L	App	T	L	U	App	
2019-07-16 6:00AM	7	9	0	16	4	132	0	136	2	0	0	2	154
6:15AM	12	9	0	21	1	187	4	192	6	0	0	6	219
6:30AM	9	23	0	32	1	195	2	198	13	0	0	13	243
6:45AM	10	20	0	30	3	162	8	173	9	0	0	9	212
Hourly Total	38	61	0	99	9	676	14	699	30	0	0	30	828
7:00AM	9	16	0	25	7	128	3	138	9	0	0	9	172
7:15AM	13	33	1	47	6	157	3	166	11	0	0	11	224
7:30AM	11	24	0	35	2	172	3	177	15	0	0	15	227
7:45AM	8	28	0	36	10	168	2	180	21	0	0	21	237
Hourly Total	41	101	1	143	25	625	11	661	56	0	0	56	860
8:00AM	8	20	0	28	3	142	3	148	7	1	0	8	184
8:15AM	6	14	0	20	1	140	2	143	7	0	0	7	170
8:30AM	10	17	0	27	5	146	3	154	12	0	0	12	193
8:45AM	8	14	0	22	10	146	2	158	24	1	0	25	205
Hourly Total	32	65	0	97	19	574	10	603	50	2	0	52	752
9:00AM	11	15	0	26	5	127	3	135	7	0	0	7	168
9:15AM	4	15	0	19	7	156	1	164	8	0	0	8	191
9:30AM	8	13	0	21	10	142	6	158	9	0	0	9	188
9:45AM	7	17	0	24	6	158	2	166	4	1	0	5	195
Hourly Total	30	60	0	90	28	583	12	623	28	1	0	29	742
10:00AM	6	10	0	16	7	138	6	151	6	0	0	6	173
10:15AM	9	13	0	22	6	173	2	181	9	1	0	10	213
10:30AM	6	17	0	23	9	138	2	149	13	0	0	13	185
10:45AM	8	15	0	23	13	143	3	159	19	0	0	19	201
Hourly Total	29	55	0	84	35	592	13	640	47	1	0	48	772
11:00AM	13	13	0	26	10	158	1	169	9	0	0	9	204
11:15AM	10	8	0	18	8	164	1	173	14	0	0	14	205
11:30AM	18	17	0	35	7	170	1	178	8	0	0	8	221
11:45AM	16	12	0	28	8	187	1	196	10	1	0	11	235
Hourly Total	57	50	0	107	33	679	4	716	41	1	0	42	865
12:00PM	4	24	0	28	9	188	3	200	15	1	0	16	244
12:15PM	7	10	0	17	6	170	3	179	13	0	0	13	209
12:30PM	5	17	0	22	9	170	7	186	13	0	0	13	221
12:45PM	8	9	0	17	4	136	0	140	19	3	0	22	179
Hourly Total	24	60	0	84	28	664	13	705	60	4	0	64	853
1:00PM	9	13	0	22	6	157	8	171	21	0	0	21	214
1:15PM	11	19	0	30	6	175	3	184	8	0	0	8	222
1:30PM	8	13	0	21	5	158	2	165	11	0	0	11	197
1:45PM	13	24	0	37	4	100	3	107	11	0	0	11	155
Hourly Total	41	69	0	110	21	590	16	627	51	0	0	51	788
2:00PM	5	11	0	16	11	185	1	197	4	0	0	4	217
2:15PM	9	6	0	15	6	212	0	218	9	0	0	9	242
2:30PM	6	13	0	19	7	176	2	185	9	0	0	9	213
2:45PM	4	12	0	16	6	163	3	172	6	0	0	6	194
Hourly Total	24	42	0	66	30	736	6	772	28	0	0	28	866
3:00PM	5	18	0	23	8	193	5	206	15	0	0	15	244
3:15PM	16	5	0	21	6	187	3	196	11	0	0	11	228
3:30PM	11	18	0	29	8	214	1	223	12	2	0	14	266
3:45PM	7	15	0	22	11	238	6	255	23	1	0	24	301
Hourly Total	39	56	0	95	33	832	15	880	61	3	0	64	1039
4:00PM	11	13	0	24	13	240	4	257	18	0	0	18	299
4:15PM	15	16	0	31	10	257	9	276	20	0	0	20	327
4:30PM	9	14	0	23	9	231	6	246	21	0	0	21	290

Leg Direction	Hillsdale Rd Westbound				US 41 Northbound				Hillsdale Rd Eastbound				
Time	R	T	U	App	R	T	L	App	T	L	U	App	Int
4:45PM	10	8	0	18	13	221	4	238	20	1	0	21	277
Hourly Total	45	51	0	96	45	949	23	1017	79	1	0	80	1193
5:00PM	6	10	0	16	15	240	6	261	31	1	0	32	309
5:15PM	15	16	0	31	10	246	1	257	18	0	0	18	306
5:30PM	15	13	0	28	14	238	3	255	13	0	0	13	296
5:45PM	9	12	0	21	17	170	3	190	18	0	0	18	229
Hourly Total	45	51	0	96	56	894	13	963	80	1	0	81	1140
6:00PM	13	10	0	23	8	174	2	184	18	0	0	18	225
6:15PM	6	10	0	16	6	158	1	165	11	0	0	11	192
6:30PM	5	18	0	23	9	135	3	147	14	0	0	14	184
6:45PM	9	8	0	17	9	134	2	145	15	0	0	15	177
Hourly Total	33	46	0	79	32	601	8	641	58	0	0	58	778
7:00PM	6	12	0	18	7	130	3	140	17	1	0	18	176
7:15PM	9	17	0	26	1	166	4	171	18	0	0	18	215
7:30PM	4	11	0	15	7	121	2	130	10	1	0	11	156
7:45PM	4	7	0	11	4	149	4	157	8	0	0	8	176
Hourly Total	23	47	0	70	19	566	13	598	53	2	0	55	723
8:00PM	5	10	0	15	5	143	1	149	12	0	0	12	176
8:15PM	6	9	0	15	14	129	2	145	17	1	0	18	178
8:30PM	4	5	0	9	8	125	3	136	12	0	0	12	157
8:45PM	0	8	0	8	9	81	2	92	8	0	0	8	108
Hourly Total	15	32	0	47	36	478	8	522	49	1	0	50	619
9:00PM	4	7	0	11	5	87	0	92	10	0	0	10	113
9:15PM	2	3	0	5	2	78	2	82	6	0	0	6	93
9:30PM	3	4	0	7	2	99	3	104	4	0	0	4	115
9:45PM	6	3	0	9	6	60	1	67	0	0	0	0	76
Hourly Total	15	17	0	32	15	324	6	345	20	0	0	20	397
10:00PM	2	1	0	3	4	40	2	46	3	0	0	3	52
10:15PM	2	1	0	3	2	58	3	63	4	0	0	4	70
10:30PM	1	3	0	4	1	27	1	29	3	0	0	3	36
10:45PM	2	6	0	8	1	42	1	44	0	0	0	0	52
Hourly Total	7	11	0	18	8	167	7	182	10	0	0	10	210
11:00PM	0	2	0	2	0	40	2	42	1	0	0	1	45
11:15PM	0	1	0	1	2	33	1	36	0	0	0	0	37
11:30PM	0	2	0	2	3	32	0	35	3	0	0	3	40
11:45PM	1	3	0	4	0	33	1	34	2	0	0	2	40
Hourly Total	1	8	0	9	5	138	4	147	6	0	0	6	162
2019-07-17 12:00AM	1	0	0	1	2	19	0	21	0	0	0	0	22
12:15AM	1	0	0	1	1	19	2	22	2	1	0	3	26
12:30AM	0	0	0	0	0	17	0	17	1	0	0	1	18
12:45AM	0	2	0	2	0	15	1	16	3	0	0	3	21
Hourly Total	2	2	0	4	3	70	3	76	6	1	0	7	87
1:00AM	1	1	0	2	0	18	1	19	1	0	0	1	22
1:15AM	0	1	0	1	1	20	0	21	2	0	0	2	24
1:30AM	1	0	0	1	1	19	0	20	1	0	0	1	22
1:45AM	0	0	0	0	0	13	0	13	0	0	0	0	13
Hourly Total	2	2	0	4	2	70	1	73	4	0	0	4	81
2:00AM	0	1	0	1	1	13	0	14	0	0	0	0	15
2:15AM	0	0	0	0	0	13	1	14	0	0	0	0	14
2:30AM	0	0	0	0	1	13	0	14	0	0	0	0	14
2:45AM	0	1	0	1	1	17	0	18	1	0	0	1	20
Hourly Total	0	2	0	2	3	56	1	60	1	0	0	1	63
3:00AM	2	0	0	2	0	27	0	27	2	0	0	2	31
3:15AM	0	4	0	4	0	22	0	22	0	0	0	0	26
3:30AM	1	0	0	1	0	20	0	20	0	0	0	0	21
3:45AM	0	2	0	2	0	26	1	27	2	0	0	2	31
Hourly Total	3	6	0	9	0	95	1	96	4	0	0	4	109
4:00AM	1	1	0	2	0	54	0	54	0	0	0	0	56
4:15AM	4	1	0	5	2	51	0	53	1	0	0	1	59
4:30AM	0	1	0	1	0	57	1	58	3	0	0	3	62

Leg Direction	Hillsdale Rd Westbound				US 41 Northbound				Hillsdale Rd Eastbound				Int
	R	T	U	App	R	T	L	App	T	L	U	App	
4:45AM	6	3	0	9	0	74	0	74	0	0	0	0	83
Hourly Total	11	6	0	17	2	236	1	239	4	0	0	4	260
5:00AM	5	8	0	13	0	97	1	98	4	0	0	4	115
5:15AM	10	6	0	16	1	125	1	127	3	0	0	3	146
5:30AM	8	11	0	19	0	137	4	141	9	1	0	10	170
5:45AM	6	8	0	14	2	124	0	126	6	1	0	7	147
Hourly Total	29	33	0	62	3	483	6	492	22	2	0	24	578
<b>Total</b>	586	933	1	1520	490	11678	209	12377	848	20	0	868	14765
<b>% Approach</b>	38.6%	61.4%	0.1%	-	4.0%	94.4%	1.7%	-	97.7%	2.3%	0%	-	-
<b>% Total</b>	4.0%	6.3%	0%	10.3%	3.3%	79.1%	1.4%	83.8%	5.7%	0.1%	0%	5.9%	-
<b>Motorcycles</b>	2	0	0	2	0	10	0	10	1	0	0	1	13
<b>% Motorcycles</b>	0.3%	0%	0%	0.1%	0%	0.1%	0%	0.1%	0.1%	0%	0%	0.1%	0.1%
<b>Cars</b>	456	690	0	1146	412	7860	136	8408	602	17	0	619	10173
<b>% Cars</b>	77.8%	74.0%	0%	75.4%	84.1%	67.3%	65.1%	67.9%	71.0%	85.0%	0%	71.3%	68.9%
<b>Light Goods Vehicles</b>	118	234	1	353	72	2246	60	2378	224	3	0	227	2958
<b>% Light Goods Vehicles</b>	20.1%	25.1%	100%	23.2%	14.7%	19.2%	28.7%	19.2%	26.4%	15.0%	0%	26.2%	20.0%
<b>Single-Unit Trucks</b>	9	6	0	15	5	463	9	477	13	0	0	13	505
<b>% Single-Unit Trucks</b>	1.5%	0.6%	0%	1.0%	1.0%	4.0%	4.3%	3.9%	1.5%	0%	0%	1.5%	3.4%
<b>Articulate d Trucks</b>	1	2	0	3	1	1077	4	1082	8	0	0	8	1093
<b>% Articulate d Trucks</b>	0.2%	0.2%	0%	0.2%	0.2%	9.2%	1.9%	8.7%	0.9%	0%	0%	0.9%	7.4%
<b>Buses</b>	0	1	0	1	0	22	0	22	0	0	0	0	23
<b>% Buses</b>	0%	0.1%	0%	0.1%	0%	0.2%	0%	0.2%	0%	0%	0%	0%	0.2%

\*L: Left, R: Right, T: Thru, U: U-Turn

**EMPO/INDOT US 41 @ Hillsdale Rd - TMC**

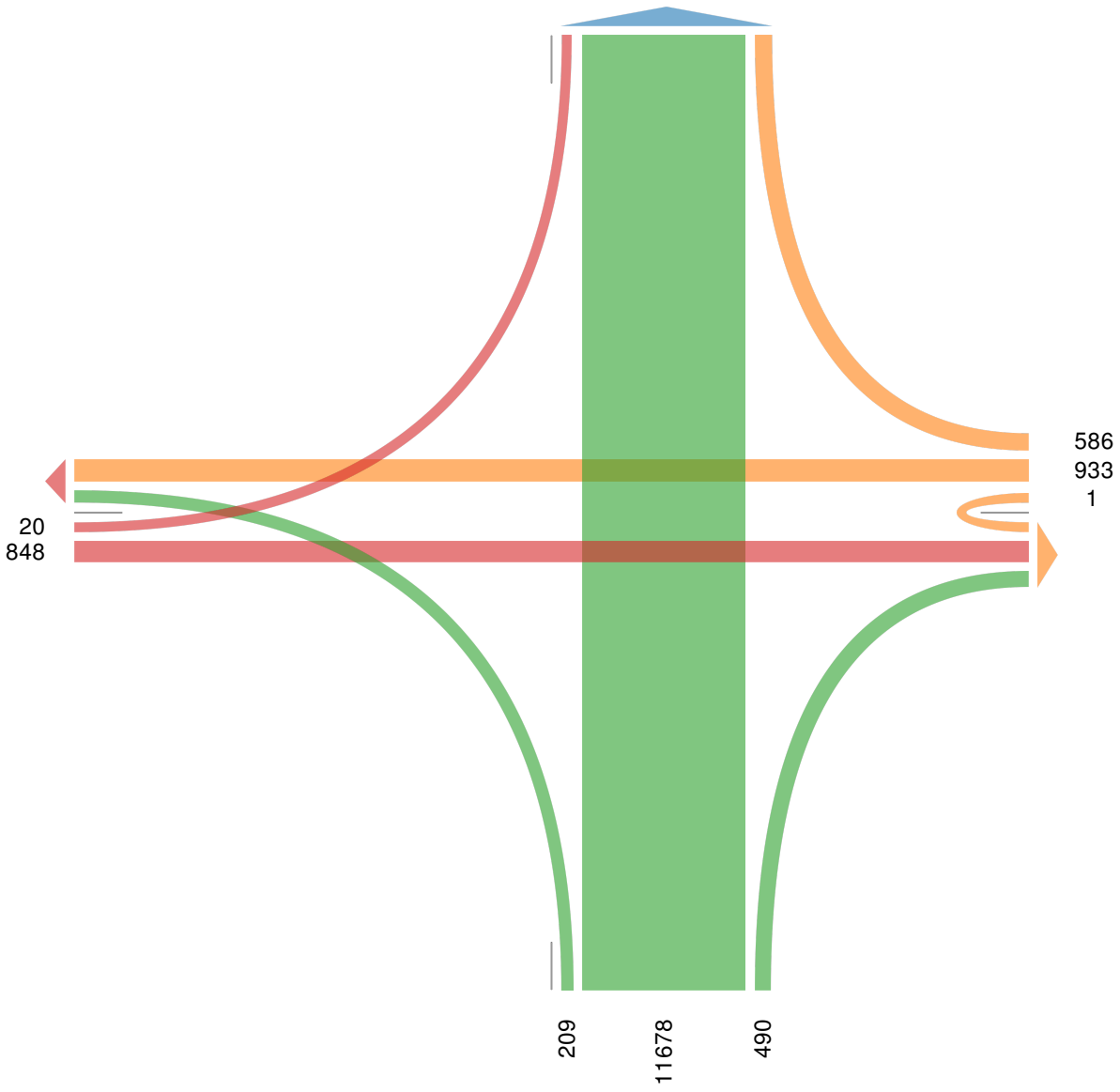
Tue Jul 16, 2019  
 Full Length (6 AM-6 AM (+1))  
 All Classes (Motorcycles, Cars, Light Goods Vehicles, Single-Unit Trucks,  
 Articulated Trucks, Buses)  
 All Movements  
 ID: 679143, Location: 38.079492, -87.554662



Provided by: Indiana DOT  
 100 N. Senate Ave.,  
 Indianapolis, IN, 46204, US

[N] US 41  
 Total: 12284  
 In: 0 Out: 12284

[W] Hillsdale Rd  
 Total: 2010  
 In: 868 Out: 1142



Out: 1339 In: 1520  
 Total: 2859  
 [E] Hillsdale Rd

Out: 0 In: 12377  
 Total: 12377  
 [S] US 41

**EMPO/INDOT US 41 @ Hillsdale Rd - TMC**

Tue Jul 16, 2019

AM Peak (Jul 16 2019 7:15AM - 8:15 AM)

All Classes (Motorcycles, Cars, Light Goods Vehicles, Single-Unit Trucks, Articulated Trucks, Buses)

All Movements

ID: 679143, Location: 38.079492, -87.554662



Provided by: Indiana DOT  
100 N. Senate Ave.,  
Indianapolis, IN, 46204, US

Leg Direction	Hillsdale Rd Westbound				US 41 Northbound				Hillsdale Rd Eastbound				
Time	R	T	U	App	R	T	L	App	T	L	U	App	Int
2019-07-16 7:15AM	13	33	1	47	6	157	3	166	11	0	0	11	224
7:30AM	11	24	0	35	2	172	3	177	15	0	0	15	227
7:45AM	8	28	0	36	10	168	2	180	21	0	0	21	237
8:00AM	8	20	0	28	3	142	3	148	7	1	0	8	184
<b>Total</b>	40	105	1	146	21	639	11	671	54	1	0	55	872
<b>% Approach</b>	27.4%	71.9%	0.7%	-	3.1%	95.2%	1.6%	-	98.2%	1.8%	0%	-	-
<b>% Total</b>	4.6%	12.0%	0.1%	16.7%	2.4%	73.3%	1.3%	76.9%	6.2%	0.1%	0%	6.3%	-
<b>PHF</b>	0.769	0.795	0.250	0.777	0.525	0.929	0.917	0.932	0.643	0.250	-	0.655	0.920
<b>Motorcycles</b>	0	0	0	0	0	0	0	0	0	0	0	0	0
<b>% Motorcycles</b>	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
<b>Cars</b>	29	82	0	111	14	396	7	417	38	1	0	39	567
<b>% Cars</b>	72.5%	78.1%	0%	76.0%	66.7%	62.0%	63.6%	62.1%	70.4%	100%	0%	70.9%	65.0%
<b>Light Goods Vehicles</b>	10	23	1	34	6	144	2	152	15	0	0	15	201
<b>% Light Goods Vehicles</b>	25.0%	21.9%	100%	23.3%	28.6%	22.5%	18.2%	22.7%	27.8%	0%	0%	27.3%	23.1%
<b>Single-Unit Trucks</b>	1	0	0	1	1	47	1	49	0	0	0	0	50
<b>% Single-Unit Trucks</b>	2.5%	0%	0%	0.7%	4.8%	7.4%	9.1%	7.3%	0%	0%	0%	0%	5.7%
<b>Articulated Trucks</b>	0	0	0	0	0	50	1	51	1	0	0	1	52
<b>% Articulated Trucks</b>	0%	0%	0%	0%	0%	7.8%	9.1%	7.6%	1.9%	0%	0%	1.8%	6.0%
<b>Buses</b>	0	0	0	0	0	2	0	2	0	0	0	0	2
<b>% Buses</b>	0%	0%	0%	0%	0%	0.3%	0%	0.3%	0%	0%	0%	0%	0.2%

\*L: Left, R: Right, T: Thru, U: U-Turn



**EMPO/INDOT US 41 @ Hillsdale Rd - TMC**

Tue Jul 16, 2019

AM Peak (Jul 16 2019 7:15AM - 8:15 AM)

All Classes (Motorcycles, Cars, Light Goods Vehicles, Single-Unit Trucks, Articulated Trucks, Buses)

All Movements

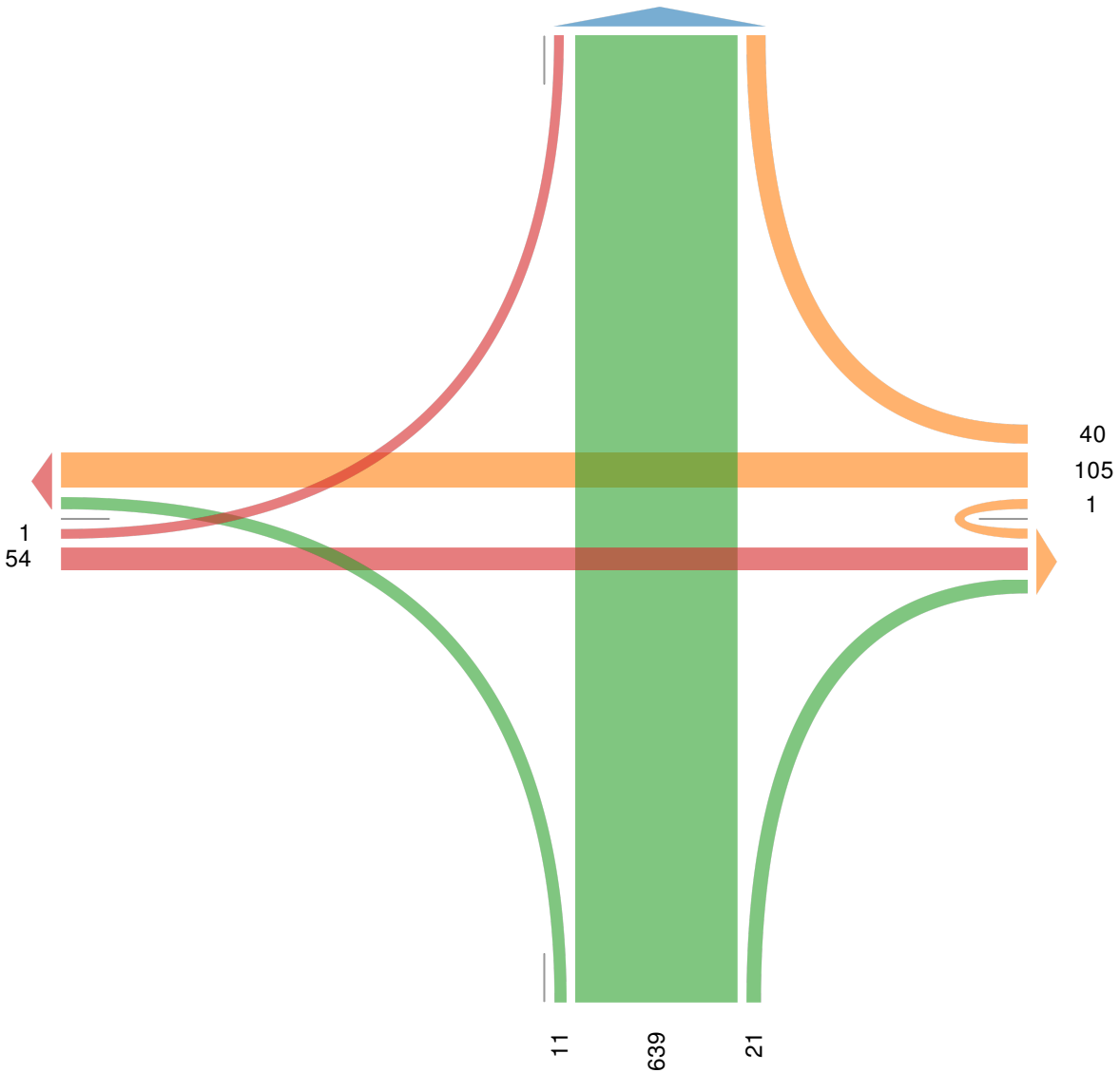
ID: 679143, Location: 38.079492, -87.554662



Provided by: Indiana DOT  
100 N. Senate Ave.,  
Indianapolis, IN, 46204, US

[N] US 41  
Total: 680  
In: 0 Out: 680

[W] Hillsdale Rd  
Total: 171  
In: 55 Out: 116



Out: 0 In: 671  
Total: 671  
[S] US 41

Out: 76 In: 146  
Total: 222  
[E] Hillsdale Rd

**EMPO/INDOT US 41 @ Hillsdale Rd - TMC**

Tue Jul 16, 2019

Midday Peak (Jul 16 2019 11:30AM - 12:30 PM)

All Classes (Motorcycles, Cars, Light Goods Vehicles, Single-Unit Trucks, Articulated Trucks, Buses)

All Movements

ID: 679143, Location: 38.079492, -87.554662



Provided by: Indiana DOT  
100 N. Senate Ave.,  
Indianapolis, IN, 46204, US

Leg Direction	Hillsdale Rd Westbound				US 41 Northbound				Hillsdale Rd Eastbound				
Time	R	T	U	App	R	T	L	App	T	L	U	App	Int
2019-07-16 11:30AM	18	17	0	35	7	170	1	178	8	0	0	8	221
11:45AM	16	12	0	28	8	187	1	196	10	1	0	11	235
12:00PM	4	24	0	28	9	188	3	200	15	1	0	16	244
12:15PM	7	10	0	17	6	170	3	179	13	0	0	13	209
<b>Total</b>	45	63	0	108	30	715	8	753	46	2	0	48	909
<b>% Approach</b>	41.7%	58.3%	0%	-	4.0%	95.0%	1.1%	-	95.8%	4.2%	0%	-	-
<b>% Total</b>	5.0%	6.9%	0%	11.9%	3.3%	78.7%	0.9%	82.8%	5.1%	0.2%	0%	5.3%	-
<b>PHF</b>	0.625	0.656	-	0.771	0.833	0.951	0.667	0.941	0.767	0.500	-	0.750	0.931
<b>Motorcycles</b>	0	0	0	0	0	0	0	0	0	0	0	0	0
<b>% Motorcycles</b>	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
<b>Cars</b>	32	44	0	76	21	463	4	488	36	2	0	38	602
<b>% Cars</b>	71.1%	69.8%	0%	70.4%	70.0%	64.8%	50.0%	64.8%	78.3%	100%	0%	79.2%	66.2%
<b>Light Goods Vehicles</b>	13	14	0	27	7	152	3	162	9	0	0	9	198
<b>% Light Goods Vehicles</b>	28.9%	22.2%	0%	25.0%	23.3%	21.3%	37.5%	21.5%	19.6%	0%	0%	18.8%	21.8%
<b>Single-Unit Trucks</b>	0	4	0	4	2	25	1	28	1	0	0	1	33
<b>% Single-Unit Trucks</b>	0%	6.3%	0%	3.7%	6.7%	3.5%	12.5%	3.7%	2.2%	0%	0%	2.1%	3.6%
<b>Articulated Trucks</b>	0	1	0	1	0	72	0	72	0	0	0	0	73
<b>% Articulated Trucks</b>	0%	1.6%	0%	0.9%	0%	10.1%	0%	9.6%	0%	0%	0%	0%	8.0%
<b>Buses</b>	0	0	0	0	0	3	0	3	0	0	0	0	3
<b>% Buses</b>	0%	0%	0%	0%	0%	0.4%	0%	0.4%	0%	0%	0%	0%	0.3%

\*L: Left, R: Right, T: Thru, U: U-Turn

**EMPO/INDOT US 41 @ Hillsdale Rd - TMC**

Tue Jul 16, 2019

Midday Peak (Jul 16 2019 11:30AM - 12:30 PM)

All Classes (Motorcycles, Cars, Light Goods Vehicles, Single-Unit Trucks, Articulated Trucks, Buses)

All Movements

ID: 679143, Location: 38.079492, -87.554662



Provided by: Indiana DOT  
100 N. Senate Ave.,  
Indianapolis, IN, 46204, US

[N] US 41

Total: 762

In: 0

Out: 762

[W] Hillsdale Rd

Total: 119

Out: 71

In: 48

46

2

45

63

Out: 76

In: 108

Total: 184

[E] Hillsdale Rd

8

715

30

Out: 0

In: 753

Total: 753

[S] US 41

**EMPO/INDOT US 41 @ Hillsdale Rd - TMC**

Tue Jul 16, 2019

PM Peak (Jul 16 2019 3:45PM - 4:45 PM) - Overall Peak Hour

All Classes (Motorcycles, Cars, Light Goods Vehicles, Single-Unit Trucks, Articulated Trucks, Buses)

All Movements

ID: 679143, Location: 38.079492, -87.554662



Provided by: Indiana DOT  
100 N. Senate Ave.,  
Indianapolis, IN, 46204, US

Leg Direction	Hillsdale Rd Westbound				US 41 Northbound				Hillsdale Rd Eastbound				Int
	R	T	U	App	R	T	L	App	T	L	U	App	
2019-07-16 3:45PM	7	15	0	22	11	238	6	255	23	1	0	24	301
4:00PM	11	13	0	24	13	240	4	257	18	0	0	18	299
4:15PM	15	16	0	31	10	257	9	276	20	0	0	20	327
4:30PM	9	14	0	23	9	231	6	246	21	0	0	21	290
<b>Total</b>	42	58	0	100	43	966	25	1034	82	1	0	83	1217
<b>% Approach</b>	42.0%	58.0%	0%	-	4.2%	93.4%	2.4%	-	98.8%	1.2%	0%	-	-
<b>% Total</b>	3.5%	4.8%	0%	8.2%	3.5%	79.4%	2.1%	85.0%	6.7%	0.1%	0%	6.8%	-
<b>PHF</b>	0.700	0.906	-	0.806	0.827	0.940	0.694	0.937	0.891	0.250	-	0.865	0.930
<b>Motorcycles</b>	0	0	0	0	0	0	0	0	0	0	0	0	0
<b>% Motorcycles</b>	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
<b>Cars</b>	32	43	0	75	34	674	18	726	61	1	0	62	863
<b>% Cars</b>	76.2%	74.1%	0%	75.0%	79.1%	69.8%	72.0%	70.2%	74.4%	100%	0%	74.7%	70.9%
<b>Light Goods Vehicles</b>	9	15	0	24	9	175	6	190	21	0	0	21	235
<b>% Light Goods Vehicles</b>	21.4%	25.9%	0%	24.0%	20.9%	18.1%	24.0%	18.4%	25.6%	0%	0%	25.3%	19.3%
<b>Single-Unit Trucks</b>	1	0	0	1	0	44	1	45	0	0	0	0	46
<b>% Single-Unit Trucks</b>	2.4%	0%	0%	1.0%	0%	4.6%	4.0%	4.4%	0%	0%	0%	0%	3.8%
<b>Articulated Trucks</b>	0	0	0	0	0	71	0	71	0	0	0	0	71
<b>% Articulated Trucks</b>	0%	0%	0%	0%	0%	7.3%	0%	6.9%	0%	0%	0%	0%	5.8%
<b>Buses</b>	0	0	0	0	0	2	0	2	0	0	0	0	2
<b>% Buses</b>	0%	0%	0%	0%	0%	0.2%	0%	0.2%	0%	0%	0%	0%	0.2%

\*L: Left, R: Right, T: Thru, U: U-Turn

**EMPO/INDOT US 41 @ Hillsdale Rd - TMC**

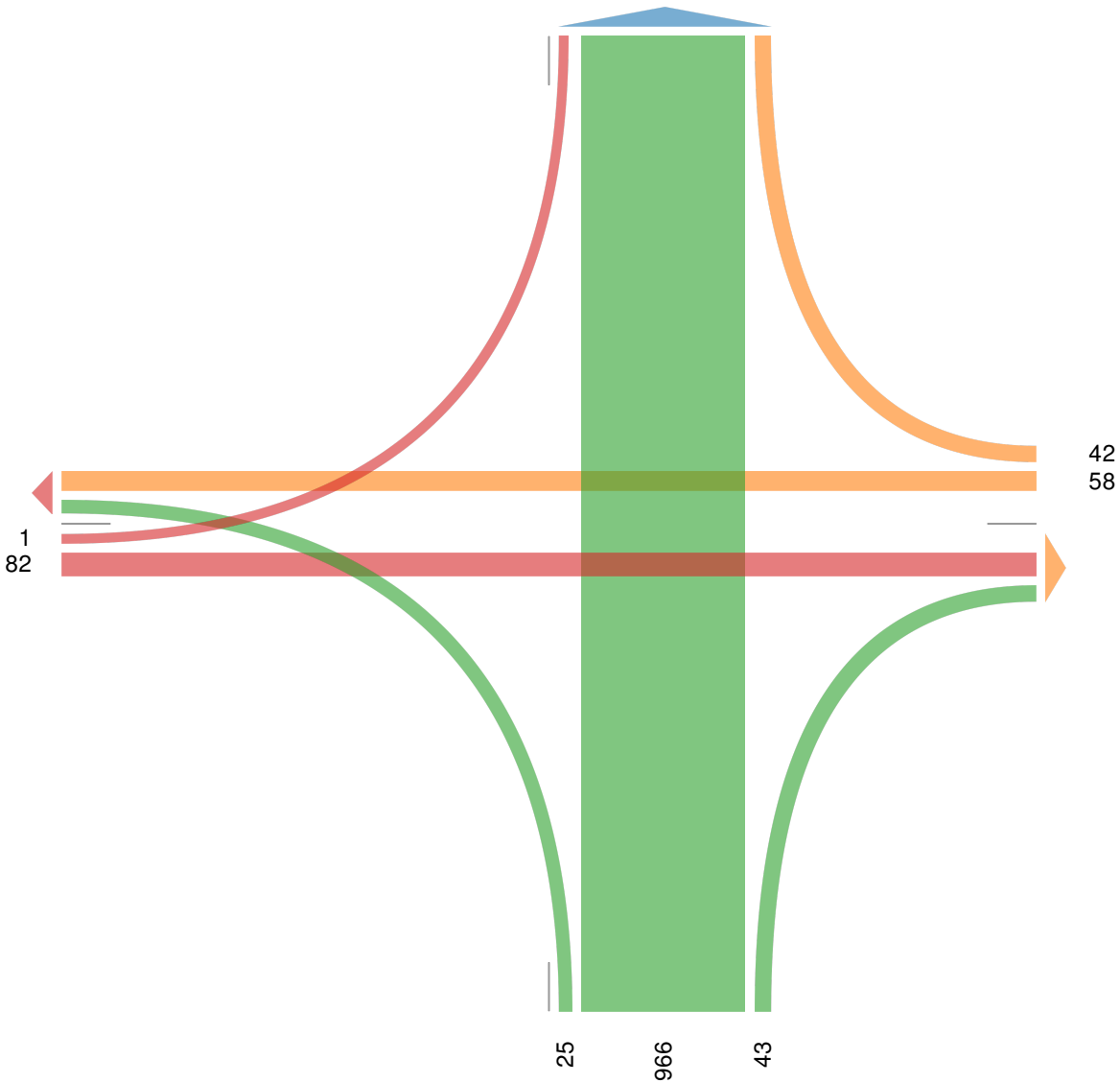
Tue Jul 16, 2019  
 PM Peak (Jul 16 2019 3:45PM - 4:45 PM) - Overall Peak Hour  
 All Classes (Motorcycles, Cars, Light Goods Vehicles, Single-Unit Trucks,  
 Articulated Trucks, Buses)  
 All Movements  
 ID: 679143, Location: 38.079492, -87.554662



Provided by: Indiana DOT  
 100 N. Senate Ave.,  
 Indianapolis, IN, 46204, US

[N] US 41  
 Total: 1009  
 In: 0 Out: 1009

[W] Hillsdale Rd  
 Total: 166  
 In: 83 Out: 83



Out: 0 In: 1034  
 Total: 1034  
 [S] US 41

Out: 125 In: 100  
 Total: 225  
 [E] Hillsdale Rd

**EMPO/INDOT US 41 @ Hillsdale Rd - TMC**

Tue Jul 16, 2019

Full Length (6 AM-6 AM (+1))

All Classes (Motorcycles, Cars, Light Goods Vehicles, Single-Unit Trucks, Articulated Trucks, Buses)

All Movements

ID: 680180, Location: 38.079492, -87.554662



Provided by: Indiana DOT  
100 N. Senate Ave.,  
Indianapolis, IN, 46204, US

Leg Direction	US 41 Southbound				Hillsdale Rd Westbound				Hillsdale Rd Eastbound				Int
	R	T	L	App	T	L	U	App	R	T	U	App	
2019-07-16 6:00AM	0	163	2	165	0	8	0	8	3	0	0	3	176
6:15AM	0	151	4	155	2	12	0	14	1	2	0	3	172
6:30AM	0	182	9	191	4	19	0	23	2	4	0	6	220
6:45AM	0	171	4	175	4	24	0	28	3	4	0	7	210
Hourly Total	0	667	19	686	10	63	0	73	9	10	0	19	778
7:00AM	0	214	7	221	2	18	0	20	3	2	0	5	246
7:15AM	0	224	7	231	6	30	0	36	4	4	0	8	275
7:30AM	0	230	7	237	5	21	0	26	4	7	0	11	274
7:45AM	0	211	12	223	5	26	0	31	3	9	0	12	266
Hourly Total	0	879	33	912	18	95	0	113	14	22	0	36	1061
8:00AM	0	174	5	179	5	18	0	23	5	4	0	9	211
8:15AM	0	160	5	165	0	15	0	15	3	2	0	5	185
8:30AM	0	168	7	175	3	15	0	18	2	6	0	8	201
8:45AM	0	171	18	189	2	16	0	18	2	7	0	9	216
Hourly Total	0	673	35	708	10	64	0	74	12	19	0	31	813
9:00AM	1	164	5	170	3	14	0	17	1	2	0	3	190
9:15AM	2	165	4	171	4	12	0	16	1	3	0	4	191
9:30AM	2	158	3	163	6	13	0	19	2	5	0	7	189
9:45AM	0	155	1	156	6	13	0	19	1	2	0	3	178
Hourly Total	5	642	13	660	19	52	0	71	5	12	0	17	748
10:00AM	0	155	5	160	7	9	0	16	1	1	0	2	178
10:15AM	0	162	5	167	6	10	0	16	0	5	0	5	188
10:30AM	0	167	7	174	7	12	0	19	1	6	0	7	200
10:45AM	0	174	14	188	4	14	0	18	4	5	0	9	215
Hourly Total	0	658	31	689	24	45	0	69	6	17	0	23	781
11:00AM	0	179	7	186	5	9	0	14	3	3	0	6	206
11:15AM	0	196	12	208	4	8	0	12	2	2	0	4	224
11:30AM	0	218	7	225	7	11	0	18	2	2	0	4	247
11:45AM	1	210	8	219	7	9	0	16	2	2	0	4	239
Hourly Total	1	803	34	838	23	37	0	60	9	9	0	18	916
12:00PM	0	180	13	193	9	17	1	27	3	4	0	7	227
12:15PM	0	204	9	213	3	9	0	12	0	5	0	5	230
12:30PM	0	199	9	208	10	15	0	25	2	4	0	6	239
12:45PM	0	196	15	211	0	9	0	9	0	7	0	7	227
Hourly Total	0	779	46	825	22	50	1	73	5	20	0	25	923
1:00PM	1	174	15	190	5	12	0	17	0	6	0	6	213
1:15PM	0	177	6	183	7	16	0	23	3	2	0	5	211
1:30PM	1	175	9	185	2	13	0	15	2	2	0	4	204
1:45PM	1	190	8	199	9	18	0	27	1	3	0	4	230
Hourly Total	3	716	38	757	23	59	0	82	6	13	0	19	858
2:00PM	0	198	2	200	4	8	0	12	1	3	0	4	216
2:15PM	2	186	7	195	2	5	0	7	1	2	0	3	205
2:30PM	0	210	8	218	8	7	0	15	1	1	0	2	235
2:45PM	0	186	5	191	9	6	0	15	0	1	0	1	207
Hourly Total	2	780	22	804	23	26	0	49	3	7	0	10	863
3:00PM	0	213	10	223	9	15	0	24	2	6	0	8	255
3:15PM	0	215	8	223	4	3	0	7	3	2	0	5	235
3:30PM	0	265	9	274	7	12	0	19	2	6	0	8	301
3:45PM	1	276	13	290	9	13	0	22	1	9	0	10	322
Hourly Total	1	969	40	1010	29	43	0	72	8	23	0	31	1113
4:00PM	0	253	14	267	11	6	0	17	1	5	0	6	290
4:15PM	0	221	15	236	14	10	0	24	2	4	0	6	266
4:30PM	0	194	17	211	13	9	0	22	1	4	0	5	238

Leg Direction	US 41 Southbound				Hillsdale Rd Westbound				Hillsdale Rd Eastbound				Int
	R	T	L	App	T	L	U	App	R	T	U	App	
4:45PM	0	195	13	208	2	10	0	12	2	8	0	10	230
Hourly Total	0	863	59	922	40	35	0	75	6	21	0	27	1024
5:00PM	0	200	24	224	8	8	0	16	0	9	0	9	249
5:15PM	0	181	13	194	8	9	0	17	2	5	0	7	218
5:30PM	0	181	11	192	5	10	0	15	4	3	0	7	214
5:45PM	0	162	11	173	6	11	0	17	2	5	0	7	197
Hourly Total	0	724	59	783	27	38	0	65	8	22	0	30	878
6:00PM	0	149	16	165	0	12	0	12	1	2	0	3	180
6:15PM	1	141	7	149	6	5	0	11	1	4	0	5	165
6:30PM	0	128	11	139	7	13	0	20	2	4	0	6	165
6:45PM	0	109	13	122	7	4	0	11	0	2	0	2	135
Hourly Total	1	527	47	575	20	34	0	54	4	12	0	16	645
7:00PM	0	199	12	211	6	9	0	15	2	3	0	5	231
7:15PM	0	121	11	132	9	12	0	21	1	8	0	9	162
7:30PM	0	109	9	118	6	8	0	14	1	2	0	3	135
7:45PM	0	76	6	82	7	4	0	11	0	5	0	5	98
Hourly Total	0	505	38	543	28	33	0	61	4	18	0	22	626
8:00PM	0	100	6	106	6	5	0	11	0	4	0	4	121
8:15PM	3	107	16	126	4	7	0	11	1	2	0	3	140
8:30PM	0	119	11	130	4	4	0	8	0	2	0	2	140
8:45PM	0	108	7	115	2	8	0	10	0	0	0	0	125
Hourly Total	3	434	40	477	16	24	0	40	1	8	0	9	526
9:00PM	0	74	9	83	3	5	0	8	1	1	0	2	93
9:15PM	0	75	4	79	2	2	0	4	2	2	0	4	87
9:30PM	0	68	4	72	5	2	0	7	0	0	0	0	79
9:45PM	0	51	0	51	2	1	0	3	1	0	0	1	55
Hourly Total	0	268	17	285	12	10	0	22	4	3	0	7	314
10:00PM	0	49	3	52	2	1	0	3	0	0	0	0	55
10:15PM	0	61	3	64	3	1	0	4	0	0	0	0	68
10:30PM	0	55	2	57	1	3	0	4	0	0	0	0	61
10:45PM	0	33	0	33	1	6	0	7	0	0	0	0	40
Hourly Total	0	198	8	206	7	11	0	18	0	0	0	0	224
11:00PM	1	36	1	38	1	2	0	3	1	0	0	1	42
11:15PM	0	25	0	25	1	1	0	2	0	0	0	0	27
11:30PM	0	23	0	23	0	2	0	2	0	3	0	3	28
11:45PM	0	27	2	29	3	0	0	3	2	0	0	2	34
Hourly Total	1	111	3	115	5	5	0	10	3	3	0	6	131
2019-07-17 12:00AM	0	17	0	17	0	0	0	0	0	0	0	0	17
12:15AM	0	19	2	21	1	1	0	2	0	0	0	0	23
12:30AM	0	21	1	22	0	0	0	0	0	0	0	0	22
12:45AM	0	38	2	40	1	2	0	3	0	1	0	1	44
Hourly Total	0	95	5	100	2	3	0	5	0	1	0	1	106
1:00AM	0	27	1	28	0	2	0	2	0	0	0	0	30
1:15AM	0	20	2	22	0	1	0	1	0	0	0	0	23
1:30AM	0	38	1	39	0	0	0	0	0	0	0	0	39
1:45AM	0	18	0	18	0	0	0	0	0	0	0	0	18
Hourly Total	0	103	4	107	0	3	0	3	0	0	0	0	110
2:00AM	0	21	0	21	0	1	0	1	0	0	0	0	22
2:15AM	0	15	0	15	0	1	0	1	0	0	0	0	16
2:30AM	0	21	0	21	0	0	0	0	0	0	0	0	21
2:45AM	0	22	1	23	0	1	0	1	0	0	0	0	24
Hourly Total	0	79	1	80	0	3	0	3	0	0	0	0	83
3:00AM	0	21	1	22	0	0	0	0	0	1	0	1	23
3:15AM	0	20	0	20	0	4	0	4	0	0	0	0	24
3:30AM	0	17	0	17	0	0	0	0	0	0	0	0	17
3:45AM	0	17	1	18	0	3	0	3	0	1	0	1	22
Hourly Total	0	75	2	77	0	7	0	7	0	2	0	2	86
4:00AM	0	31	0	31	0	1	0	1	1	0	0	1	33
4:15AM	0	41	0	41	0	1	0	1	0	0	0	0	42
4:30AM	0	49	3	52	0	2	0	2	1	0	0	1	55

Leg Direction	US 41 Southbound				Hillsdale Rd Westbound				Hillsdale Rd Eastbound				Int
	R	T	L	App	T	L	U	App	R	T	U	App	
Time													
4:45AM	0	44	0	<b>44</b>	0	4	0	<b>4</b>	0	0	0	<b>0</b>	<b>48</b>
Hourly Total	0	165	3	<b>168</b>	0	8	0	<b>8</b>	2	0	0	<b>2</b>	<b>178</b>
5:00AM	0	52	2	<b>54</b>	0	9	0	<b>9</b>	1	2	0	<b>3</b>	<b>66</b>
5:15AM	0	70	2	<b>72</b>	2	5	0	<b>7</b>	0	1	0	<b>1</b>	<b>80</b>
5:30AM	0	90	8	<b>98</b>	3	12	0	<b>15</b>	1	2	0	<b>3</b>	<b>116</b>
5:45AM	0	108	1	<b>109</b>	1	8	0	<b>9</b>	0	6	0	<b>6</b>	<b>124</b>
Hourly Total	0	320	13	<b>333</b>	6	34	0	<b>40</b>	2	11	0	<b>13</b>	<b>386</b>
<b>Total</b>	17	12033	610	<b>12660</b>	364	782	1	<b>1147</b>	111	253	0	<b>364</b>	<b>14171</b>
<b>% Approach</b>	0.1%	95.0%	4.8%	-	31.7%	68.2%	0.1%	-	30.5%	69.5%	0%	-	-
<b>% Total</b>	0.1%	84.9%	4.3%	<b>89.3%</b>	2.6%	5.5%	0%	<b>8.1%</b>	0.8%	1.8%	0%	<b>2.6%</b>	-
<b>Motorcycles</b>	0	17	1	<b>18</b>	0	0	0	<b>0</b>	0	0	0	<b>0</b>	18
<b>% Motorcycles</b>	0%	0.1%	0.2%	<b>0.1%</b>	0%	0%	0%	<b>0%</b>	0%	0%	0%	<b>0%</b>	0.1%
<b>Cars</b>	17	8210	438	<b>8665</b>	262	594	1	<b>857</b>	72	184	0	<b>256</b>	9778
<b>% Cars</b>	100%	68.2%	71.8%	<b>68.4%</b>	72.0%	76.0%	100%	<b>74.7%</b>	64.9%	72.7%	0%	<b>70.3%</b>	69.0%
<b>Light Goods Vehicles</b>	0	2368	153	<b>2521</b>	99	167	0	<b>266</b>	39	66	0	<b>105</b>	2892
<b>% Light Goods Vehicles</b>	0%	19.7%	25.1%	<b>19.9%</b>	27.2%	21.4%	0%	<b>23.2%</b>	35.1%	26.1%	0%	<b>28.8%</b>	20.4%
<b>Single-Unit Trucks</b>	0	453	14	<b>467</b>	2	18	0	<b>20</b>	0	3	0	<b>3</b>	490
<b>% Single-Unit Trucks</b>	0%	3.8%	2.3%	<b>3.7%</b>	0.5%	2.3%	0%	<b>1.7%</b>	0%	1.2%	0%	<b>0.8%</b>	3.5%
<b>Articulated Trucks</b>	0	967	4	<b>971</b>	1	2	0	<b>3</b>	0	0	0	<b>0</b>	974
<b>% Articulated Trucks</b>	0%	8.0%	0.7%	<b>7.7%</b>	0.3%	0.3%	0%	<b>0.3%</b>	0%	0%	0%	<b>0%</b>	6.9%
<b>Buses</b>	0	18	0	<b>18</b>	0	1	0	<b>1</b>	0	0	0	<b>0</b>	19
<b>% Buses</b>	0%	0.1%	0%	<b>0.1%</b>	0%	0.1%	0%	<b>0.1%</b>	0%	0%	0%	<b>0%</b>	0.1%

\*L: Left, R: Right, T: Thru, U: U-Turn



**EMPO/INDOT US 41 @ Hillsdale Rd - TMC**

Tue Jul 16, 2019

Full Length (6 AM-6 AM (+1))

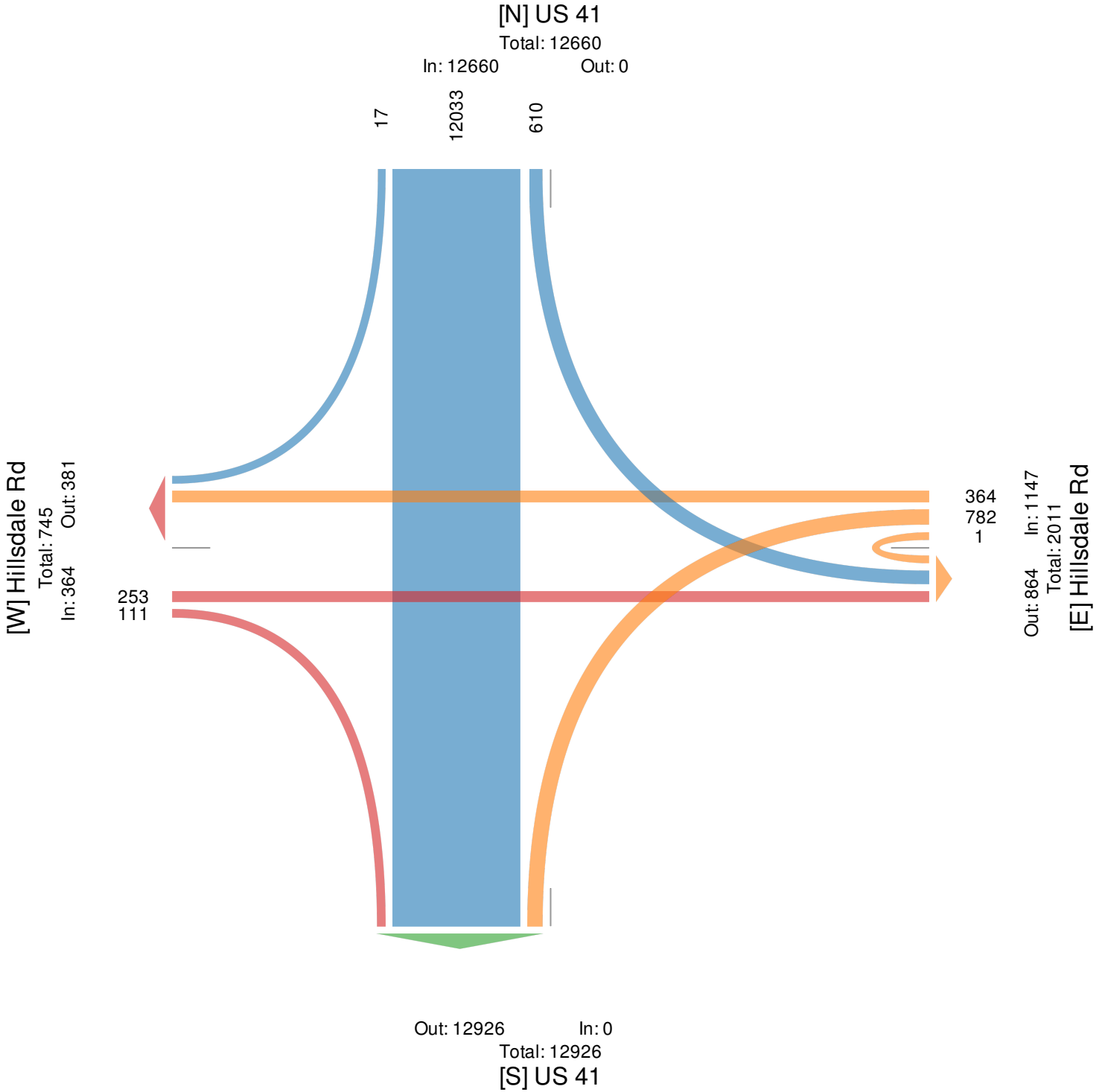
All Classes (Motorcycles, Cars, Light Goods Vehicles, Single-Unit Trucks, Articulated Trucks, Buses)

All Movements

ID: 680180, Location: 38.079492, -87.554662



Provided by: Indiana DOT  
100 N. Senate Ave.,  
Indianapolis, IN, 46204, US



**EMPO/INDOT US 41 @ Hillsdale Rd - TMC**

Tue Jul 16, 2019

AM Peak (Jul 16 2019 7AM - 8 AM)

All Classes (Motorcycles, Cars, Light Goods Vehicles, Single-Unit Trucks, Articulated Trucks, Buses)

All Movements

ID: 680180, Location: 38.079492, -87.554662



Provided by: Indiana DOT  
100 N. Senate Ave.,  
Indianapolis, IN, 46204, US

Leg Direction	US 41 Southbound				Hillsdale Rd Westbound				Hillsdale Rd Eastbound				Int
	R	T	L	App	T	L	U	App	R	T	U	App	
2019-07-16 7:00AM	0	214	7	221	2	18	0	20	3	2	0	5	246
7:15AM	0	224	7	231	6	30	0	36	4	4	0	8	275
7:30AM	0	230	7	237	5	21	0	26	4	7	0	11	274
7:45AM	0	211	12	223	5	26	0	31	3	9	0	12	266
<b>Total</b>	0	879	33	912	18	95	0	113	14	22	0	36	1061
<b>% Approach</b>	0%	96.4%	3.6%	-	15.9%	84.1%	0%	-	38.9%	61.1%	0%	-	-
<b>% Total</b>	0%	82.8%	3.1%	86.0%	1.7%	9.0%	0%	10.7%	1.3%	2.1%	0%	3.4%	-
<b>PHF</b>	-	0.955	0.688	0.962	0.750	0.792	-	0.785	0.875	0.611	-	0.750	0.965
<b>Motorcycles</b>	0	2	0	2	0	0	0	0	0	0	0	0	2
<b>% Motorcycles</b>	0%	0.2%	0%	0.2%	0%	0%	0%	0%	0%	0%	0%	0%	0.2%
<b>Cars</b>	0	625	21	646	11	75	0	86	11	16	0	27	759
<b>% Cars</b>	0%	71.1%	63.6%	70.8%	61.1%	78.9%	0%	76.1%	78.6%	72.7%	0%	75.0%	71.5%
<b>Light Goods Vehicles</b>	0	180	11	191	7	16	0	23	3	6	0	9	223
<b>% Light Goods Vehicles</b>	0%	20.5%	33.3%	20.9%	38.9%	16.8%	0%	20.4%	21.4%	27.3%	0%	25.0%	21.0%
<b>Single-Unit Trucks</b>	0	19	1	20	0	4	0	4	0	0	0	0	24
<b>% Single-Unit Trucks</b>	0%	2.2%	3.0%	2.2%	0%	4.2%	0%	3.5%	0%	0%	0%	0%	2.3%
<b>Articulated Trucks</b>	0	52	0	52	0	0	0	0	0	0	0	0	52
<b>% Articulated Trucks</b>	0%	5.9%	0%	5.7%	0%	0%	0%	0%	0%	0%	0%	0%	4.9%
<b>Buses</b>	0	1	0	1	0	0	0	0	0	0	0	0	1
<b>% Buses</b>	0%	0.1%	0%	0.1%	0%	0%	0%	0%	0%	0%	0%	0%	0.1%

\*L: Left, R: Right, T: Thru, U: U-Turn

**EMPO/INDOT US 41 @ Hillsdale Rd - TMC**

Tue Jul 16, 2019

AM Peak (Jul 16 2019 7AM - 8 AM)

All Classes (Motorcycles, Cars, Light Goods Vehicles, Single-Unit Trucks, Articulated Trucks, Buses)

All Movements

ID: 680180, Location: 38.079492, -87.554662



Provided by: Indiana DOT  
100 N. Senate Ave.,  
Indianapolis, IN, 46204, US

[N] US 41

Total: 912

In: 912

Out: 0

879

33

[W] Hillsdale Rd

Total: 54

In: 36

Out: 18

22

14

18

95

Out: 55

In: 113

Total: 168

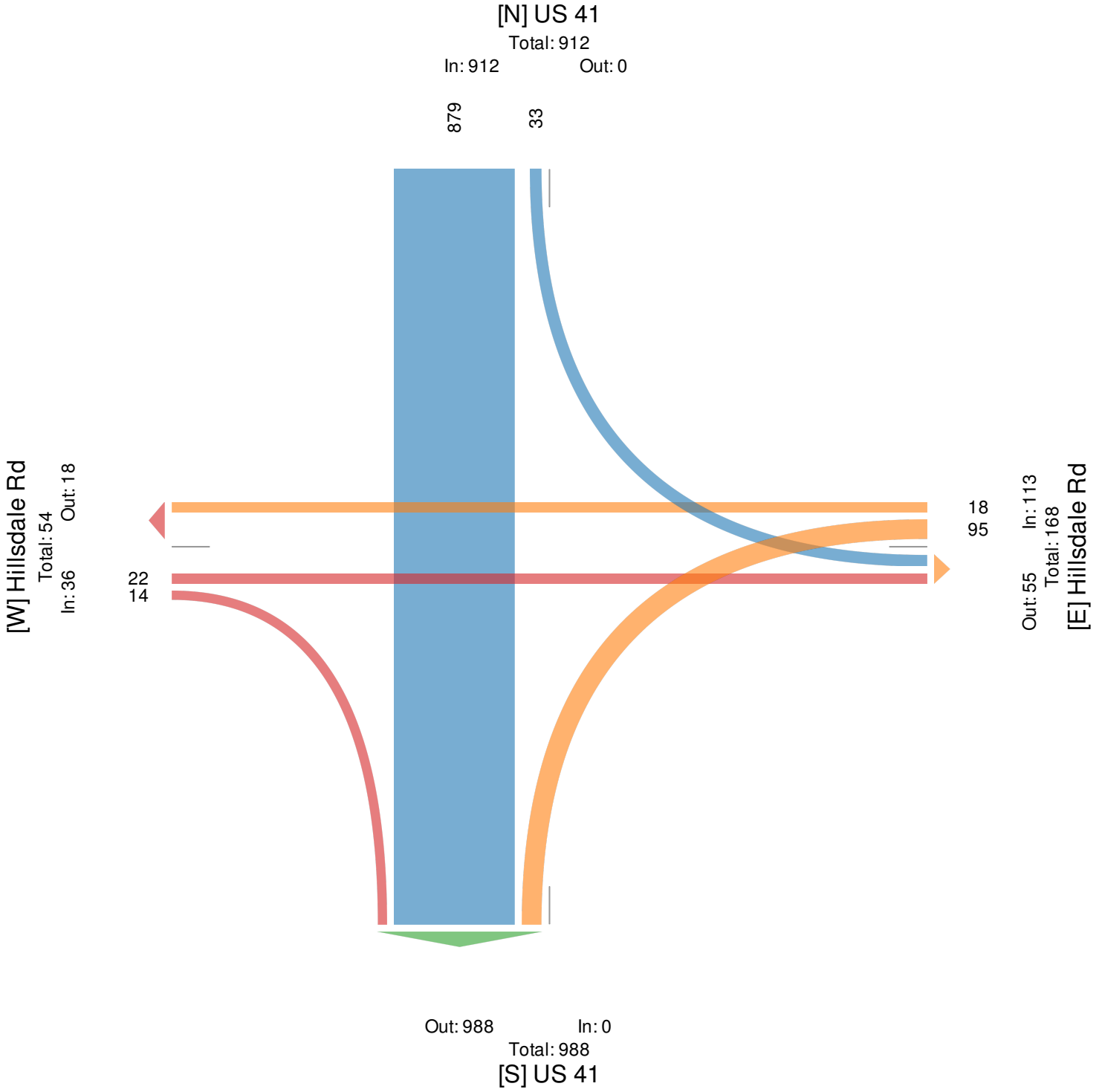
[E] Hillsdale Rd

Out: 988

In: 0

Total: 988

[S] US 41



**EMPO/INDOT US 41 @ Hillsdale Rd - TMC**

Tue Jul 16, 2019

Midday Peak (Jul 16 2019 11:30AM - 12:30 PM)

All Classes (Motorcycles, Cars, Light Goods Vehicles, Single-Unit Trucks, Articulated Trucks, Buses)

All Movements

ID: 680180, Location: 38.079492, -87.554662



Provided by: Indiana DOT  
100 N. Senate Ave.,  
Indianapolis, IN, 46204, US

Leg Direction	US 41 Southbound				Hillsdale Rd Westbound				Hillsdale Rd Eastbound				
Time	R	T	L	App	T	L	U	App	R	T	U	App	Int
2019-07-16 11:30AM	0	218	7	225	7	11	0	18	2	2	0	4	247
11:45AM	1	210	8	219	7	9	0	16	2	2	0	4	239
12:00PM	0	180	13	193	9	17	1	27	3	4	0	7	227
12:15PM	0	204	9	213	3	9	0	12	0	5	0	5	230
<b>Total</b>	1	812	37	850	26	46	1	73	7	13	0	20	943
<b>% Approach</b>	0.1%	95.5%	4.4%	-	35.6%	63.0%	1.4%	-	35.0%	65.0%	0%	-	-
<b>% Total</b>	0.1%	86.1%	3.9%	90.1%	2.8%	4.9%	0.1%	7.7%	0.7%	1.4%	0%	2.1%	-
<b>PHF</b>	0.250	0.931	0.712	0.944	0.722	0.676	0.250	0.676	0.583	0.650	-	0.714	0.954
<b>Motorcycles</b>	0	4	0	4	0	0	0	0	0	0	0	0	4
<b>% Motorcycles</b>	0%	0.5%	0%	0.5%	0%	0%	0%	0%	0%	0%	0%	0%	0.4%
<b>Cars</b>	1	563	30	594	20	33	1	54	2	8	0	10	658
<b>% Cars</b>	100%	69.3%	81.1%	69.9%	76.9%	71.7%	100%	74.0%	28.6%	61.5%	0%	50.0%	69.8%
<b>Light Goods Vehicles</b>	0	153	6	159	4	9	0	13	5	4	0	9	181
<b>% Light Goods Vehicles</b>	0%	18.8%	16.2%	18.7%	15.4%	19.6%	0%	17.8%	71.4%	30.8%	0%	45.0%	19.2%
<b>Single-Unit Trucks</b>	0	37	1	38	1	4	0	5	0	1	0	1	44
<b>% Single-Unit Trucks</b>	0%	4.6%	2.7%	4.5%	3.8%	8.7%	0%	6.8%	0%	7.7%	0%	5.0%	4.7%
<b>Articulated Trucks</b>	0	51	0	51	1	0	0	1	0	0	0	0	52
<b>% Articulated Trucks</b>	0%	6.3%	0%	6.0%	3.8%	0%	0%	1.4%	0%	0%	0%	0%	5.5%
<b>Buses</b>	0	4	0	4	0	0	0	0	0	0	0	0	4
<b>% Buses</b>	0%	0.5%	0%	0.5%	0%	0%	0%	0%	0%	0%	0%	0%	0.4%

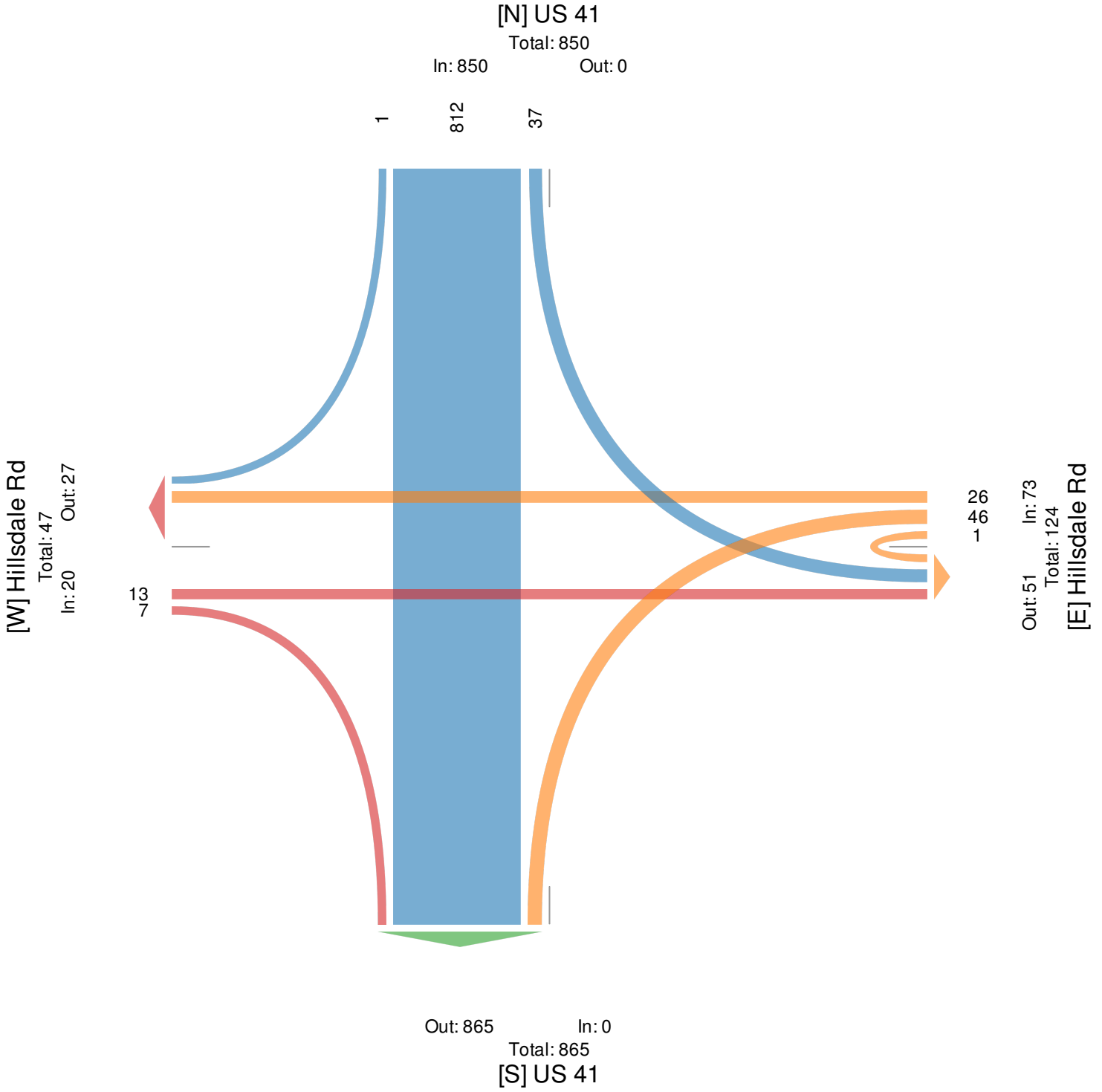
\*L: Left, R: Right, T: Thru, U: U-Turn

**EMPO/INDOT US 41 @ Hillsdale Rd - TMC**

Tue Jul 16, 2019  
 Midday Peak (Jul 16 2019 11:30AM - 12:30 PM)  
 All Classes (Motorcycles, Cars, Light Goods Vehicles, Single-Unit Trucks,  
 Articulated Trucks, Buses)  
 All Movements  
 ID: 680180, Location: 38.079492, -87.554662



Provided by: Indiana DOT  
 100 N. Senate Ave.,  
 Indianapolis, IN, 46204, US



**EMPO/INDOT US 41 @ Hillsdale Rd - TMC**

Tue Jul 16, 2019

PM Peak (Jul 16 2019 3:30PM - 4:30 PM) - Overall Peak Hour

All Classes (Motorcycles, Cars, Light Goods Vehicles, Single-Unit Trucks, Articulated Trucks, Buses)

All Movements

ID: 680180, Location: 38.079492, -87.554662



Provided by: Indiana DOT  
100 N. Senate Ave.,  
Indianapolis, IN, 46204, US

Leg Direction	US 41 Southbound				Hillsdale Rd Westbound				Hillsdale Rd Eastbound				
Time	R	T	L	App	T	L	U	App	R	T	U	App	Int
2019-07-16 3:30PM	0	265	9	274	7	12	0	19	2	6	0	8	301
3:45PM	1	276	13	290	9	13	0	22	1	9	0	10	322
4:00PM	0	253	14	267	11	6	0	17	1	5	0	6	290
4:15PM	0	221	15	236	14	10	0	24	2	4	0	6	266
<b>Total</b>	1	1015	51	1067	41	41	0	82	6	24	0	30	1179
<b>% Approach</b>	0.1%	95.1%	4.8%	-	50.0%	50.0%	0%	-	20.0%	80.0%	0%	-	-
<b>% Total</b>	0.1%	86.1%	4.3%	90.5%	3.5%	3.5%	0%	7.0%	0.5%	2.0%	0%	2.5%	-
<b>PHF</b>	0.250	0.919	0.850	0.920	0.732	0.788	-	0.854	0.750	0.667	-	0.750	0.915
<b>Motorcycles</b>	0	0	0	0	0	0	0	0	0	0	0	0	0
<b>% Motorcycles</b>	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%
<b>Cars</b>	1	727	37	765	28	29	0	57	4	17	0	21	843
<b>% Cars</b>	100%	71.6%	72.5%	71.7%	68.3%	70.7%	0%	69.5%	66.7%	70.8%	0%	70.0%	71.5%
<b>Light Goods Vehicles</b>	0	197	14	211	13	12	0	25	2	7	0	9	245
<b>% Light Goods Vehicles</b>	0%	19.4%	27.5%	19.8%	31.7%	29.3%	0%	30.5%	33.3%	29.2%	0%	30.0%	20.8%
<b>Single-Unit Trucks</b>	0	34	0	34	0	0	0	0	0	0	0	0	34
<b>% Single-Unit Trucks</b>	0%	3.3%	0%	3.2%	0%	0%	0%	0%	0%	0%	0%	0%	2.9%
<b>Articulated Trucks</b>	0	56	0	56	0	0	0	0	0	0	0	0	56
<b>% Articulated Trucks</b>	0%	5.5%	0%	5.2%	0%	0%	0%	0%	0%	0%	0%	0%	4.7%
<b>Buses</b>	0	1	0	1	0	0	0	0	0	0	0	0	1
<b>% Buses</b>	0%	0.1%	0%	0.1%	0%	0%	0%	0%	0%	0%	0%	0%	0.1%

\*L: Left, R: Right, T: Thru, U: U-Turn

**EMPO/INDOT US 41 @ Hillsdale Rd - TMC**

Tue Jul 16, 2019

PM Peak (Jul 16 2019 3:30PM - 4:30 PM) - Overall Peak Hour

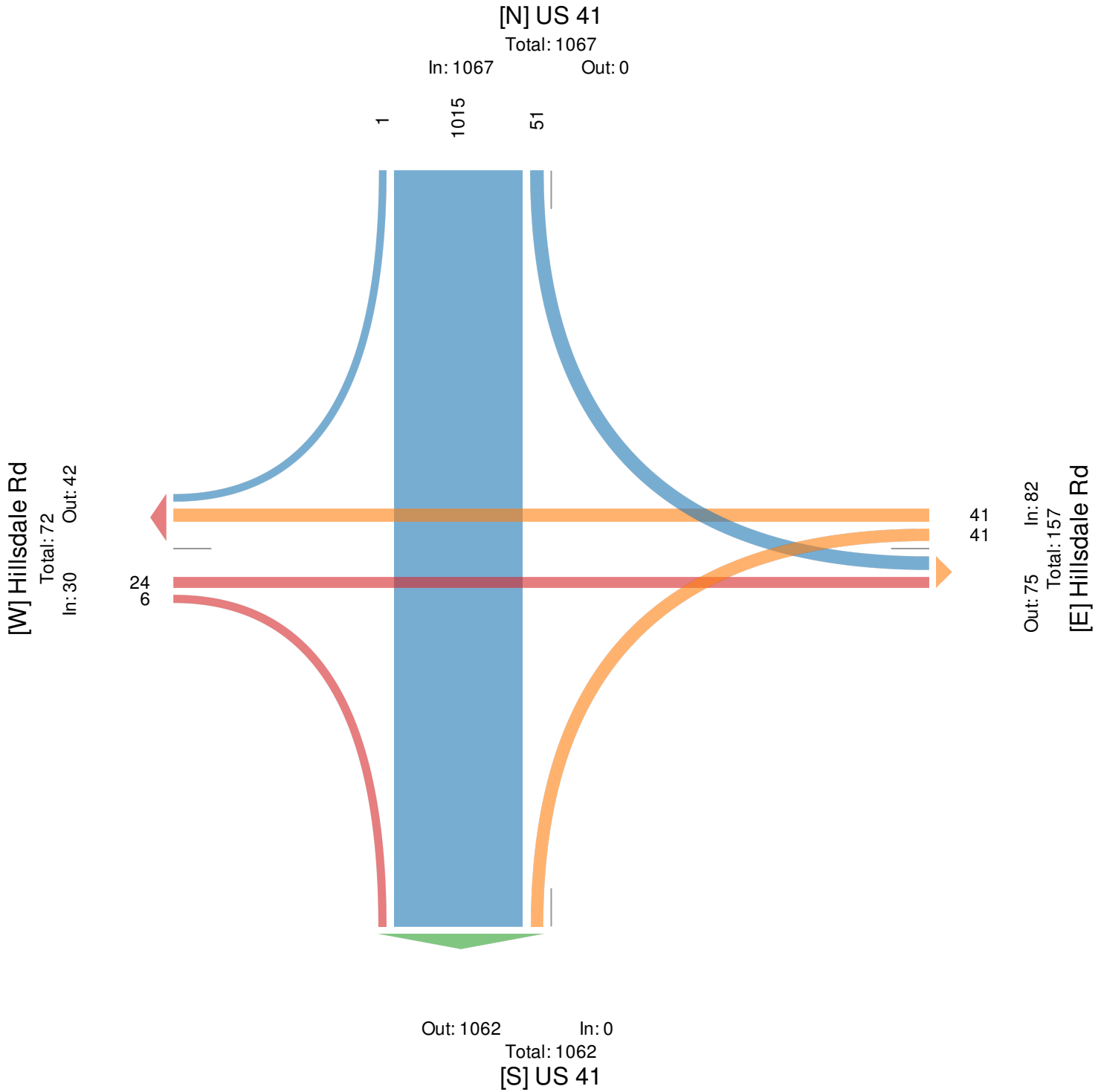
All Classes (Motorcycles, Cars, Light Goods Vehicles, Single-Unit Trucks, Articulated Trucks, Buses)

All Movements

ID: 680180, Location: 38.079492, -87.554662

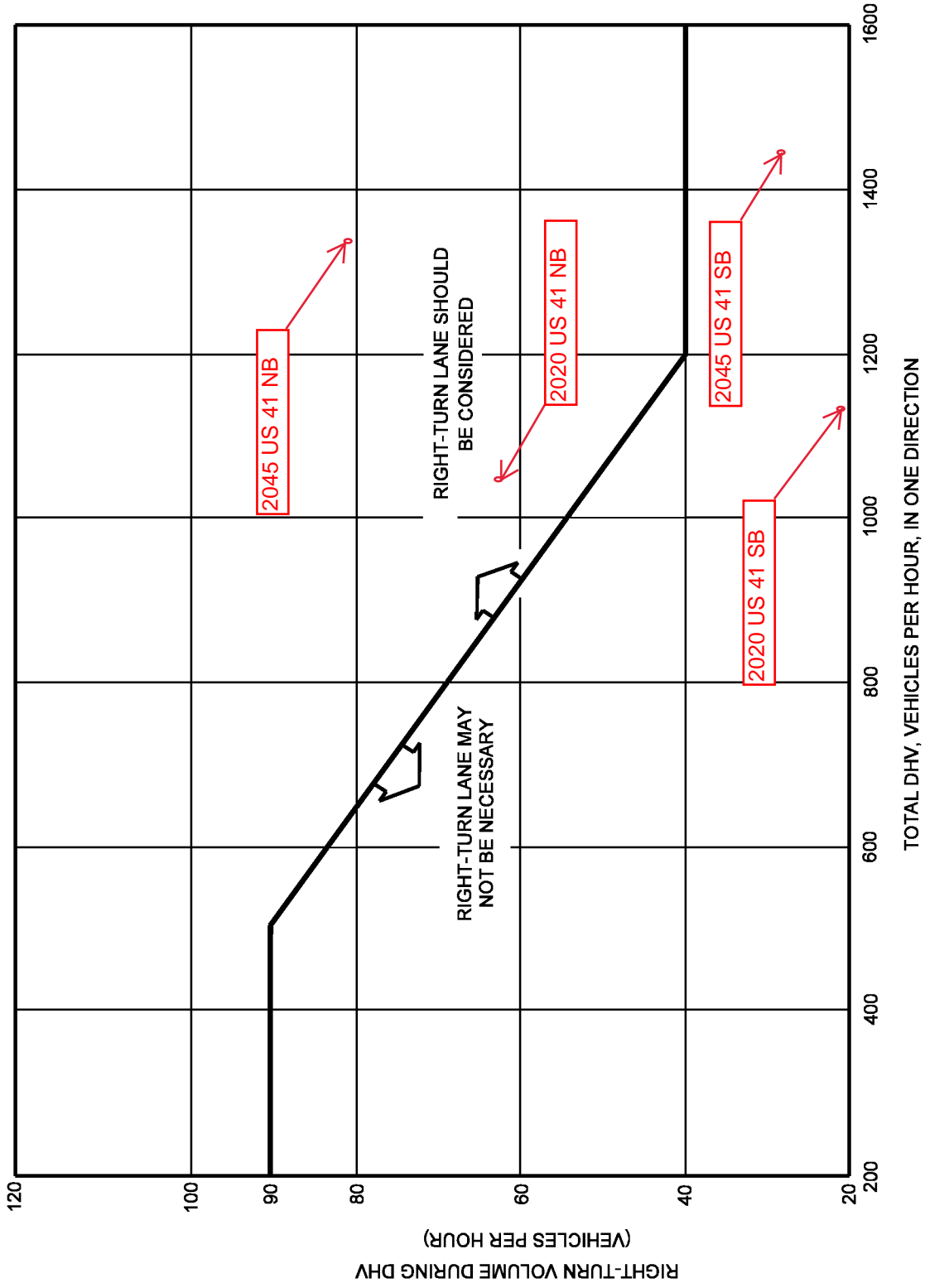


Provided by: Indiana DOT  
100 N. Senate Ave.,  
Indianapolis, IN, 46204, US



## **APPENDIX K**





**GUIDELINES FOR RIGHT-TURN LANES AT UNSIGNALIZED INTERSECTION ON 4-LANE HIGHWAYS**

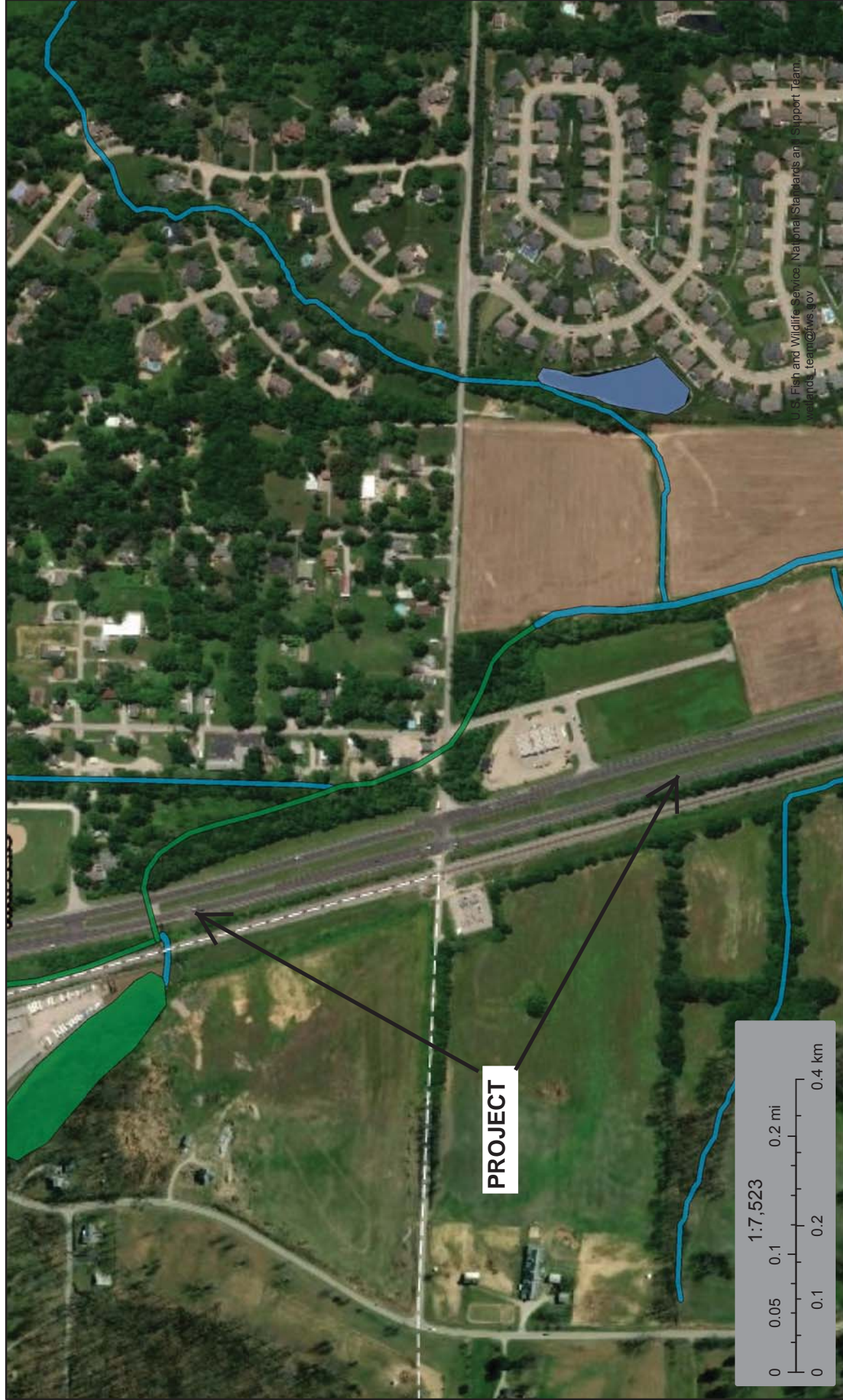
**Figure 46-4B**

## **APPENDIX L**



U.S. Fish and Wildlife Service

# National Wetlands Inventory



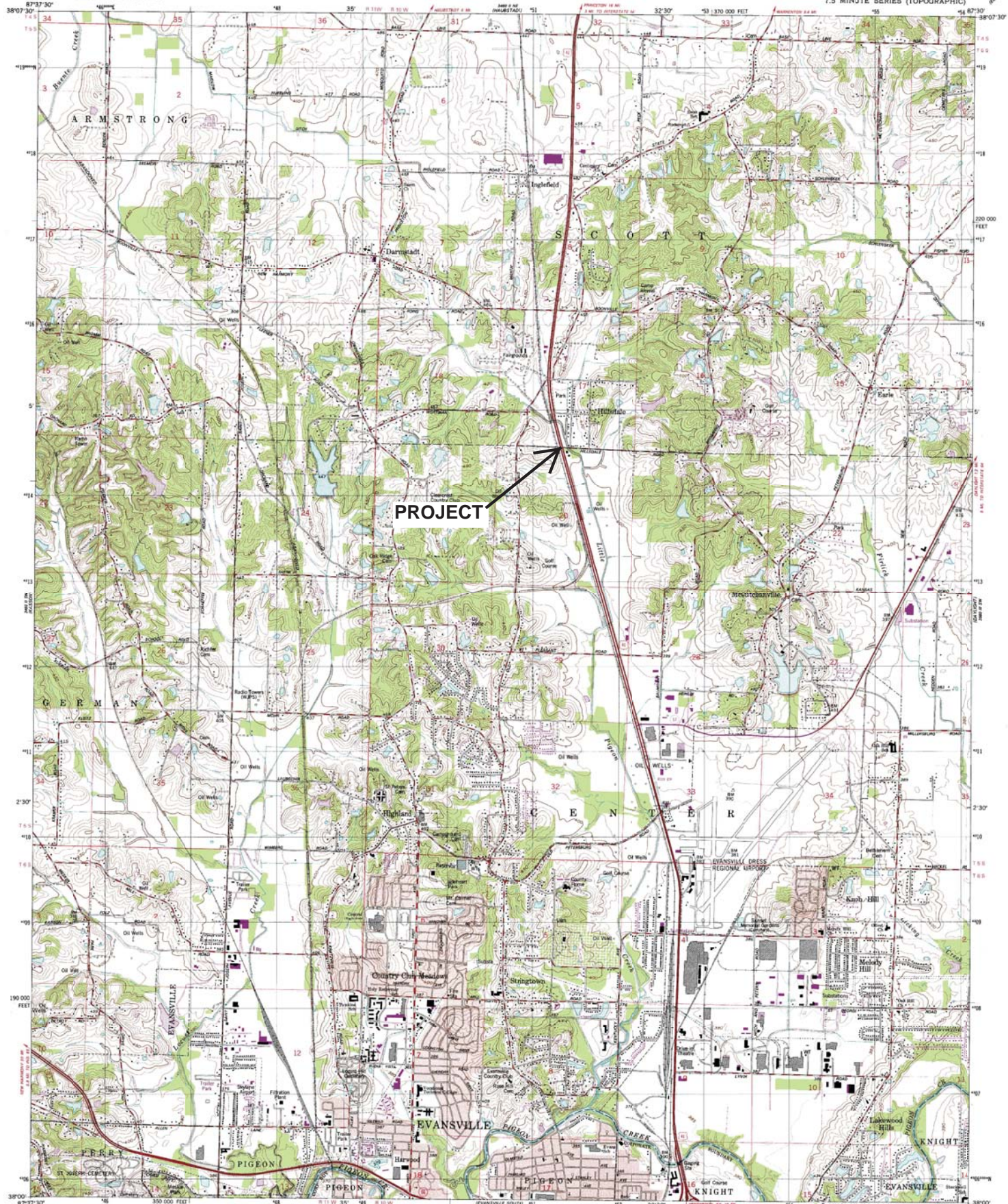
July 3, 2019

## Wetlands

- Estuarine and Marine Deepwater
- Estuarine and Marine Wetland
- Freshwater Emergent Wetland
- Freshwater Forested/Shrub Wetland
- Freshwater Pond
- Lake
- Other
- Riverine

This map is for general reference only. The US Fish and Wildlife Service is not responsible for the accuracy or currentness of the base data shown on this map. All wetlands related data should be used in accordance with the layer metadata found on the Wetlands Mapper web site.





Mapped, edited, and published by the Geological Survey in cooperation with Indiana Department of Natural Resources Control by USGS and NDS/INOAA

Topography by photogrammetric methods from aerial photographs taken 1958. Field checked 1961. Revised from aerial photographs taken 1973. Field checked 1976. Map edited 1981

Polyconic projection. 10,000-foot grid ticks based on Indiana coordinate system, west zone

1000-meter Universal Transverse Mercator grid, zone 16 1967 North American Datum

To place on the predicted North American Datum 1983 move the projection lines 3 meters south and 2 meters east as shown by dashed corner ticks

Fine red dashed lines indicate selected fence and field lines where generally visible on aerial photographs. This information is unchecked

Red tint indicate areas in which only landmark buildings are shown



CONTOUR INTERVAL 3 FEET  
DOTTED LINES REPRESENT 5-FOOT CONTOURS  
NATIONAL GEODETIC VERTICAL DATUM OF 1955

THIS MAP COMPLIES WITH NATIONAL MAP ACCURACY STANDARDS  
FOR SALE BY U.S. GEOLOGICAL SURVEY, DENVER, COLORADO 80225 OR RESTON, VIRGINIA 22092,  
AND INDIANA DEPARTMENT OF NATURAL RESOURCES, INDIANAPOLIS, INDIANA 46204  
A FOLDER DESCRIBING TOPOGRAPHIC MAPS AND SYMBOLS IS AVAILABLE ON REQUEST



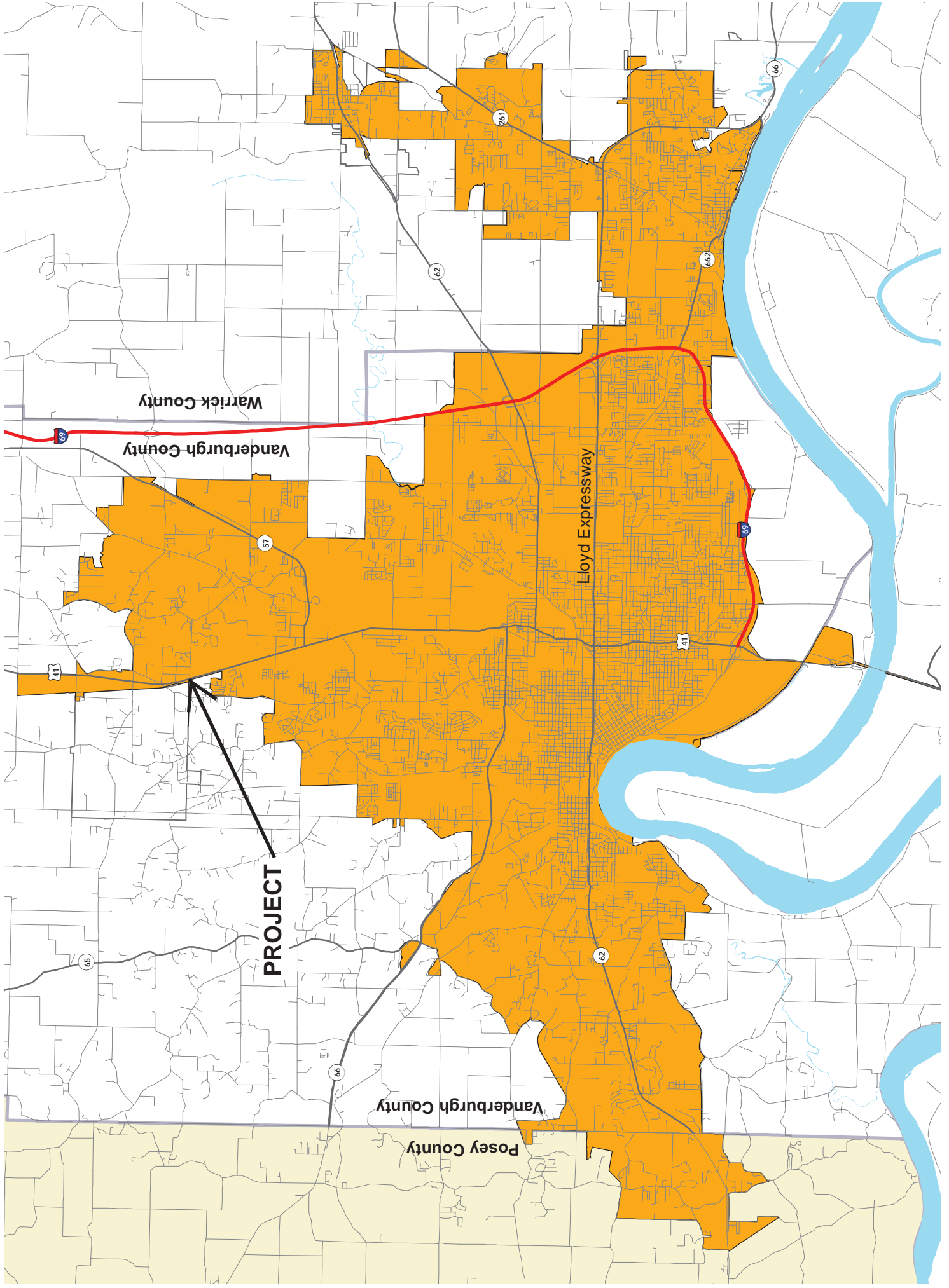
**EVANSVILLE NORTH, IND**  
38087-45-17-024

1981  
PHOTOREVISED - 1988  
DMA 3460 II 86-SERIES V93

Revisions shown in purple and outlined in black in cooperation with State of Indiana agencies from aerial photographs taken 1983 and other sources. This information not field checked  
Map edited 1988



# Indiana Urbanized Boundary



## **APPENDIX M**



Hillsdale Road / US 41: (Looking northwest)



Hillsdale Road / US 41: East Approach (Looking west)



Hillsdale Road / US 41: East Approach (Looking east)





Hillsdale Road / US 41: Median (Looking east)



Hillsdale Road / US 41: Median (Looking south)



Hillsdale Road / US 41: Median (Looking north)





Hillsdale Road / US 41: West Approach (Looking west)



Hillsdale Road / US 41: West Approach (Looking east)





Hillsdale Road at Walnut Road (looking east)



Hillsdale Road at Walnut Road (looking west)



Walnut Road at Hillsdale Road (looking north)



Hillsdale Road at Walnut Road (looking south)







Campbell Road at Walnut Road (looking east)



Campbell Road at Walnut Road (looking west)



Walnut Road at Campbell Road (looking north)

## **APPENDIX N**



HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_AM\_NO BUILD\_Hillisdale-Walnut-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Hillisdale Rd / Walnut Rd  
 Major Street Direction: East-West  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: Walnut Avenue  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Volume		1	72	22		12	102	4	
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		1	78	24		13	111	4	
Percent Heavy Vehicles		3				3			
Number of Lanes	0	0	1	0	0	0	1	0	
Lane Configuration			LTR				LTR		
Median Type					Undivided				
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street:	NorthBound			SouthBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume	22	5	20	4	5	2
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR	24	5	22	4	5	2
Percent Heavy Vehicles	3	3	3	3	3	3
Number of Lanes	0	1	0	0	1	0
Lane Configuration		LTR			LTR	
RT channelized?						
Flared Approach   Storage	No			No		
Percent Grade		0			0	

Pedestrian Volumes and Adjustments

Approach	EB	WB	NB	SB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	EB	WB	NorthBound			SouthBound		
Movement	1U	4U	7	8	9	10	11	12
Lane Configuration				LTR			LTR	
Flow Rate	1	13	51			12		
Lane Capacity	1467	1484	790			701		
v/c	0.00	0.01	0.06			0.02		
95% Queue Length	0.0	0.0	0.2			0.1		
Control Delay	7.5	7.4	9.9			10.2		
LOS	A	A	A			B		
Approach Delay	0.1	0.8	9.9			10.2		
Approach LOS			A			B		
Intersection Delay	2.5							

Step 1: MOVEMENT PRIORITIES

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Priority									

Minor Street:

Approach Priority Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x		1	72	22			12	102	4
Flow Rate, v_x		1	78	24			13	111	4

Minor Street: Approach Movement	NorthBound				SouthBound				
	7 L	8 T	9 R		10 L	11 T	12 R		
Volume, V_x		22	5	20			4	5	2
Flow Rate, v_x		24	5	22			4	5	2

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		1	78	24			13	111	4
Conflicting Flow, v_c, x		115					102		

Minor Street: Approach Movement	NorthBound				SouthBound				
	7 L	8 T	9 R		10 L	11 T	12 R		
Flow Rate, v_x		24	5	22			4	5	2
Conflicting Flow, v_c, x		235	234	90			245	243	113

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_c, base											
Single Stage		4.1		4.1	7.1	6.5	6.2	7.1	6.5	6.2	
Stage I											
Stage II											
t_c, HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	
P_HV		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1	
G					0	0	0	0	0	0	
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
t_c											
Single Stage		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23	
Stage I											
Stage II											

FOLLOW-UP HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_f, base											
t_f, HV		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3	
P_HV		0.9		0.9	0.9	0.9	0.9	0.9	0.9	0.9	
t_f		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
c_p, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound		
					7 L	8 T	9 R	10 L	11 T	12 R
v_c, x		115		102	235	234	90	245	243	113
t_c, x		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23
t_f, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33
c_p, x		1467		1484	717	665	965	707	657	937

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	FR	WR	NR	SB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate v_x Lane Width, w Walking Speed, S_p Pedestrian Blockage Factor, f_pb	0	0	0	0
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j Major L-Shared Probability Queue-free State, p*_0, j		115 1467 1.000 1467 0.999 0.999	102 1484 1.000 1484 0.991 0.991	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j		90 965 1.000 965 0.977	113 937 1.000 937 0.998	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Capacity Adjustment Factor, f_x Movement Capacity, c_m, x Shared L/U Capacity, c_SH Probability of Queue-free State, p_0, j				
Minor-Street Through Movements		8	11	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Capacity Adjustment Factor, f_x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j		234 665 1.000 0.990 658 0.992	243 657 1.000 0.990 650 0.992	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Major L, Minor T Adjusted Impedance Factor, p" Major L, Minor T Impedance Factor, p' Capacity Adjustment Factor, f_x Movement Capacity, c_m, x		235 717 1.000 0.982 0.986 0.984 705	245 707 1.000 0.982 0.986 0.964 681	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	NorthBound			SouthBound		
Movement	7	8	9	10	11	12
Lane Configuration		LTR			LTR	
Shared Flow Rate, v_y		51			12	
Movement Capacity, c_m, x	705	658	965	681	650	937
Shared Capacity, c_SH		790			701	

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		1		13	24	5	22	4	5	2
Movement Capacity		1467		1484	705	658	965	681	650	937
Lane Configuration						LTR			LTR	
Shared Capacity						790			701	
Control Delay		7.5		7.4		9.9			10.2	

CONTROL DELAY TO RANK 1 MOVEMENTS		
Approach	EB	WB
Movement	2	5
Number of Major Street Through Lanes, N	1	1
Proportion of Rank 1 vehicles not blocked, p*_0, j	0.999	0.991
Delay to Major Left-turning Vehicles, d_MLT	7.5	7.4
Major Street Through Vehicles in Shared Lane, v_i1	78	111
Major Street Turning Vehicles in Shared Lane, v_i2	25	17
Saturation Flow Rate for Major Street Through, s_i1	1800	1800



Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

1500  
 0.0

1500  
 0.1

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement Lane Configuration	1U	EB 1	4U	WB 4	7	NorthBound 8	9	10	SouthBound 11	12
						LTR			LTR	
Flow Rate		1		13		51			12	
Lane Capacity		1467		1484		790			701	
v/c		0.00		0.01		0.06			0.02	
95% Queue Length		0.0		0.0		0.2			0.1	
Control Delay		7.5		7.4		9.9			10.2	
LOS		A		A		A			B	
Approach Delay		0.1		0.8		9.9			10.2	
Approach LOS						A			B	
Intersction Delay		2.5								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 11:23:38 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_PM\_NO BUILD\_Hillsdale-Walnut-x.txt  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillsdale  
 Units: U.S. Customary  
 Intersection Name: Hillsdale Rd / Walnut Rd  
 Major Street Direction: East-West  
 East/West Street Name: Hillsdale Road  
 North/South Street Name: Walnut Avenue  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Volume		11	143	22		9	85	20	
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		12	155	24		10	92	22	
Percent Heavy Vehicles		3				3			
Number of Lanes	0	0	1	0	0	0	1	0	
Lane Configuration			LTR				LTR		
Median Type					Undivided				
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street:	NorthBound			SouthBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume	28	9	30	6	7	2
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR	30	10	33	7	8	2
Percent Heavy Vehicles	3	3	3	3	3	3
Number of Lanes	0	1	0	0	1	0
Lane Configuration		LTR			LTR	
RT channelized?						
Flared Approach   Storage	No			No		
Percent Grade		0			0	

Pedestrian Volumes and Adjustments

Approach	EB	WB	NB	SB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	EB	WB	NorthBound			SouthBound		
Movement	1U	4U	7	8	9	10	11	12
Lane Configuration				LTR			LTR	
Flow Rate	12	10	73			16		
Lane Capacity	1469	1390	704			611		
v/c	0.01	0.01	0.10			0.03		
95% Queue Length	0.0	0.0	0.3			0.1		
Control Delay	7.5	7.6	10.7			11.0		
LOS	A	A	B			B		
Approach Delay	0.5	0.7	10.7			11.0		
Approach LOS			B			B		
Intersection Delay	2.8							

Step 1: MOVEMENT PRIORITIES

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Minor Street:									

Approach Priority Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x		11	143	22			9	85	20
Flow Rate, v_x		12	155	24			10	92	22

Minor Street: Approach Movement	NorthBound				SouthBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Volume, V_x		28	9	30		6	7	2
Flow Rate, v_x		30	10	33		7	8	2

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		12	155	24			10	92	22
Conflicting Flow, v_c, x		114					179		

Minor Street: Approach Movement	NorthBound				SouthBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Flow Rate, v_x		30	10	33		7	8	2
Conflicting Flow, v_c, x		319	325	167		335	326	103

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_c, base											
Single Stage		4.1		4.1	7.1	6.5	6.2	7.1	6.5	6.2	
Stage I											
Stage II											
t_c, HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	
P_HV		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1	
G					0	0	0	0	0	0	
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
t_c											
Single Stage		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23	
Stage I											
Stage II											

FOLLOW-UP HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_f, base											
t_f, HV		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3	
P_HV		0.9		0.9	0.9	0.9	0.9	0.9	0.9	0.9	
t_f		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
c_p, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound		
					7 L	8 T	9 R	10 L	11 T	12 R
v_c, x		114		179	319	325	167	335	326	103
t_c, x		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23
t_f, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33
c_p, x		1469		1390	632	591	874	616	591	949

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	FR	WR	NR	SB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate v_x	0	0	0	0
Lane Width, w				
Walking Speed, S_p				
Pedestrian Blockage Factor, f_pb				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, v_c, x		114	179	
Potential Capacity, c_p, x		1469	1390	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Movement Capacity, c_m, x		1469	1390	
Probability of Queue-free State, p_0, j		0.992	0.993	
Major L-Shared Probability Queue-free State, p*_0, j		0.991	0.992	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, v_c, x		167	103	
Potential Capacity, c_p, x		874	949	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Movement Capacity, c_m, x		874	949	
Probability of Queue-free State, p_0, j		0.963	0.998	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, v_c, x				
Potential Capacity, c_p, x				
Capacity Adjustment Factor, f_x				
Movement Capacity, c_m, x				
Shared L/U Capacity, c_SH				
Probability of Queue-free State, p_0, j				
Minor-Street Through Movements		8	11	
Conflicting Flow, v_c, x		325	326	
Potential Capacity, c_p, x		591	591	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Capacity Adjustment Factor, f_x		0.983	0.983	
Movement Capacity, c_m, x		582	581	
Probability of Queue-free State, p_0, j		0.983	0.987	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, v_c, x		319	335	
Potential Capacity, c_p, x		632	616	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, p"		0.971	0.967	
Major L, Minor T Impedance Factor, p'		0.978	0.975	
Capacity Adjustment Factor, f_x		0.975	0.938	
Movement Capacity, c_m, x		616	578	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	NorthBound			SouthBound		
Movement	7	8	9	10	11	12
Lane Configuration		LTR			LTR	
Shared Flow Rate, v_y		73			16	
Movement Capacity, c_m, x	616	582	874	578	581	949
Shared Capacity, c_SH		704			611	

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		12		10	30	10	33	7	8	2
Movement Capacity		1469		1390	616	582	874	578	581	949
Lane Configuration						LTR			LTR	
Shared Capacity						704			611	
Control Delay		7.5		7.6		10.7			11.0	

CONTROL DELAY TO RANK 1 MOVEMENTS		
Approach	EB	WB
Movement	2	5
Number of Major Street Through Lanes, N	1	1
Proportion of Rank 1 vehicles not blocked, p*_0, j	0.991	0.992
Delay to Major Left-turning Vehicles, d_MLT	7.5	7.6
Major Street Through Vehicles in Shared Lane, v_i1	155	92
Major Street Turning Vehicles in Shared Lane, v_i2	36	32
Saturation Flow Rate for Major Street Through, s_i1	1800	1800

Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

1500  
 0.1

1500  
 0.1

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement Lane Configuration	1U	EB 1	4U	WB 4	7	NorthBound 8	9	10	SouthBound 11	12
						LTR			LTR	
Flow Rate		12		10		73			16	
Lane Capacity		1469		1390		704			611	
v/c		0.01		0.01		0.10			0.03	
95% Queue Length		0.0		0.0		0.3			0.1	
Control Delay		7.5		7.6		10.7			11.0	
LOS		A		A		B			B	
Approach Delay		0.5		0.7		10.7			11.0	
Approach LOS						B			B	
Intersection Delay		2.8								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 11:25:53 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_AM\_NO BUILD\_Hillisdale-US 41-x. xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Hillisdale Rd / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R
Volume	0	11	631	25		0	33	888	0
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		12	686	27			36	965	0
Percent Heavy Vehicles	3	3				3	3		
Number of Lanes	0	1	2	0		0	1	2	0
Lane Configuration		L	T	TR			L	T	TR
Median Type					Undivided				
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street: Approach Movement	WestBound 7 L	8 T	9 R		EastBound 10 L	11 T	12 R
Volume	96	8	41		0	22	14
Peak Hour Factor, PHF				0.92			
Hourly Flow Rate, HFR	104	9	45		0	24	15
Percent Heavy Vehicles	3	3	3		3	3	3
Number of Lanes	0	1	0		0	1	0
Lane Configuration		LTR				LTR	
RT channelized?							
Flared Approach   Storage	No				No		
Percent Grade		0				0	

Pedestrian Volumes and Adjustments

Approach Movement	NB 13	SB 14	WB 15	EB 16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach Movement	1U U	NB 1 L	4U L	7 L	8 LTR	9 L	10 L	EastBound 11 LTR	12 L
Lane Configuration									
Flow Rate	12		36		158			39	
Lane Capacity	703		876		111			115	
v/c	0.02		0.04		1.42			0.34	
95% Queue Length	0.1		0.1		11.1			1.4	
Control Delay	10.2		9.3		302.4			51.9	
LOS	B		A		F			F	
Approach Delay	0.2		0.3		302.4			51.9	
Approach LOS					F			F	
Intersction Delay	26.1								

Step 1: MOVEMENT PRIORITIES

Major Street: Approach Priority Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R
Minor Street:									

Approach Priority Movement	WestBound				EastBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x	0	11	631	25	0	33	888	0	
Flow Rate, v_x		12	686	27		36	965	0	

Minor Street: Approach Movement	WestBound				EastBound		
	7 L	8 T	9 R		10 L	11 T	12 R
Volume, V_x	96	8	41	0	22	14	
Flow Rate, v_x	104	9	45	0	24	15	

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		12	686	27		36	965	0	
Conflicting Flow, v_c, x		965				713			

Minor Street: Approach Movement	WestBound				EastBound		
	7 L	8 T	9 R		10 L	11 T	12 R
Flow Rate, v_x	104	9	45	0	24	15	
Conflicting Flow, v_c, x	1290	1760	357	1408	1774	483	

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	NB 1U U	1 L	SB 4U U	4 L	7 L	WestBound		10 L	EastBound	
						8 T	9 R		11 T	12 R
t_c, base										
Single Stage		4.1		4.1	7.5	6.5	6.9	7.5	6.5	6.9
Stage I										
Stage II										
t_c, HV		2.0		2.0	2.0	2.0	2.0	2.0	2.0	2.0
P_HV		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1
G					0	0	0	0	0	0
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0
t_c										
Single Stage		4.16		4.16	7.56	6.56	6.96	7.56	6.56	6.96
Stage I										
Stage II										

FOLLOW-UP HEADWAYS Approach Movement	NB 1U U	1 L	SB 4U U	4 L	7 L	WestBound		10 L	EastBound	
						8 T	9 R		11 T	12 R
t_f, base										
t_f, HV		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3
P_HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0
t_f		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03
c_p, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	NB 1U U	1 L	SB 4U U	4 L	7 L	WestBound		10 L	EastBound	
						8 T	9 R		11 T	12 R
v_c, x		965		713	1290	1760	357	1408	1774	483
t_c, x		4.16		4.16	7.56	6.56	6.96	7.56	6.56	6.96
t_f, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33
c_p, x		703		876	120	83	637	98	81	527

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	NR	SR	WR	EB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate v_x Lane Width, w Walking Speed, S_p Pedestrian Blockage Factor, f_pb	0	0	0	0
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j Major L-Shared Probability Queue-free State, p*_0, j		965 703 1.000 703 0.983	713 876 1.000 876 0.959	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j		357 637 1.000 637 0.930	483 527 1.000 527 0.971	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Capacity Adjustment Factor, f_x Movement Capacity, c_m, x Shared L/U Capacity, c_SH Probability of Queue-free State, p_0, j				
Minor-Street Through Movements		8	11	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Capacity Adjustment Factor, f_x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j		1760 83 1.000 0.943 78 0.889	1774 81 1.000 0.943 76 0.687	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Major L, Minor T Adjusted Impedance Factor, p" Major L, Minor T Impedance Factor, p' Capacity Adjustment Factor, f_x Movement Capacity, c_m, x		1290 120 1.000 0.648 0.727 0.706 85	1408 98 1.000 0.838 0.875 0.814 80	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES										
Approach	WestBound				EastBound					
Movement	7	8	9	10	11	12				
Lane Configuration	LTR				LTR					
Shared Flow Rate, v_y		158			39					
Movement Capacity, c_m, x	85	78	637	80	76	527				
Shared Capacity, c_SH		111			115					

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		12		36	104	9	45	0	24	15
Movement Capacity		703		876	85	78	637	80	76	527
Lane Configuration		L		L		LTR			LTR	
Shared Capacity						111			115	
Control Delay		10.2		9.3		302.4			51.9	

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration	L		L		LTR			LTR		
Flow Rate		12		36		158			39	
Lane Capacity		703		876		111			115	
v/c		0.02		0.04		1.42			0.34	
95% Queue Length		0.1		0.1		11.1			1.4	
Control Delay		10.2		9.3		302.4			51.9	



LOS	B	A	F	F
Approach Delay	0.2	0.3	302.4	51.9
Approach LOS			F	F
Intersection Delay	26.1			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 11:32:00 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_PM\_NO BUILD\_HILLSDALE-US 41-x. xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillsdale  
 Units: U. S. Customary  
 Intersection Name: Hillsdale Rd / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Hillsdale Road  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R
Volume	0	20	958	42		0	52	1025	1
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		22	1041	46			57	1114	1
Percent Heavy Vehicles	3	3				3	3		
Number of Lanes	0	1	2	0		0	1	2	0
Lane Configuration		L	T	TR			L	T	TR
Median Type					Undivided				
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street: Approach Movement	WestBound 7 L	8 T	9 R		EastBound 10 L	11 T	12 R
Volume	41	21	44		3	21	6
Peak Hour Factor, PHF				0.92			
Hourly Flow Rate, HFR	45	23	48		3	23	7
Percent Heavy Vehicles	3	3	3		3	3	3
Number of Lanes	0	1	0		0	1	0
Lane Configuration		LTR				LTR	
RT channelized?							
Flared Approach   Storage	No				No		
Percent Grade		0				0	

Pedestrian Volumes and Adjustments

Approach Movement	NB 13	SB 14	WB 15	EB 16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach Movement	1U U	NB 1 L	4U L	7 L	8 LTR	9 L	10 L	EastBound 11 LTR	12 L
Lane Configuration									
Flow Rate		22	57		115			33	
Lane Capacity		616	632		34			34	
v/c		0.04	0.09		3.35			0.96	
95% Queue Length		0.1	0.3		13.3			3.4	
Control Delay		11.1	11.3		1301.9			317.0	
LOS		B	B		F			F	
Approach Delay		0.2	0.5		1301.9			317.0	
Approach LOS					F			F	
Intersection Delay		66.4							

Step 1: MOVEMENT PRIORITIES

Major Street: Approach Priority Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R
Minor Street:									

Approach Priority Movement	WestBound				EastBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x	0	20	958	42		0	52	1025	1
Flow Rate, v_x		22	1041	46			57	1114	1

Minor Street: Approach Movement	WestBound				EastBound		
	7 L	8 T	9 R		10 L	11 T	12 R
Volume, V_x	41	21	44		3	21	6
Flow Rate, v_x	45	23	48		3	23	7

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		22	1041	46			57	1114	1
Conflicting Flow, v_c, x		1115					1087		

Minor Street: Approach Movement	WestBound				EastBound		
	7 L	8 T	9 R		10 L	11 T	12 R
Flow Rate, v_x	45	23	48		3	23	7
Conflicting Flow, v_c, x	1789	2336	543		1803	2358	558

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	NB 1U U	1 L	SB 4U U	4 L	7 L	WestBound			EastBound		
						8 T	9 R	10 L	11 T	12 R	
t_c, base Single Stage Stage I Stage II		4.1		4.1	7.5	6.5	6.9	7.5	6.5	6.9	
t_c, HV		2.0		2.0	2.0	2.0	2.0	2.0	2.0	2.0	
P_HV		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1	
G					0	0	0	0	0	0	
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
t_c Single Stage Stage I Stage II		4.16		4.16	7.56	6.56	6.96	7.56	6.56	6.96	

FOLLOW-UP HEADWAYS Approach Movement	NB 1U U	1 L	SB 4U U	4 L	7 L	WestBound			EastBound		
						8 T	9 R	10 L	11 T	12 R	
t_f, base		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3	
t_f, HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	
P_HV		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
t_f		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	NB 1U U	1 L	SB 4U U	4 L	7 L	WestBound			EastBound		
						8 T	9 R	10 L	11 T	12 R	
v_c, x		1115		1087	1789	2336	543	1803	2358	558	
t_c, x		4.16		4.16	7.56	6.56	6.96	7.56	6.56	6.96	
t_f, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33	
c_p, x		616		632	51	36	481	49	35	471	

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	NR	SR	WR	EB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		1115	1087	
Potential Capacity, $c_{p,x}$		616	632	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		616	632	
Probability of Queue-free State, $p_{0,j}$		0.965	0.911	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		543	558	
Potential Capacity, $c_{p,x}$		481	471	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		481	471	
Probability of Queue-free State, $p_{0,j}$		0.901	0.986	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$		2336	2358	
Potential Capacity, $c_{p,x}$		36	35	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Capacity Adjustment Factor, $f_x$		0.878	0.878	
Movement Capacity, $c_{m,x}$		31	30	
Probability of Queue-free State, $p_{0,j}$		0.272	0.248	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$		1789	1803	
Potential Capacity, $c_{p,x}$		51	49	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$		0.218	0.239	
Major L, Minor T Impedance Factor, $p'$		0.354	0.375	
Capacity Adjustment Factor, $f_x$		0.349	0.338	
Movement Capacity, $c_{m,x}$		18	17	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	WestBound			EastBound		
Movement	7	8	9	10	11	12
Lane Configuration		LTR			LTR	
Shared Flow Rate, $v_y$		115			33	
Movement Capacity, $c_{m,x}$	18	31	481	17	30	471
Shared Capacity, $c_{SH}$		34			34	

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS											
Approach	NB			SB			WestBound			EastBound	
	1U	1	4U	4	7	8	9	10	11	12	
Flow Rate		22		57	45	23	48	3	23	7	
Movement Capacity		616		632	18	31	481	17	30	471	
Lane Configuration		L		L		LTR			LTR		
Shared Capacity						34			34		
Control Delay		11.1		11.3		1301.9			317.0		

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB			SB			WestBound			EastBound	
	1U	1	4U	4	7	8	9	10	11	12	
Lane Configuration		L		L		LTR			LTR		
Flow Rate		22		57		115			33		
Lane Capacity		616		632		34			34		
$v/c$		0.04		0.09		3.35			0.96		
95% Queue Length		0.1		0.3		13.3			3.4		
Control Delay		11.1		11.3		1301.9			317.0		

LOS	B	B	F	F
Approach Delay	0.2	0.5	1301.9	317.0
Approach LOS			F	F
Intersection Delay	66.4			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 11:39:18 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_AM\_NO BUILD\_Hillisdale-Old State-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Hillisdale Rd / Old State  
 Major Street Direction: North-South  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: Old State Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound				SouthBound			
Approach	1U	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R
Volume			97	23		17	60	
Peak Hour Factor, PHF					0.92			
Hourly Flow Rate, HFR			105	25		18	65	
Percent Heavy Vehicles						3		
Number of Lanes	0	0	1	0	0	0	1	0
Lane Configuration				TR		LT		
Median Type					Undivided			
Median Storage								
RT channelized?								
Left-Turn Lane Storage								
Upstream Signal?					Not Present			

Minor Street:	WestBound			EastBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume	31		6			
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR	34		7			
Percent Heavy Vehicles	3		3			
Number of Lanes	0	1	0	0	0	0
Lane Configuration		LR				
RT channelized?						
Flared Approach   Storage	No					
Percent Grade		0				

Pedestrian Volumes and Adjustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	NB	SB	WestBound	EastBound
Movement	1U	4U	7	8
Lane Configuration		LT	LR	
Flow Rate		18	40	
Lane Capacity		1449	780	
v/c		0.01	0.05	
95% Queue Length		0.0	0.2	
Control Delay		7.5	9.9	
LOS		A	A	
Approach Delay		1.7	9.9	
Approach LOS			A	
Intersection Delay	2.1			

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound				SouthBound			
Approach	1U	2	3	4U	4	5	6	
Priority	U	L	T	R	U	L	T	R
Movement								

Minor Street:

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement	1U	NorthBound				4U	SouthBound		
	U	1	2	3		U	4	5	6
		L	T	R			L	T	R
Volume, V_x		97				17		60	
Flow Rate, v_x		105				18		65	

Minor Street:								
Approach Movement		WestBound				EastBound		
		7	8	9		10	11	12
		L	T	R		L	T	R
Volume, V_x		31				6		
Flow Rate, v_x		34				7		

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement	1U	NorthBound				4U	SouthBound		
	U	1	2	3		U	4	5	6
		L	T	R			L	T	R
Flow Rate, v_x		105				18		65	
Conflicting Flow, v_c, x						130			

Minor Street:								
Approach Movement		WestBound				EastBound		
		7	8	9		10	11	12
		L	T	R		L	T	R
Flow Rate, v_x		34				7		
Conflicting Flow, v_c, x		220				118		

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS											
Approach Movement		NB		SB		WestBound			EastBound		
	1U	1		4U	4	7	8	9	10	11	12
	U	L		U	L	L	T	R	L	T	R
t_c, base											
Single Stage					4.1	7.1		6.2			
Stage I											
Stage II											
t_c, HV					1.0	1.0		1.0			
P_HV					0.03	0.03		0.03			
t_c, G						0.2		0.1			
G						0		0			
t_3, LT					0.0	0.7		0.0			
t_c											
Single Stage					4.13	6.43		6.23			
Stage I											
Stage II											

FOLLOW-UP HEADWAYS											
Approach Movement		NB		SB		WestBound			EastBound		
	1U	1		4U	4	7	8	9	10	11	12
	U	L		U	L	L	T	R	L	T	R
t_f, base					2.2	3.5		3.3			
t_f, HV					0.9	0.9		0.9			
P_HV					0.03	0.03		0.03			
t_f					2.23	3.53		3.33			

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT											
Approach Movement		NB		SB		WestBound			EastBound		
	1U	1		4U	4	7	8	9	10	11	12
	U	L		U	L	L	T	R	L	T	R
v_c, x					130	220		118			
t_c, x					4.13	6.43		6.23			
t_f, x					2.23	3.53		3.33			
c_p, x					1449	766		931			

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach		NR		SR		WR		EB
-------------------------------	--	----	--	----	--	----	--	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$ Lane Width, $w$ Walking Speed, $S_p$ Pedestrian Blockage Factor, $f_{pb}$	0	0	0	
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$ Major L-Shared Probability Queue-free State, $p^*_{0,j}$			130 1449 1.000 1449 0.987 0.987	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$		118 931 1.000 931 0.993		
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Shared L/U Capacity, $c_{SH}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Major L, Minor T Adjusted Impedance Factor, $p''$ Major L, Minor T Impedance Factor, $p'$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$		220 766 1.000   0.987 756		

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES										
Approach	WestBound							EastBound		
Movement	7	8	9		10	11	12			
Lane Configuration		LR								
Shared Flow Rate, $v_y$		40								
Movement Capacity, $c_{m,x}$	756		931							
Shared Capacity, $c_{SH}$		780								

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate							7			
Movement Capacity							931			
Lane Configuration						LR				
Shared Capacity						780				
Control Delay				7.5		9.9				

CONTROL DELAY TO RANK 1 MOVEMENTS										
Approach					NB		SB			
Movement					2		5			
Number of Major Street Through Lanes, $N$					1		1			
Proportion of Rank 1 vehicles not blocked, $p^*_{0,j}$							0.987			
Delay to Major Left-turning Vehicles, $d_{MLT}$							7.5			
Major Street Through Vehicles in Shared Lane, $v_{i1}$							65			
Major Street Turning Vehicles in Shared Lane, $v_{i2}$							18			
Saturation Flow Rate for Major Street Through, $s_{i1}$						1800	1800			



Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

1500

0.1

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement	1U	NB 1	4U	SB 4	7	WestBound 8	9	10	EastBound 11	12
Lane Configuration				LT		LR				
Flow Rate				18		40				
Lane Capacity				1449		780				
v/c				0.01		0.05				
95% Queue Length				0.0		0.2				
Control Delay				7.5		9.9				
LOS				A		A				
Approach Delay				1.7		9.9				
Approach LOS						A				
Intersection Delay		2.1								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 12:40:04 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_PM\_NO BUILD\_Hillisdale-Old State-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Hillisdale Rd / Old State  
 Major Street Direction: North-South  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: Old State Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound				SouthBound			
Approach	1U	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R
Volume		94	30			9	123	
Peak Hour Factor, PHF				0.92				
Hourly Flow Rate, HFR		102	33			10	134	
Percent Heavy Vehicles						3		
Number of Lanes	0	0	1	0	0	0	1	0
Lane Configuration				TR		LT		
Median Type					Undivided			
Median Storage								
RT channelized?								
Left-Turn Lane Storage								
Upstream Signal?					Not Present			

Minor Street:	WestBound			EastBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume	16		11			
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR	17		12			
Percent Heavy Vehicles	3		3			
Number of Lanes	0	1	0	0	0	0
Lane Configuration		LR				
RT channelized?						
Flared Approach   Storage	No					
Percent Grade		0				

Pedestrian Volumes and Adjustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	NB	SB	WestBound	EastBound
Movement	1U	4U	7	8
Lane Configuration	1	4U	7	8
	1U	4U	7	8
	U	L	T	R
Flow Rate		10	29	
Lane Capacity		1443	786	
v/c		0.01	0.04	
95% Queue Length		0.0	0.1	
Control Delay		7.5	9.8	
LOS		A	A	
Approach Delay		0.6	9.8	
Approach LOS			A	
Intersection Delay	1.2			

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound				SouthBound			
Approach	1U	2	3	4U	4	5	6	
Priority	U	L	T	R	U	L	T	R
Movement	U	L	T	R	U	L	T	R

Minor Street:

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:							
Approach Movement	NorthBound				SouthBound		
	1U	2	3		4U	5	6
	U	L	T		U	L	R
Volume, V_x	94				123		
Flow Rate, v_x	102				134		

Minor Street:							
Approach Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Volume, V_x	16				11		
Flow Rate, v_x	17				12		

Step 3: CONFLICTING FLOW RATES

Major Street:							
Approach Movement	NorthBound				SouthBound		
	1U	2	3		4U	5	6
	U	L	T		U	L	R
Flow Rate, v_x	102				134		
Conflicting Flow, v_c, x	272				135		

Minor Street:							
Approach Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Flow Rate, v_x	17				12		
Conflicting Flow, v_c, x	272				118		

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_c, base			4.1		7.1			6.2		
Single Stage										
Stage I										
Stage II										
t_c, HV			1.0		1.0			1.0		
P_HV			0.03		0.03			0.03		
t_c, G					0.2			0.1		
G					0			0		
t_3, LT			0.0		0.7			0.0		
t_c			4.13		6.43			6.23		
Single Stage										
Stage I										
Stage II										

FOLLOW-UP HEADWAYS										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_f, base			2.2		3.5			3.3		
t_f, HV			0.9		0.9			0.9		
P_HV			0.03		0.03			0.03		
t_f			2.23		3.53			3.33		

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
v_c, x			135		272			118		
t_c, x			4.13		6.43			6.23		
t_f, x			2.23		3.53			3.33		
c_p, x			1443		716			931		

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	NR	SR	WR	EB
-------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$ Lane Width, $w$ Walking Speed, $S_p$ Pedestrian Blockage Factor, $f_{pb}$	0	0	0	
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$ Major L-Shared Probability Queue-free State, $p^*_{0,j}$			135 1443 1.000 1443 0.993 0.993	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$		118 931 1.000 931 0.987		
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Shared L/U Capacity, $c_{SH}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Major L, Minor T Adjusted Impedance Factor, $p''$ Major L, Minor T Impedance Factor, $p'$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$		272 716 1.000   0.993 710		

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES											
Approach	WestBound				EastBound						
Movement	7	8	9	10	11	12					
Lane Configuration		LR									
Shared Flow Rate, $v_y$		29									
Movement Capacity, $c_{m,x}$	710		931								
Shared Capacity, $c_{SH}$		786									

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS												
Approach	NB				SB				EastBound			
	1U	1	4U	4	7	8	9	10	11	12		
Movement						LR						
Flow Rate							12					
Movement Capacity						710	931					
Lane Configuration						LR						
Shared Capacity						786						
Control Delay				7.5		9.8						

CONTROL DELAY TO RANK 1 MOVEMENTS												
Approach					NB				SB			
Movement					2				5			
Number of Major Street Through Lanes, $N$					1				1			
Proportion of Rank 1 vehicles not blocked, $p^*_{0,j}$									0.993			
Delay to Major Left-turning Vehicles, $d_{MLT}$									7.5			
Major Street Through Vehicles in Shared Lane, $v_{i1}$									134			
Major Street Turning Vehicles in Shared Lane, $v_{i2}$									10			
Saturation Flow Rate for Major Street Through, $s_{i1}$					1800				1800			

Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

1500

0.1

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement	1U	NB 1	4U	SB 4	7	WestBound 8	9	10	EastBound 11	12
Lane Configuration				LT		LR				
Flow Rate				10		29				
Lane Capacity				1443		786				
v/c				0.01		0.04				
95% Queue Length				0.0		0.1				
Control Delay				7.5		9.8				
LOS				A		A				
Approach Delay				0.6		9.8				
Approach LOS						A				
Intersection Delay		1.2								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 12:37:51 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_AM\_NO BUILD\_Radio-Walnut-x. xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Radio Ave / Walnut Rd  
 Major Street Direction: East-West  
 East/West Street Name: Radio Ave  
 North/South Street Name: Walnut Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Volume		2	2	1		3	1	1	
Peak Hour Factor, PHF					0.92				
Hourly Flow Rtae, HFR		2	2	1		3	1	1	
Percent Heavy Vehicles		3				3			
Number of Lanes	0	0	1	0	0	0	1	0	
Lane Configuration			LTR				LTR		
Median Type					Undivided				
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street:	NorthBound			SouthBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume	7	12	7	1	3	4
Peak Hour Factor, PHF				0.92		
Hourly Flow Rtae, HFR	8	13	8	1	3	4
Percent Heavy Vehicles	3	3	3	3	3	3
Number of Lanes	0	1	0	0	1	0
Lane Configuration		LTR			LTR	
RT channelized?						
Flared Approach   Storage	No			No		
Percent Grade		0			0	

Pedestrian Volumes and Adjustments

Approach	EB	WB	NB	SB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	EB	WB	NorthBound			SouthBound		
Movement	1U	4U	7	8	9	10	11	12
Lane Configuration				LTR			LTR	
Flow Rate	2	3	28			9		
Lane Capacity	1614	1612	951			978		
v/c	0.00	0.00	0.03			0.01		
95% Queue Length	0.0	0.0	0.1			0.0		
Control Delay	7.2	7.2	8.9			8.7		
LOS	A	A	A			A		
Approach Delay	2.9	4.3	8.9			8.7		
Approach LOS			A			A		
Intersction Delay	7.7							

Step 1: MOVEMENT PRIORITIES

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Priority	U	L	T	R	U	L	T	R	
Minor Street:									

Approach Priority Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x		2	2	1			3	1	1
Flow Rate, v_x		2	2	1			3	1	1

Minor Street: Approach Movement	NorthBound				SouthBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Volume, V_x		7	12	7		1	3	4
Flow Rate, v_x		8	13	8		1	3	4

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		2	2	1			3	1	1
Conflicting Flow, v_c, x		2	2	1			3	1	1

Minor Street: Approach Movement	NorthBound				SouthBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Flow Rate, v_x		8	13	8		1	3	4
Conflicting Flow, v_c, x		19	16	3		26	16	2

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_c, base											
Single Stage		4.1		4.1	7.1	6.5	6.2	7.1	6.5	6.2	
Stage I											
Stage II											
t_c, HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	
P_HV		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1	
G					0	0	0	0	0	0	
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
t_c											
Single Stage		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23	
Stage I											
Stage II											

FOLLOW-UP HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_f, base											
t_f, HV		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3	
P_HV		0.9		0.9	0.9	0.9	0.9	0.9	0.9	0.9	
t_f		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
c_p, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound		
					7 L	8 T	9 R	10 L	11 T	12 R
v_c, x		2		3	19	16	3	26	16	2
t_c, x		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23
t_f, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33
c_p, x		1614		1612	992	876	1078	983	876	1080

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	FR	WR	NR	SB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate v_x	0	0	0	0
Lane Width, w				
Walking Speed, S_p				
Pedestrian Blockage Factor, f_pb				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, v_c, x		2	3	
Potential Capacity, c_p, x		1614	1612	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Movement Capacity, c_m, x		1614	1612	
Probability of Queue-free State, p_0, j		0.999	0.998	
Major L-Shared Probability Queue-free State, p*_0, j		0.999	0.998	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, v_c, x		3	2	
Potential Capacity, c_p, x		1078	1080	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Movement Capacity, c_m, x		1078	1080	
Probability of Queue-free State, p_0, j		0.993	0.996	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, v_c, x				
Potential Capacity, c_p, x				
Capacity Adjustment Factor, f_x				
Movement Capacity, c_m, x				
Shared L/U Capacity, c_SH				
Probability of Queue-free State, p_0, j				
Minor-Street Through Movements		8	11	
Conflicting Flow, v_c, x		16	16	
Potential Capacity, c_p, x		876	876	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Capacity Adjustment Factor, f_x		0.997	0.997	
Movement Capacity, c_m, x		873	873	
Probability of Queue-free State, p_0, j		0.985	0.996	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, v_c, x		19	26	
Potential Capacity, c_p, x		992	983	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, p"		0.993	0.982	
Major L, Minor T Impedance Factor, p'		0.995	0.986	
Capacity Adjustment Factor, f_x		0.991	0.979	
Movement Capacity, c_m, x		983	962	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	NorthBound			SouthBound		
Movement	7	8	9	10	11	12
Lane Configuration		LTR			LTR	
Shared Flow Rate, v_y		28			9	
Movement Capacity, c_m, x	983	873	1078	962	873	1080
Shared Capacity, c_SH		951			978	

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		2		3	8	13	8	1	3	4
Movement Capacity		1614		1612	983	873	1078	962	873	1080
Lane Configuration						LTR			LTR	
Shared Capacity						951			978	
Control Delay		7.2		7.2		8.9			8.7	

CONTROL DELAY TO RANK 1 MOVEMENTS		
Approach	EB	WB
Movement	2	5
Number of Major Street Through Lanes, N	1	1
Proportion of Rank 1 vehicles not blocked, p*_0, j	0.999	0.998
Delay to Major Left-turning Vehicles, d_MLT	7.2	7.2
Major Street Through Vehicles in Shared Lane, v_i1	2	1
Major Street Turning Vehicles in Shared Lane, v_i2	3	4
Saturation Flow Rate for Major Street Through, s_i1	1800	1800



Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

1500  
 0.0

1500  
 0.0

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement Lane Configuration	1U	EB 1	4U	WB 4	7	NorthBound 8	9	10	SouthBound 11	12
						LTR			LTR	
Flow Rate		2		3		28			9	
Lane Capacity		1614		1612		951			978	
v/c		0.00		0.00		0.03			0.01	
95% Queue Length		0.0		0.0		0.1			0.0	
Control Delay		7.2		7.2		8.9			8.7	
LOS		A		A		A			A	
Approach Delay		2.9		4.3		8.9			8.7	
Approach LOS						A			A	
Intersection Delay		7.7								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 12:45:54 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_PM\_NO BUILD\_Radio-Walnut-x. xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Radio Ave / Walnut Rd  
 Major Street Direction: East-West  
 East/West Street Name: Radio Ave  
 North/South Street Name: Walnut Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	1U U	EastBound 1 L 2 T 3 R	4U U	WestBound 4 L 5 T 6 R
Volume		0 4 0		1 4 3
Peak Hour Factor, PHF			0.92	
Hourly Flow Rate, HFR		0 4 0		1 4 3
Percent Heavy Vehicles		3		3
Number of Lanes	0	0 1 0	0	0 1 0
Lane Configuration		LTR		LTR
Median Type			Undivided	
Median Storage				
RT channelized?				
Left-Turn Lane Storage				
Upstream Signal?			Not Present	

Minor Street: Approach Movement	NorthBound 7 L 8 T 9 R	SouthBound 10 L 11 T 12 R
Volume	6 10 1	1 10 1
Peak Hour Factor, PHF		0.92
Hourly Flow Rate, HFR	7 11 1	1 11 1
Percent Heavy Vehicles	3 3 3	3 3 3
Number of Lanes	0 1 0	0 1 0
Lane Configuration	LTR	LTR
RT channelized?		
Flared Approach   Storage	No   0	No   0
Percent Grade		

Pedestrian Volumes and Adjustments

Approach Movement	EB 13	WB 14	NB 15	SB 16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach Movement	1U U	EB 1	4U U	WB 4	7 7	NorthBound 8 LTR	9 9	10 10	SouthBound 11 LTR	12 12
Lane Configuration						LTR			LTR	
Flow Rate		0		1		18			13	
Lane Capacity		1606		1611		922			901	
v/c		0.00		0.00		0.02			0.01	
95% Queue Length		0.0		0.0		0.1			0.0	
Control Delay		7.2		7.2		9.0			9.1	
LOS		A		A		A			A	
Approach Delay		0.0		0.9		9.0			9.1	
Approach LOS						A			A	
Intersection Delay		6.6								

Step 1: MOVEMENT PRIORITIES

Major Street: Approach Priority Movement	1U U	EastBound 1 L 2 T 3 R	4U U	WestBound 4 L 5 T 6 R
Minor Street:				

Approach Priority Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x		0	4	0			1	4	3
Flow Rate, v_x		0	4	0			1	4	3

Minor Street: Approach Movement	NorthBound				SouthBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Volume, V_x		6	10	1		1	10	1
Flow Rate, v_x		7	11	1		1	11	1

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		0	4	0			1	4	3
Conflicting Flow, v_c, x		8					4		

Minor Street: Approach Movement	NorthBound				SouthBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Flow Rate, v_x		7	11	1		1	11	1
Conflicting Flow, v_c, x		18	14	4		18	13	6

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_c, base											
Single Stage		4.1		4.1	7.1	6.5	6.2	7.1	6.5	6.2	
Stage I											
Stage II											
t_c, HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	
P_HV		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1	
G					0	0	0	0	0	0	
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
t_c											
Single Stage		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23	
Stage I											
Stage II											

FOLLOW-UP HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_f, base											
t_f, HV		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3	
P_HV		0.9		0.9	0.9	0.9	0.9	0.9	0.9	0.9	
t_f		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
c_p, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound		
					7 L	8 T	9 R	10 L	11 T	12 R
v_c, x		8		4	18	14	4	18	13	6
t_c, x		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23
t_f, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33
c_p, x		1606		1611	993	878	1076	993	880	1074

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	FR	WR	NR	SB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate v_x Lane Width, w Walking Speed, S_p Pedestrian Blockage Factor, f_pb	0	0	0	0
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j Major L-Shared Probability Queue-free State, p*_0, j		8 1606 1.000 1606 1.000 1.000	4 1611 1.000 1611 0.999 0.999	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j		4 1076 1.000 1076 0.999	6 1074 1.000 1074 0.999	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Capacity Adjustment Factor, f_x Movement Capacity, c_m, x Shared L/U Capacity, c_SH Probability of Queue-free State, p_0, j				
Minor-Street Through Movements		8	11	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Capacity Adjustment Factor, f_x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j		14 878 1.000 0.999 878 0.988	13 880 1.000 0.999 879 0.988	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Major L, Minor T Adjusted Impedance Factor, p" Major L, Minor T Impedance Factor, p' Capacity Adjustment Factor, f_x Movement Capacity, c_m, x		18 993 1.000 0.987 0.990 0.989 982	18 993 1.000 0.987 0.990 0.989 982	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	NorthBound			SouthBound		
Movement	7	8	9	10	11	12
Lane Configuration		LTR			LTR	
Shared Flow Rate, v_y		18			13	
Movement Capacity, c_m, x	982	878	1076	982	879	1074
Shared Capacity, c_SH		922			901	

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		0		1	7	11	1	1	11	1
Movement Capacity		1606		1611	982	878	1076	982	879	1074
Lane Configuration						LTR			LTR	
Shared Capacity						922			901	
Control Delay		7.2		7.2		9.0			9.1	

CONTROL DELAY TO RANK 1 MOVEMENTS		
Approach	EB	WB
Movement	2	5
Number of Major Street Through Lanes, N	1	1
Proportion of Rank 1 vehicles not blocked, p*_0, j	1.000	0.999
Delay to Major Left-turning Vehicles, d_MLT	7.2	7.2
Major Street Through Vehicles in Shared Lane, v_i1	4	4
Major Street Turning Vehicles in Shared Lane, v_i2	0	4
Saturation Flow Rate for Major Street Through, s_i1	1800	1800

Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

1500  
 0.0

1500  
 0.0

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement Lane Configuration	1U	EB 1	4U	WB 4	7	NorthBound 8	9	10	SouthBound 11	12
						LTR			LTR	
Flow Rate		0		1		18			13	
Lane Capacity		1606		1611		922			901	
v/c		0.00		0.00		0.02			0.01	
95% Queue Length		0.0		0.0		0.1			0.0	
Control Delay		7.2		7.2		9.0			9.1	
LOS		A		A		A			A	
Approach Delay		0.0		0.9		9.0			9.1	
Approach LOS						A			A	
Intersection Delay		6.6								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 12:48:06 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_AM\_NO BUILD\_Radio-US 41-x. xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Radio Ave / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Radio Ave  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound			SouthBound		
Approach Movement	1U	2	3	4U	5	6
	U	L	T	U	L	T
Volume		851	4	0	0	922
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR		925	4		0	1002
Percent Heavy Vehicles				0	3	
Number of Lanes	0	0	2	0	1	2
Lane Configuration			T		L	T
Median Type				Undivided		
Median Storage						
RT channelized?						
Left-Turn Lane Storage						
Upstream Signal?				Not Present		

Minor Street:	WestBound			EastBound		
Approach Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	4		14			
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR	4		15			
Percent Heavy Vehicles	3		3			
Number of Lanes	1	0	1	0	0	0
Lane Configuration	L		R			
RT channelized?			No			
Flared Approach   Storage						
Percent Grade		0				

Pedestrian Volumes and Adjustments

Approach Movement	NB	SB	WB	EB
	13	14	15	16
Flow (ped/hr)	0	0	0	
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach Movement	NB	SB	WestBound	EastBound
	1	4U	7	10
	1	L	8	11
			9	12
			R	
Flow Rate		0	4	15
Lane Capacity		725	125	542
v/c		0.00	0.03	0.03
95% Queue Length		0.0	0.1	0.1
Control Delay		10.0	34.9	11.8
LOS		A	D	B
Approach Delay		0.0		17.0
Approach LOS				C
Intersection Delay				

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound			SouthBound		
Approach Priority Movement	1U	2	3	4U	5	6
	U	L	T	U	L	T
Minor Street:						

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:								
Approach Movement	1U	NorthBound			4U	SouthBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Volume, V_x		851			0	922		
Flow Rate, v_x		925			4	1002		

Minor Street:								
Approach Movement	WestBound				EastBound			
		7	8	9		10	11	12
		L	T	R		L	T	R
Volume, V_x	4			14	0			
Flow Rate, v_x	4			15	0			

Step 3: CONFLICTING FLOW RATES

Major Street:								
Approach Movement	1U	NorthBound			4U	SouthBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Flow Rate, v_x		925			4	1002		
Conflicting Flow, v_c, x					0	929		

Minor Street:								
Approach Movement	WestBound				EastBound			
		7	8	9		10	11	12
		L	T	R		L	T	R
Flow Rate, v_x	4			15	0			
Conflicting Flow, v_c, x	1428			465	0			

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_c, base										
Single Stage				4.1	7.5		6.9			
Stage I										
Stage II										
t_c, HV				2.0	2.0		2.0			
P_HV				0.03	0.03		0.03			
t_c, G					0.2		0.1			
G					0		0			
t_3, LT				0.0	0.7		0.0			
t_c										
Single Stage				4.16	6.86		6.96			
Stage I										
Stage II										

FOLLOW-UP HEADWAYS										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_f, base				2.2	3.5		3.3			
t_f, HV				1.0	1.0		1.0			
P_HV				0.03	0.03		0.03			
t_f				2.23	3.53		3.33			

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
v_c, x				929	1428		465			
t_c, x				4.16	6.86		6.96			
t_f, x				2.23	3.53		3.33			
c_p, x				725	125		542			

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$ Lane Width, $w$ Walking Speed, $S_p$ Pedestrian Blockage Factor, $f_{pb}$	0	0	0	
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$ Major L-Shared Probability Queue-free State, $p^*_{0,j}$			929 725 1.000 725 1.000	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$		465 542 1.000 542 0.972		
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Shared L/U Capacity, $c_{SH}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Major L, Minor T Adjusted Impedance Factor, $p''$ Major L, Minor T Impedance Factor, $p'$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$		1428 125 1.000  1.000 125		

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES										
Approach	WestBound							EastBound		
Movement	7	8	9		10	11	12			
Lane Configuration	L		R							
Shared Flow Rate, $v_y$	4		15							
Movement Capacity, $c_{m,x}$	125		542							
Shared Capacity, $c_{SH}$										

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate	0	4					15			
Movement Capacity	725	125					542			
Lane Configuration	L	L					R			
Shared Capacity										
Control Delay	10.0	34.9					11.8			

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration	L	L					R			
Flow Rate	0	4					15			
Lane Capacity	725	125					542			
$v/c$	0.00	0.03					0.03			
95% Queue Length	0.0	0.1					0.1			
Control Delay	10.0	34.9					11.8			



LOS	A	D	B
Approach Delay	0.0		17.0
Approach LOS			C
Intersection Delay			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 12:54:04 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_PM\_NO BUILD\_Radio-US 41-x. xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Radio Ave / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Radio Ave  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound			SouthBound			
Approach	1U	2	3	4U	5	6	
Movement	U	L	T	R	L	T	R
Volume		822	3	0	3	1097	
Peak Hour Factor, PHF				0.92			
Hourly Flow Rate, HFR		893	3		3	1192	
Percent Heavy Vehicles					0	3	
Number of Lanes	0	0	2	0	0	1	2
Lane Configuration			T	TR		L	T
Median Type					Undivided		
Median Storage							
RT channelized?							
Left-Turn Lane Storage							
Upstream Signal?					Not Present		

Minor Street:	WestBound			EastBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume	1		4			
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR	1		4			
Percent Heavy Vehicles	3		3			
Number of Lanes	1	0	1	0	0	0
Lane Configuration	L		R			
RT channelized?			No			
Flared Approach   Storage						
Percent Grade		0				

Pedestrian Volumes and Adjustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	NB	SB	WestBound	EastBound
Movement	1U	4U	7	10
Lane Configuration	1	4	8	11
	U	L	R	T
Flow Rate		3	1	4
Lane Capacity		746	112	555
v/c		0.00	0.01	0.01
95% Queue Length		0.0	0.0	0.0
Control Delay		9.8	37.6	11.5
LOS		A	E	B
Approach Delay		0.0	16.7	
Approach LOS			C	
Intersection Delay				

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound			SouthBound		
Approach	1U	2	3	4U	5	6
Priority	U	L	T	U	L	T
Movement	U	L	T	R	L	T

Minor Street:

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:								
Approach Movement	1U	NorthBound			4U	SouthBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Volume, V_x		822			0	1097		
Flow Rate, v_x		893				1192		

Minor Street:								
Approach Movement		WestBound				EastBound		
		7	8	9		10	11	12
		L	T	R		L	T	R
Volume, V_x		1				4		
Flow Rate, v_x		1				4		

Step 3: CONFLICTING FLOW RATES

Major Street:								
Approach Movement	1U	NorthBound			4U	SouthBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Flow Rate, v_x		893			3	1192		
Conflicting Flow, v_c, x						897		

Minor Street:								
Approach Movement		WestBound				EastBound		
		7	8	9		10	11	12
		L	T	R		L	T	R
Flow Rate, v_x		1				4		
Conflicting Flow, v_c, x		1498				448		

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS										
Approach Movement	NB	SB			WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_c, base										
Single Stage				4.1	7.5		6.9			
Stage I										
Stage II										
t_c, HV				2.0	2.0		2.0			
P_HV				0.03	0.03		0.03			
t_c, G					0.2		0.1			
G					0		0			
t_3, LT				0.0	0.7		0.0			
t_c										
Single Stage				4.16	6.86		6.96			
Stage I										
Stage II										

FOLLOW-UP HEADWAYS										
Approach Movement	NB	SB			WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_f, base				2.2	3.5		3.3			
t_f, HV				1.0	1.0		1.0			
P_HV				0.03	0.03		0.03			
t_f				2.23	3.53		3.33			

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach Movement	NB	SB			WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
v_c, x				897	1498		448			
t_c, x				4.16	6.86		6.96			
t_f, x				2.23	3.53		3.33			
c_p, x				746	112		555			

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$			897	
Potential Capacity, $c_{p,x}$			746	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Movement Capacity, $c_{m,x}$			746	
Probability of Queue-free State, $p_{0,j}$			0.996	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		448		
Potential Capacity, $c_{p,x}$		555		
Pedestrian Impedance Factor, $p_{p,x}$		1.000		
Movement Capacity, $c_{m,x}$		555		
Probability of Queue-free State, $p_{0,j}$		0.992		
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$		1498		
Potential Capacity, $c_{p,x}$		112		
Pedestrian Impedance Factor, $p_{p,x}$		1.000		
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$		0.996		
Movement Capacity, $c_{m,x}$		112		

#### Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES										
Approach	WestBound				EastBound					
Movement	7	8	9	10	11	12				
Lane Configuration	L		R							
Shared Flow Rate, $v_y$	1		4							
Movement Capacity, $c_{m,x}$	112		555							
Shared Capacity, $c_{SH}$										

#### Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration										
Flow Rate	3	1					4			
Movement Capacity	746	112					555			
Lane Configuration	L	L					R			
Shared Capacity										
Control Delay	9.8	37.6					11.5			

#### Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration										
Flow Rate	3	1					4			
Lane Capacity	746	112					555			
$v/c$	0.00	0.01					0.01			
95% Queue Length	0.0	0.0					0.0			
Control Delay	9.8	37.6					11.5			

LOS	A	E	B
Approach Delay	0.0		16.7
Approach LOS			C
Intersection Delay			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 12:57:36 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_AM\_NO BUILD\_Campbell - Walnut-x. xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Campbell Rd / Walnut Rd  
 Major Street Direction: East-West  
 East/West Street Name: Campbell Road  
 North/South Street Name: Walnut Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	1U U	EastBound 1 L 2 T	3 R	4U U	WestBound 4 L 5 T	6 R
Volume		4	2		1	12
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR		4	2		1	13
Percent Heavy Vehicles					3	
Number of Lanes	0	0	1	0	1	1
Lane Configuration			T		L	T
Median Type	Undivided					
Median Storage						
RT channelized?	No					
Left-Turn Lane Storage						
Upstream Signal?	Not Present					

Minor Street: Approach Movement	NorthBound 7 L 8 T	9 R	SouthBound 10 L 11 T	12 R
Volume	13	2		
Peak Hour Factor, PHF			0.92	
Hourly Flow Rate, HFR	14	2		
Percent Heavy Vehicles	3	3		
Number of Lanes	1	0	1	0
Lane Configuration	L		R	
RT channelized?	No			
Flared Approach   Storage				
Percent Grade	0			

Pedestrian Volumes and Adjustments

Approach Movement	EB 13	WB 14	NB 15	SB 16
Flow (ped/hr)	0	0	0	
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach Movement	1U U	EB 1	4U U	WB 4 L	7 L	NorthBound 8	9 R	10 L	SouthBound 11 T	12 R
Lane Configuration				L	L		R			
Flow Rate				1	14		2			
Lane Capacity				1608	994		1076			
v/c				0.00	0.01		0.00			
95% Queue Length				0.0	0.0		0.0			
Control Delay				7.2	8.7		8.4			
LOS				A	A		A			
Approach Delay				0.6		8.6				
Approach LOS						A				
Intersection Delay	4.0									

Step 1: MOVEMENT PRIORITIES

Major Street: Approach Priority Movement	1U U	EastBound 1 L 2 T	3 R	4U U	WestBound 4 L 5 T	6 R
Minor Street:						

Approach Priority Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement	1U U	EastBound		3 R		4U U	WestBound		6 R
		1 L	2 T				4 L	5 T	
Volume, V_x		4		2		1		12	
Flow Rate, v_x		4		2		1		13	

Minor Street:							
Approach Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R
Volume, V_x	13		2		1		12
Flow Rate, v_x	14		2		1		13

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement	1U U	EastBound		3 R		4U U	WestBound		6 R
		1 L	2 T				4 L	5 T	
Flow Rate, v_x		4		2		1		13	
Conflicting Flow, v_c, x						7			

Minor Street:							
Approach Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R
Flow Rate, v_x	14		2		1		12
Conflicting Flow, v_c, x	20		4				

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS												
Approach Movement	1U U	EB L	1	4U U	WB L	4	7 L	NorthBound 8 T	9 R	10 L	SouthBound 11 T	12 R
t_c, base												
Single Stage						4.1	7.1		6.2			
Stage I												
Stage II												
t_c, HV						1.0	1.0		1.0			
P_HV						0.03	0.03		0.03			
t_c, G							0.2		0.1			
G							0		0			
t_3, LT						0.0	0.7		0.0			
t_c												
Single Stage						4.13	6.43		6.23			
Stage I												
Stage II												

FOLLOW-UP HEADWAYS												
Approach Movement	1U U	EB L	1	4U U	WB L	4	7 L	NorthBound 8 T	9 R	10 L	SouthBound 11 T	12 R
t_f, base												
t_f, HV						2.2	3.5		3.3			
P_HV						0.9	0.9		0.9			
t_f						0.03	0.03		0.03			
c_p, x						2.23	3.53		3.33			

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT												
Approach Movement	1U U	EB L	1	4U U	WB L	4	7 L	NorthBound 8 T	9 R	10 L	SouthBound 11 T	12 R
v_c, x						7	20		4			
t_c, x						4.13	6.43		6.23			
t_f, x						2.23	3.53		3.33			
c_p, x						1608	995		1076			

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	FR	WR	NR	SB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$ Lane Width, $w$ Walking Speed, $S_p$ Pedestrian Blockage Factor, $f_{pb}$	0	0	0	
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$ Major L-Shared Probability Queue-free State, $p^*_{0,j}$			7 1608 1.000 1608 0.999	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$		4 1076 1.000 1076 0.998		
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Shared L/U Capacity, $c_{SH}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Major L, Minor T Adjusted Impedance Factor, $p''$ Major L, Minor T Impedance Factor, $p'$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$		20 995 1.000   0.999 994		

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES										
Approach	NorthBound							SouthBound		
Movement	7	8	9		10	11	12			
Lane Configuration	L		R							
Shared Flow Rate, $v_y$	14		2							
Movement Capacity, $c_{m,x}$	994		1076							
Shared Capacity, $c_{SH}$										

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate	1				14		2			
Movement Capacity	1608				994		1076			
Lane Configuration	L				L		R			
Shared Capacity										
Control Delay	7.2				8.7		8.4			

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration				L	L		R			
Flow Rate	1				14		2			
Lane Capacity	1608				994		1076			
$v/c$	0.00				0.01		0.00			
95% Queue Length	0.0				0.0		0.0			
Control Delay	7.2				8.7		8.4			



LOS		A	A		A
Approach Delay		0.6		8.6	
Approach LOS				A	
Intersection Delay	4.0				

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 1:03:31 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_PM\_NO BUILD\_Campbell - Walnut-x. xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Campbell Rd / Walnut Rd  
 Major Street Direction: East-West  
 East/West Street Name: Campbell Road  
 North/South Street Name: Walnut Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Volume			15	15		1	5		
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR			16	16		1	5		
Percent Heavy Vehicles						3			
Number of Lanes	0	0	1	1	0	1	1	0	
Lane Configuration			T	R		L	T		
Median Type	Undivided								
Median Storage									
RT channelized?	No								
Left-Turn Lane Storage									
Upstream Signal?	Not Present								

Minor Street:	NorthBound			SouthBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume	8		0			
Peak Hour Factor, PHF						0.92
Hourly Flow Rate, HFR	9		0			
Percent Heavy Vehicles	3		3			
Number of Lanes	1	0	1	0	0	0
Lane Configuration	L		R			
RT channelized?			No			
Flared Approach   Storage						
Percent Grade		0				

Pedestrian Volumes and Adjustments

Approach	EB	WB	NB	SB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	EB	WB	NorthBound			SouthBound			
Movement	1U	4U	4	7	8	9	10	11	12
Lane Configuration			L	L		R			
Flow Rate			1	9		0			
Lane Capacity			1573	989		1060			
v/c			0.00	0.01		0.00			
95% Queue Length			0.0	0.0		0.0			
Control Delay			7.3	8.7		8.4			
LOS			A	A		A			
Approach Delay			1.2		8.7				
Approach LOS					A				
Intersection Delay	1.7								

Step 1: MOVEMENT PRIORITIES

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Minor Street:									

Approach Priority Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement	1U U	EastBound				WestBound			
		1 L	2 T	3 R		4U U	4 L	5 T	6 R
Volume, V_x		15	15			1	5		
Flow Rate, v_x		16	16			1	5		

Minor Street:									
Approach Movement	NorthBound				SouthBound				
		7 L	8 T	9 R		10 L	11 T	12 R	
Volume, V_x		8	0						
Flow Rate, v_x		9	0						

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement	1U U	EastBound				WestBound			
		1 L	2 T	3 R		4U U	4 L	5 T	6 R
Flow Rate, v_x		16	16			1	5		
Conflicting Flow, v_c, x						33			

Minor Street:									
Approach Movement	NorthBound				SouthBound				
		7 L	8 T	9 R		10 L	11 T	12 R	
Flow Rate, v_x		9	0						
Conflicting Flow, v_c, x		24	16						

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS												
Approach Movement	1U U	EB L	1	4U U	WB L	4	7 L	NorthBound 8 T	9 R	10 L	SouthBound 11 T	12 R
t_c, base												
Single Stage						4.1	7.1		6.2			
Stage I												
Stage II												
t_c, HV						1.0	1.0		1.0			
P_HV						0.03	0.03		0.03			
t_c, G							0.2		0.1			
G							0		0			
t_3, LT						0.0	0.7		0.0			
t_c												
Single Stage						4.13	6.43		6.23			
Stage I												
Stage II												

FOLLOW-UP HEADWAYS												
Approach Movement	1U U	EB L	1	4U U	WB L	4	7 L	NorthBound 8 T	9 R	10 L	SouthBound 11 T	12 R
t_f, base												
t_f, HV						2.2	3.5		3.3			
P_HV						0.9	0.9		0.9			
t_f						0.03	0.03		0.03			
c_p, x						2.23	3.53		3.33			

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT												
Approach Movement	1U U	EB L	1	4U U	WB L	4	7 L	NorthBound 8 T	9 R	10 L	SouthBound 11 T	12 R
v_c, x						33	24		16			
t_c, x						4.13	6.43		6.23			
t_f, x						2.23	3.53		3.33			
c_p, x						1573	990		1060			

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	FR	WR	NR	SB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$ Lane Width, $w$ Walking Speed, $S_p$ Pedestrian Blockage Factor, $f_{pb}$	0	0	0	
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$ Major L-Shared Probability Queue-free State, $p^*_{0,j}$			33 1573 1.000 1573 0.999	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$		16 1060 1.000 1060 1.000		
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Shared L/U Capacity, $c_{SH}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Major L, Minor T Adjusted Impedance Factor, $p''$ Major L, Minor T Impedance Factor, $p'$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$		24 990 1.000   0.999 989		

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES										
Approach	NorthBound							SouthBound		
Movement	7	8	9		10	11	12			
Lane Configuration	L		R							
Shared Flow Rate, $v_y$	9		0							
Movement Capacity, $c_{m,x}$	989		1060							
Shared Capacity, $c_{SH}$										

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate	1				9		0			
Movement Capacity	1573				989		1060			
Lane Configuration	L				L		R			
Shared Capacity										
Control Delay	7.3				8.7		8.4			

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration				L	L		R			
Flow Rate	1				9		0			
Lane Capacity	1573				989		1060			
$v/c$	0.00				0.01		0.00			
95% Queue Length	0.0				0.0		0.0			
Control Delay	7.3				8.7		8.4			

LOS		A	A		A
Approach Delay		1.2		8.7	
Approach LOS				A	
Intersection Delay	1.7				

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 1:05:21 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis  
 File Name: 2020\_AM\_NO BUILD\_Campbell-OSR-x.twt  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Campbell Rd / OSR  
 Major Street Direction: East-West  
 East/West Street Name: Campbell Road  
 North/South Street Name: Old State Road  
 Analysis Time Period (hrs): 0.25

Vehi cle Vol umes and Adj ustments

Major Street:	EastBound						WestBound					
Approach	1U		2		3		4U		5		6	
Movement	U	L	T	R	L	T	U	L	T	R	L	R
Volume		37	25						11	17		
Peak Hour Factor, PHF							0.92					
Hourly Flow Rtae, HFR		40	27						12	18		
Percent Heavy Vehicles		3										
Number of Lanes	0	1	1	0			0	0	1	1		
Lane Configuration		L	T						T	R		
Median Type							Undivided					
Median Storage												
RT channelized?												No
Left-Turn Lane Storage												
Upstream Signal?							Not Present					

Minor Street:	NorthBound						SouthBound					
Approach	7		8		9		10		11		12	
Movement	L	T	R	L	T	R	L	T	R	L	R	
Volume									14		64	
Peak Hour Factor, PHF							0.92					
Hourly Flow Rtae, HFR									15		70	
Percent Heavy Vehicles									3		3	
Number of Lanes	0	0	0						1	0	1	
Lane Configuration									L		R	
RT channelized?												No
Flared Approach   Storage										0		
Percent Grade												

Pedestri an Vol umes and Adj ustments

Approach	EB			WB			NB			SB		
Movement	13			14			15			16		
Flow (ped/hr)	0			0						0		
Lane Width (ft)												
Walking Speed (ft/sec)												
Pedestrian Blockage Factor, f_pb												

Del ay, Queue Length, and Level of Service

Approach	EB			WB			NorthBound			SouthBound		
Movement	1U	2	3	4U	4	7	8	9	10	11	12	
Lane Configuration	L	T	R	L	T	R	L	T	L	T	R	
Flow Rate	40						15			70		
Lane Capacity	1576						851			1066		
v/c	0.03						0.02			0.07		
95% Queue Length	0.1						0.1			0.2		
Control Delay	7.3						9.3			8.6		
LOS	A						A			A		
Approach Delay	4.4									8.7		
Approach LOS										A		
Intersction Delay	5.7											

Step 1: MOVEMENT PRI OR TI ES

Major Street:	EastBound						WestBound					
Approach	1U		2		3		4U		5		6	
Pri ority	U	L	T	R	L	T	U	L	T	R	L	R
Minor Street:												

Approach Priority Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	EastBound		3 R		4U U	WestBound		6 R
		1 L	2 T				4 L	5 T	
Volume, V_x		37	25				11	17	
Flow Rate, v_x		40	27				12	18	

Minor Street: Approach Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R
Volume, V_x					14		64
Flow Rate, v_x					15		70

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	EastBound		3 R		4U U	WestBound		6 R
		1 L	2 T				4 L	5 T	
Flow Rate, v_x		40	27				12	18	
Conflicting Flow, v_c, x		30							

Minor Street: Approach Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R
Flow Rate, v_x					15		70
Conflicting Flow, v_c, x					120		12

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	EB	WB			NorthBound			SouthBound		
	1U U	1 L	4U U	4 L	7 L	8 T	9 R	10 L	11 T	12 R
t_c, base										
Single Stage		4.1						7.1		6.2
Stage I										
Stage II										
t_c, HV		1.0						1.0		1.0
P_HV		0.03						0.03		0.03
t_c, G								0.2		0.1
G								0		0
t_3, LT		0.0						0.7		0.0
t_c										
Single Stage		4.13						6.43		6.23
Stage I										
Stage II										

FOLLOW-UP HEADWAYS Approach Movement	EB	WB			NorthBound			SouthBound		
	1U U	1 L	4U U	4 L	7 L	8 T	9 R	10 L	11 T	12 R
t_f, base		2.2						3.5		3.3
t_f, HV		0.9						0.9		0.9
P_HV		0.03						0.03		0.03
t_f		2.23						3.53		3.33

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	EB	WB			NorthBound			SouthBound		
	1U U	1 L	4U U	4 L	7 L	8 T	9 R	10 L	11 T	12 R
v_c, x		30						120		12
t_c, x		4.13						6.43		6.23
t_f, x		2.23						3.53		3.33
c_p, x		1576						874		1066

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	FR	WR	NR	SB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0		0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		30		
Potential Capacity, $c_{p,x}$		1576		
Pedestrian Impedance Factor, $p_{p,x}$		1.000		
Movement Capacity, $c_{m,x}$		1576		
Probability of Queue-free State, $p_{0,j}$		0.974		
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$			12	
Potential Capacity, $c_{p,x}$			1066	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Movement Capacity, $c_{m,x}$			1066	
Probability of Queue-free State, $p_{0,j}$			0.935	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$			120	
Potential Capacity, $c_{p,x}$			874	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$			0.974	
Movement Capacity, $c_{m,x}$			851	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES											
Approach	NorthBound						SouthBound				
Movement	7	8	9				10	11	12		
Lane Configuration							L			R	
Shared Flow Rate, $v_y$							15			70	
Movement Capacity, $c_{m,x}$							851			1066	
Shared Capacity, $c_{SH}$											

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS												
Approach	EB			WB			NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12		
Flow Rate		40						15		70		
Movement Capacity		1576						851		1066		
Lane Configuration		L						L		R		
Shared Capacity												
Control Delay		7.3						9.3		8.6		

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	EB			WB			NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12		
Lane Configuration		L						L		R		
Flow Rate		40						15		70		
Lane Capacity		1576						851		1066		
$v/c$		0.03						0.02		0.07		
95% Queue Length		0.1						0.1		0.2		
Control Delay		7.3						9.3		8.6		



LOS	A	A		A
Approach Delay	4.4		8.7	
Approach LOS			A	
Intersection Delay	5.7			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 1:10:08 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_PM\_NO BUILD\_Campbell-OSR-x. xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Campbell Rd / OSR  
 Major Street Direction: East-West  
 East/West Street Name: Campbell Road  
 North/South Street Name: Old State Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Volume		44	17				9	7	
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		48	18				10	8	
Percent Heavy Vehicles		3							
Number of Lanes	0	1	1	0	0	0	1	1	
Lane Configuration		L	T				T	R	
Median Type					Undivided				
Median Storage									
RT channelized?								No	
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street:	NorthBound			SouthBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume				26		33
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR				28		36
Percent Heavy Vehicles				3		3
Number of Lanes	0	0	0	1	0	1
Lane Configuration				L		R
RT channelized?						No
Flared Approach   Storage					0	
Percent Grade						

Pedestrian Volumes and Adjustments

Approach	EB	WB	NB	SB
Movement	13	14	15	16
Flow (ped/hr)	0	0		0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	EB	NorthBound			SouthBound			
Movement	1U	4U	7	8	9	10	11	12
Lane Configuration	L	L	T	T	T	L	T	R
Flow Rate	48					28		36
Lane Capacity	1593					843		1069
v/c	0.03					0.03		0.03
95% Queue Length	0.1					0.1		0.1
Control Delay	7.3					9.4		8.5
LOS	A					A		A
Approach Delay	5.3						8.9	
Approach LOS							A	
Intersection Delay	6.2							

Step 1: MOVEMENT PRIORITIES

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Priority	U	L	T	R	U	L	T	R	

Minor Street:

Approach Priority Movement	NorthBound				SouthBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:								
Approach Movement	1U	EastBound			4U	WestBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Volume, V_x		44	17			9	7	
Flow Rate, v_x		48	18			10	8	

Minor Street:								
Approach Movement	NorthBound				SouthBound			
	7	8	9		10	11	12	
	L	T	R		L	T	R	
Volume, V_x					26		33	
Flow Rate, v_x					28		36	

Step 3: CONFLICTING FLOW RATES

Major Street:								
Approach Movement	1U	EastBound			4U	WestBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Flow Rate, v_x		48	18			10	8	
Conflicting Flow, v_c, x		17						

Minor Street:								
Approach Movement	NorthBound				SouthBound			
	7	8	9		10	11	12	
	L	T	R		L	T	R	
Flow Rate, v_x					28		36	
Conflicting Flow, v_c, x					124		10	

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS														
Approach Movement	1U	EB	1	4U	WB	4	7	NorthBound	8	9	SouthBound	10	11	12
	U		L	U		L	L		T	R		L	T	R
t_c, base			4.1									7.1		6.2
Single Stage														
Stage I														
Stage II														
t_c, HV			1.0									1.0		1.0
P_HV			0.03									0.03		0.03
t_c, G												0.2		0.1
G												0		0
t_3, LT			0.0									0.7		0.0
t_c														
Single Stage			4.13									6.43		6.23
Stage I														
Stage II														

FOLLOW-UP HEADWAYS														
Approach Movement	1U	EB	1	4U	WB	4	7	NorthBound	8	9	SouthBound	10	11	12
	U		L	U		L	L		T	R		L	T	R
t_f, base			2.2									3.5		3.3
t_f, HV			0.9									0.9		0.9
P_HV			0.03									0.03		0.03
t_f			2.23									3.53		3.33

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT														
Approach Movement	1U	EB	1	4U	WB	4	7	NorthBound	8	9	SouthBound	10	11	12
	U		L	U		L	L		T	R		L	T	R
v_c, x			17									124		10
t_c, x			4.13									6.43		6.23
t_f, x			2.23									3.53		3.33
c_p, x			1593									869		1069

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	FR	WR	NR	SB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0		0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		17		
Potential Capacity, $c_{p,x}$		1593		
Pedestrian Impedance Factor, $p_{p,x}$		1.000		
Movement Capacity, $c_{m,x}$		1593		
Probability of Queue-free State, $p_{0,j}$		0.970		
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$			10	
Potential Capacity, $c_{p,x}$			1069	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Movement Capacity, $c_{m,x}$			1069	
Probability of Queue-free State, $p_{0,j}$			0.966	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$			124	
Potential Capacity, $c_{p,x}$			869	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$			0.970	
Movement Capacity, $c_{m,x}$			843	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	NorthBound			SouthBound		
Movement	7	8	9	10	11	12
Lane Configuration				L		R
Shared Flow Rate, $v_y$				28		36
Movement Capacity, $c_{m,x}$				843		1069
Shared Capacity, $c_{SH}$						

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		48						28		36
Movement Capacity		1593						843		1069
Lane Configuration		L						L		R
Shared Capacity										
Control Delay		7.3						9.4		8.5

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration		L						L		R
Flow Rate		48						28		36
Lane Capacity		1593						843		1069
$v/c$		0.03						0.03		0.03
95% Queue Length		0.1						0.1		0.1
Control Delay		7.3						9.4		8.5

LOS	A	A		A
Approach Delay	5.3		8.9	
Approach LOS			A	
Intersection Delay	6.2			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 1:12:05 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_AM\_NO BUILD\_OSR-41-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Campbell-Old St Rd/ US 41  
 Major Street Direction: North-South  
 East/West Street Name: Campbell/OSR  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R
Volume	0	0	981	11		0	3	920	38
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		0	1066	12			3	1000	41
Percent Heavy Vehicles	13	13				13	13		
Number of Lanes	0	1	2	0		0	1	2	1
Lane Configuration		L	T	TR			L	T	R
Median Type					Undivided				
Median Storage									
RT channelized?									No
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street: Approach Movement		WestBound 7 L	8 T	9 R		EastBound 10 L	11 T	12 R
Volume		19	15	5		69	25	6
Peak Hour Factor, PHF					0.92			
Hourly Flow Rate, HFR		21	16	5		75	27	7
Percent Heavy Vehicles		3	3	3		3	3	3
Number of Lanes		0	1	0		0	1	0
Lane Configuration			LTR				LTR	
RT channelized?								
Flared Approach   Storage		No				No		
Percent Grade			0				0	

Pedestrian Volumes and Adjustments

Approach Movement		NB 13	SB 14	WB 15	EB 16
Flow (ped/hr)		0	0	0	0
Lane Width (ft)					
Walking Speed (ft/sec)					
Pedestrian Blockage Factor, f_pb					

Delay, Queue Length, and Level of Service

Approach Movement	1U U	NB 1 L	4U L	7 L	8 LTR	9 L	10 L	EastBound 11 LTR	12 R
Lane Configuration									
Flow Rate		0	3		42			109	
Lane Capacity		602	582		50			58	
v/c		0.00	0.01		0.85			1.88	
95% Queue Length		0.0	0.0		3.6			10.3	
Control Delay		11.0	11.2		213.9			568.0	
LOS		B	B		F			F	
Approach Delay		0.0	0.0		213.9			568.0	
Approach LOS					F			F	
Intersction Delay		31.2							

Step 1: MOVEMENT PRIORITIES

Major Street: Approach Priority Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R
Minor Street:									

Approach Priority Movement	WestBound				EastBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x	0	0	981	11	0	3	920	38	
Flow Rate, v_x		0	1066	12		3	1000	41	

Minor Street: Approach Movement	WestBound				EastBound		
	7 L	8 T	9 R		10 L	11 T	12 R
Volume, V_x	19	15	5	69	25	6	
Flow Rate, v_x	21	16	5	75	27	7	

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		0	1066	12		3	1000	41	
Conflicting Flow, v_c, x		1041				1078			

Minor Street: Approach Movement	WestBound				EastBound		
	7 L	8 T	9 R		10 L	11 T	12 R
Flow Rate, v_x	21	16	5	75	27	7	
Conflicting Flow, v_c, x	1592	2120	539	1548	2085	500	

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	NB 1U U	1 L	SB 4U U	4 L	7 L	WestBound		10 L	EastBound	
						8 T	9 R		11 T	12 R
t_c, base										
Single Stage		4.1		4.1	7.5	6.5	6.9	7.5	6.5	6.9
Stage I										
Stage II										
t_c, HV		2.0		2.0	2.0	2.0	2.0	2.0	2.0	2.0
P_HV		0.13		0.13	0.03	0.03	0.03	0.03	0.03	0.03
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1
G					0	0	0	0	0	0
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0
t_c										
Single Stage		4.36		4.36	7.56	6.56	6.96	7.56	6.56	6.96
Stage I										
Stage II										

FOLLOW-UP HEADWAYS Approach Movement	NB 1U U	1 L	SB 4U U	4 L	7 L	WestBound		10 L	EastBound	
						8 T	9 R		11 T	12 R
t_f, base										
t_f, HV		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3
P_HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0
t_f		0.13		0.13	0.03	0.03	0.03	0.03	0.03	0.03
		2.33		2.33	3.53	4.03	3.33	3.53	4.03	3.33

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	NB 1U U	1 L	SB 4U U	4 L	7 L	WestBound		10 L	EastBound	
						8 T	9 R		11 T	12 R
v_c, x		1041		1078	1592	2120	539	1548	2085	500
t_c, x		4.36		4.36	7.56	6.56	6.96	7.56	6.56	6.96
t_f, x		2.33		2.33	3.53	4.03	3.33	3.53	4.03	3.33
c_p, x		602		582	71	49	484	77	52	514

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	NR	SR	WR	EB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		1041	1078	
Potential Capacity, $c_{p,x}$		602	582	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		602	582	
Probability of Queue-free State, $p_{0,j}$		1.000	0.994	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		539	500	
Potential Capacity, $c_{p,x}$		484	514	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		484	514	
Probability of Queue-free State, $p_{0,j}$		0.989	0.987	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$		2120	2085	
Potential Capacity, $c_{p,x}$		49	52	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Capacity Adjustment Factor, $f_x$		0.994	0.994	
Movement Capacity, $c_{m,x}$		49	51	
Probability of Queue-free State, $p_{0,j}$		0.666	0.471	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$		1592	1548	
Potential Capacity, $c_{p,x}$		71	77	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$		0.469	0.662	
Major L, Minor T Impedance Factor, $p'$		0.580	0.738	
Capacity Adjustment Factor, $f_x$		0.573	0.730	
Movement Capacity, $c_{m,x}$		41	56	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	WestBound			EastBound		
Movement	7	8	9	10	11	12
Lane Configuration		LTR			LTR	
Shared Flow Rate, $v_y$		42			109	
Movement Capacity, $c_{m,x}$	41	49	484	56	51	514
Shared Capacity, $c_{SH}$		50			58	

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		0		3	21	16	5	75	27	7
Movement Capacity		602		582	41	49	484	56	51	514
Lane Configuration		L		L		LTR			LTR	
Shared Capacity						50			58	
Control Delay		11.0		11.2		213.9			568.0	

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration		L		L		LTR			LTR	
Flow Rate		0		3		42			109	
Lane Capacity		602		582		50			58	
$v/c$		0.00		0.01		0.85			1.88	
95% Queue Length		0.0		0.0		3.6			10.3	
Control Delay		11.0		11.2		213.9			568.0	



LOS	B	B	F	F
Approach Delay	0.0	0.0	213.9	568.0
Approach LOS			F	F
Intersection Delay	31.2			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 3:54:41 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_AM\_NO BUILD\_OSR-41-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Hillisdale Rd / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Campbell/OSR - US 41  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound				SouthBound			
Approach Movement	1U	2	3	4U	4	5	6	
	U	L	T	R	U	L	T	R
Volume	0	0	981	11	0	3	920	38
Peak Hour Factor, PHF					0.92			
Hourly Flow Rate, HFR		0	1066	12		3	1000	41
Percent Heavy Vehicles	13	13			13	13		
Number of Lanes	0	1	2	0	0	1	2	1
Lane Configuration		L	T	TR		L	T	R
Median Type	Undivided							
Median Storage								
RT channelized?								
Left-Turn Lane Storage	No							
Upstream Signal?	Not Present							

Minor Street:	WestBound			EastBound		
Approach Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	19	15	5	69	25	6
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR	21	16	5	75	27	7
Percent Heavy Vehicles	3	3	3	3	3	3
Number of Lanes	0	1	0	1	1	0
Lane Configuration		LTR		L		TR
RT channelized?						
Flared Approach   Storage	No			No		
Percent Grade		0			0	

Pedestrian Volumes and Adjustments

Approach Movement	NB	SB	WB	EB
	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach Movement	NB	4U	7	8	9	10	11	12
	1	4U	7	8	9	10	11	12
Lane Configuration	L	L		LTR		L		TR
Flow Rate	0	3		42		75		34
Lane Capacity	602	582		50		56		62
v/c	0.00	0.01		0.85		1.34		0.54
95% Queue Length	0.0	0.0		3.6		6.6		2.2
Control Delay	11.0	11.2		213.9		356.0		116.9
LOS	B	B		F		F		F
Approach Delay	0.0	0.0		213.9			281.9	
Approach LOS				F			F	
Intersection Delay	17.5							

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound				SouthBound			
Approach Priority Movement	1U	2	3	4U	4	5	6	
	U	L	T	R	U	L	T	R
Minor Street:								

Approach Priority Movement	WestBound				EastBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x	0	0	981	11	0	3	920	38	
Flow Rate, v_x		0	1066	12		3	1000	41	

Minor Street: Approach Movement	WestBound				EastBound		
	7 L	8 T	9 R		10 L	11 T	12 R
Volume, V_x	19	15	5	69	25	6	
Flow Rate, v_x	21	16	5	75	27	7	

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x	0	1066	12	3	1000	41			
Conflicting Flow, v_c, x		1041		1078					

Minor Street: Approach Movement	WestBound				EastBound		
	7 L	8 T	9 R		10 L	11 T	12 R
Flow Rate, v_x	21	16	5	75	27	7	
Conflicting Flow, v_c, x	1592	2120	539	1548	2085	500	

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	NB 1U U	1 L	SB 4U U	4 L	7 L	WestBound		9 R	EastBound		12 R
						8 T	10 L		11 T		
t_c, base		4.1	4.1	7.5	6.5	6.9	7.5	6.5	6.9		
Single Stage											
Stage I											
Stage II											
t_c, HV		2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0		
P_HV		0.13	0.13	0.03	0.03	0.03	0.03	0.03	0.03		
t_c, G				0.2	0.2	0.1	0.2	0.2	0.1		
G				0	0	0	0	0	0		
t_3, LT		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0		
t_c											
Single Stage		4.36	4.36	7.56	6.56	6.96	7.56	6.56	6.96		
Stage I											
Stage II											

FOLLOW-UP HEADWAYS Approach Movement	NB 1U U	1 L	SB 4U U	4 L	7 L	WestBound		9 R	EastBound		12 R
						8 T	10 L		11 T		
t_f, base		2.2	2.2	3.5	4.0	3.3	3.5	4.0	3.3		
t_f, HV		1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0		
P_HV		0.13	0.13	0.03	0.03	0.03	0.03	0.03	0.03		
t_f		2.33	2.33	3.53	4.03	3.33	3.53	4.03	3.33		

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	NB 1U U	1 L	SB 4U U	4 L	7 L	WestBound		9 R	EastBound		12 R
						8 T	10 L		11 T		
v_c, x		1041	1078	1592	2120	539	1548	2085	500		
t_c, x		4.36	4.36	7.56	6.56	6.96	7.56	6.56	6.96		
t_f, x		2.33	2.33	3.53	4.03	3.33	3.53	4.03	3.33		
c_p, x		602	582	71	49	484	77	52	514		

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	NR	SR	WR	EB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		1041	1078	
Potential Capacity, $c_{p,x}$		602	582	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		602	582	
Probability of Queue-free State, $p_{0,j}$		1.000	0.994	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		539	500	
Potential Capacity, $c_{p,x}$		484	514	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		484	514	
Probability of Queue-free State, $p_{0,j}$		0.989	0.987	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$		2120	2085	
Potential Capacity, $c_{p,x}$		49	52	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Capacity Adjustment Factor, $f_x$		0.994	0.994	
Movement Capacity, $c_{m,x}$		49	51	
Probability of Queue-free State, $p_{0,j}$		0.666	0.471	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$		1592	1548	
Potential Capacity, $c_{p,x}$		71	77	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$		0.469	0.662	
Major L, Minor T Impedance Factor, $p'$		0.580	0.738	
Capacity Adjustment Factor, $f_x$		0.573	0.730	
Movement Capacity, $c_{m,x}$		41	56	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	WestBound			EastBound		
Movement	7	8	9	10	11	12
Lane Configuration		LTR		L		TR
Shared Flow Rate, $v_y$		42		75		34
Movement Capacity, $c_{m,x}$	41	49	484	56	51	514
Shared Capacity, $c_{SH}$		50				62

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
Movement										
Flow Rate		0		3	21	16	5	75	27	7
Movement Capacity		602		582	41	49	484	56	51	514
Lane Configuration		L		L		LTR		L		TR
Shared Capacity						50				62
Control Delay		11.0		11.2		213.9		356.0		116.9

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
Movement										
Lane Configuration										
Flow Rate		0		3		42		75		34
Lane Capacity		602		582		50		56		62
$v/c$		0.00		0.01		0.85		1.34		0.54
95% Queue Length		0.0		0.0		3.6		6.6		2.2
Control Delay		11.0		11.2		213.9		356.0		116.9

LOS	B	B	F	F	F
Approach Delay	0.0	0.0	213.9		281.9
Approach LOS			F		F
Intersection Delay	17.5				

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 3:51:52 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_PM\_NO BUILD\_OSR-41-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Campbell-Old St Rd/ US 41  
 Major Street Direction: North-South  
 East/West Street Name: Campbell/OSR  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R
Volume	0	6	909	16		0	5	1103	125
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		7	988	17			5	1199	136
Percent Heavy Vehicles	13	13				13	13		
Number of Lanes	0	1	2	0		0	1	2	1
Lane Configuration		L	T	TR			L	T	R
Median Type					Undivided				
Median Storage									
RT channelized?									No
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street: Approach Movement	WestBound 7 L	8 T	9 R		EastBound 10 L	11 T	12 R
Volume	24	19	4		37	15	4
Peak Hour Factor, PHF				0.92			
Hourly Flow Rate, HFR	26	21	4		40	16	4
Percent Heavy Vehicles	3	3	3		3	3	3
Number of Lanes	0	1	0		0	1	0
Lane Configuration		LTR				LTR	
RT channelized?							
Flared Approach   Storage	No				No		
Percent Grade		0				0	

Pedestrian Volumes and Adjustments

Approach Movement	NB 13	SB 14	WB 15	EB 16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach Movement	1U U	NB 1 L	4U L	7 L	8 LTR	9 L	10 L	EastBound 11 LTR	12 R
Lane Configuration									
Flow Rate	7		5		51			61	
Lane Capacity	458		622		43			33	
v/c	0.01		0.01		1.20			1.83	
95% Queue Length	0.0		0.0		4.9			6.8	
Control Delay	13.0		10.8		352.3			653.4	
LOS	B		B		F			F	
Approach Delay	0.1		0.0		352.3			653.4	
Approach LOS					F			F	
Intersection Delay	23.5								

Step 1: MOVEMENT PRIORITIES

Major Street: Approach Priority Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R
Minor Street:									

Approach Priority Movement	WestBound				EastBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x	0	6	909	16		0	5	1103	125
Flow Rate, v_x		7	988	17			5	1199	136

Minor Street: Approach Movement	WestBound				EastBound		
	7 L	8 T	9 R		10 L	11 T	12 R
Volume, V_x	24	19	4		37	15	4
Flow Rate, v_x	26	21	4		40	16	4

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		7	988	17		5	1199	136	
Conflicting Flow, v_c, x		1335				1005			

Minor Street: Approach Movement	WestBound				EastBound		
	7 L	8 T	9 R		10 L	11 T	12 R
Flow Rate, v_x	26	21	4		40	16	4
Conflicting Flow, v_c, x	1628	2355	503		1727	2228	599

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	1U U	NB	4U U	SB	4	7	WestBound		10	EastBound	
		1 L					8 T	9 R		11 T	12 R
t_c, base											
Single Stage		4.1		4.1	7.5	6.5	6.9	7.5	6.5	6.9	
Stage I											
Stage II											
t_c, HV		2.0		2.0	2.0	2.0	2.0	2.0	2.0	2.0	
P_HV		0.13		0.13	0.03	0.03	0.03	0.03	0.03	0.03	
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1	
G					0	0	0	0	0	0	
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
t_c											
Single Stage		4.36		4.36	7.56	6.56	6.96	7.56	6.56	6.96	
Stage I											
Stage II											

FOLLOW-UP HEADWAYS Approach Movement	1U U	NB	4U U	SB	4	7	WestBound		10	EastBound	
		1 L					8 T	9 R		11 T	12 R
t_f, base											
t_f, HV		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3	
P_HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	
t_f		0.13		0.13	0.03	0.03	0.03	0.03	0.03	0.03	
t_f		2.33		2.33	3.53	4.03	3.33	3.53	4.03	3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	1U U	NB	4U U	SB	4	7	WestBound		10	EastBound	
		1 L					8 T	9 R		11 T	12 R
v_c, x		1335		1005	1628	2355	503	1727	2228	599	
t_c, x		4.36		4.36	7.56	6.56	6.96	7.56	6.56	6.96	
t_f, x		2.33		2.33	3.53	4.03	3.33	3.53	4.03	3.33	
c_p, x		458		622	67	35	511	56	42	442	

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	NR	SR	WR	EB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		1335	1005	
Potential Capacity, $c_{p,x}$		458	622	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		458	622	
Probability of Queue-free State, $p_{0,j}$		0.986	0.991	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		503	599	
Potential Capacity, $c_{p,x}$		511	442	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		511	442	
Probability of Queue-free State, $p_{0,j}$		0.991	0.990	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$		2355	2228	
Potential Capacity, $c_{p,x}$		35	42	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Capacity Adjustment Factor, $f_x$		0.977	0.977	
Movement Capacity, $c_{m,x}$		34	41	
Probability of Queue-free State, $p_{0,j}$		0.391	0.602	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$		1628	1727	
Potential Capacity, $c_{p,x}$		67	56	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$		0.588	0.382	
Major L, Minor T Impedance Factor, $p'$		0.678	0.506	
Capacity Adjustment Factor, $f_x$		0.672	0.502	
Movement Capacity, $c_{m,x}$		45	28	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	WestBound			EastBound		
Movement	7	8	9	10	11	12
Lane Configuration		LTR			LTR	
Shared Flow Rate, $v_y$		51			61	
Movement Capacity, $c_{m,x}$	45	34	511	28	41	442
Shared Capacity, $c_{SH}$		43			33	

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
Movement										
Flow Rate		7		5	26	21	4	40	16	4
Movement Capacity		458		622	45	34	511	28	41	442
Lane Configuration		L		L		LTR			LTR	
Shared Capacity						43			33	
Control Delay		13.0		10.8		352.3			653.4	

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
Movement										
Lane Configuration										
Flow Rate		7		5		51			61	
Lane Capacity		458		622		43			33	
$v/c$		0.01		0.01		1.20			1.83	
95% Queue Length		0.0		0.0		4.9			6.8	
Control Delay		13.0		10.8		352.3			653.4	



LOS	B	B	F	F
Approach Delay	0.1	0.0	352.3	653.4
Approach LOS			F	F
Intersection Delay	23.5			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 4:12:05 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_PM\_NO BUILD\_OSR-41-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build w/EB\_LTL PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Campbell-Old St Rd/ US 41  
 Major Street Direction: North-South  
 East/West Street Name: Campbell/OSR  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R
Volume	0	6	909	16		0	5	1103	125
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		7	988	17			5	1199	136
Percent Heavy Vehicles	13	13				13	13		
Number of Lanes	0	1	2	0		0	1	2	1
Lane Configuration		L	T	TR			L	T	R
Median Type					Undivided				
Median Storage									
RT channelized?									No
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street: Approach Movement		WestBound 7 L	8 T	9 R		EastBound 10 L	11 T	12 R
Volume		24	19	4		37	15	4
Peak Hour Factor, PHF					0.92			
Hourly Flow Rate, HFR		26	21	4		40	16	4
Percent Heavy Vehicles		3	3	3		3	3	3
Number of Lanes		0	1	0		1	1	0
Lane Configuration			LTR			L		TR
RT channelized?								
Flared Approach   Storage		No				No		
Percent Grade			0				0	

Pedestrian Volumes and Adjustments

Approach Movement		NB 13	SB 14	WB 15	EB 16
Flow (ped/hr)		0	0	0	0
Lane Width (ft)					
Walking Speed (ft/sec)					
Pedestrian Blockage Factor, f_pb					

Delay, Queue Length, and Level of Service

Approach Movement	1U U	NB 1 L	4U 4 L	7 7	WestBound 8 LTR	9 9	10 10 L	EastBound 11 11	12 12 TR
Lane Configuration									
Flow Rate		7	5		51		40		21
Lane Capacity		458	622		43		28		51
v/c		0.01	0.01		1.20		1.42		0.41
95% Queue Length		0.0	0.0		4.9		4.7		1.5
Control Delay		13.0	10.8		352.3		529.2		118.5
LOS		B	B		F		F		F
Approach Delay		0.1	0.0		352.3			389.8	
Approach LOS					F			F	
Intersction Delay		17.0							

Step 1: MOVEMENT PRIORITIES

Major Street: Approach Priority Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R
Minor Street:									

Approach Priority Movement	WestBound				EastBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x	0	6	909	16		0	5	1103	125
Flow Rate, v_x		7	988	17			5	1199	136

Minor Street: Approach Movement	WestBound				EastBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Volume, V_x		24	19	4		37	15	4
Flow Rate, v_x		26	21	4		40	16	4

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		7	988	17		5	1199	136	
Conflicting Flow, v_c, x		1335				1005			

Minor Street: Approach Movement	WestBound				EastBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Flow Rate, v_x		26	21	4		40	16	4
Conflicting Flow, v_c, x		1628	2355	503		1727	2228	599

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	1U U	NB	4U U	SB	4	7	WestBound		10	EastBound	
		1 L					8 T	9 R		11 T	12 R
t_c, base											
Single Stage		4.1		4.1	7.5	6.5	6.9	7.5	6.5	6.9	
Stage I											
Stage II											
t_c, HV		2.0		2.0	2.0	2.0	2.0	2.0	2.0	2.0	
P_HV		0.13		0.13	0.03	0.03	0.03	0.03	0.03	0.03	
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1	
G					0	0	0	0	0	0	
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
t_c											
Single Stage		4.36		4.36	7.56	6.56	6.96	7.56	6.56	6.96	
Stage I											
Stage II											

FOLLOW-UP HEADWAYS Approach Movement	1U U	NB	4U U	SB	4	7	WestBound		10	EastBound	
		1 L					8 T	9 R		11 T	12 R
t_f, base											
t_f, HV		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3	
P_HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	
t_f		0.13		0.13	0.03	0.03	0.03	0.03	0.03	0.03	
		2.33		2.33	3.53	4.03	3.33	3.53	4.03	3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	1U U	NB	4U U	SB	4	7	WestBound		10	EastBound	
		1 L					8 T	9 R		11 T	12 R
v_c, x		1335		1005	1628	2355	503	1727	2228	599	
t_c, x		4.36		4.36	7.56	6.56	6.96	7.56	6.56	6.96	
t_f, x		2.33		2.33	3.53	4.03	3.33	3.53	4.03	3.33	
c_p, x		458		622	67	35	511	56	42	442	

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	NR	SR	WR	EB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		1335	1005	
Potential Capacity, $c_{p,x}$		458	622	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		458	622	
Probability of Queue-free State, $p_{0,j}$		0.986	0.991	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		503	599	
Potential Capacity, $c_{p,x}$		511	442	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		511	442	
Probability of Queue-free State, $p_{0,j}$		0.991	0.990	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$		2355	2228	
Potential Capacity, $c_{p,x}$		35	42	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Capacity Adjustment Factor, $f_x$		0.977	0.977	
Movement Capacity, $c_{m,x}$		34	41	
Probability of Queue-free State, $p_{0,j}$		0.391	0.602	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$		1628	1727	
Potential Capacity, $c_{p,x}$		67	56	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$		0.588	0.382	
Major L, Minor T Impedance Factor, $p'$		0.678	0.506	
Capacity Adjustment Factor, $f_x$		0.672	0.502	
Movement Capacity, $c_{m,x}$		45	28	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	WestBound			EastBound		
Movement	7	8	9	10	11	12
Lane Configuration		LTR		L		TR
Shared Flow Rate, $v_y$		51		40		21
Movement Capacity, $c_{m,x}$	45	34	511	28	41	442
Shared Capacity, $c_{SH}$		43				51

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
Movement										
Flow Rate		7		5	26	21	4	40	16	4
Movement Capacity		458		622	45	34	511	28	41	442
Lane Configuration		L		L		LTR		L		TR
Shared Capacity						43				51
Control Delay		13.0		10.8		352.3		529.2		118.5

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
Movement										
Lane Configuration										
Flow Rate		7		5		51		40		21
Lane Capacity		458		622		43		28		51
$v/c$		0.01		0.01		1.20		1.42		0.41
95% Queue Length		0.0		0.0		4.9		4.7		1.5
Control Delay		13.0		10.8		352.3		529.2		118.5

LOS	B	B	F	F	F
Approach Delay	0.1	0.0	352.3		389.8
Approach LOS			F		F
Intersection Delay	17.0				

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 4:10:30 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_AM\_NO BUILD\_Wortman-OSR-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Wortman Rd / OSR  
 Major Street Direction: North-South  
 East/West Street Name: Wortman Road  
 North/South Street Name: Old State Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound				SouthBound			
Approach	1U	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R
Volume		12	85				52	7
Peak Hour Factor, PHF				0.92				
Hourly Flow Rate, HFR		13	92				57	8
Percent Heavy Vehicles		3						
Number of Lanes	0	1	1	0	0	0	1	1
Lane Configuration		L	T				T	R
Median Type	Undivided							
Median Storage								
RT channelized?								
Left-Turn Lane Storage								
Upstream Signal?	Not Present							

Minor Street:	WestBound			EastBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume				18		24
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR				20		26
Percent Heavy Vehicles				3		3
Number of Lanes	0	0	0	1	0	1
Lane Configuration				L		R
RT channelized?						
Flared Approach   Storage					0	
Percent Grade						

Pedestrian Volumes and Adjustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)	0	0		0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	NB	SB	WB	EB
Movement	1U	4U	7	8
Lane Configuration	L	T	R	R
Flow Rate	13			20
Lane Capacity	1532			806
v/c	0.01			0.02
95% Queue Length	0.0			0.1
Control Delay	7.4			9.6
LOS	A			A
Approach Delay	0.9			
Approach LOS				9.1
Intersction Delay				A

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound				SouthBound			
Approach	1U	2	3	4U	4	5	6	
Priorty	U	L	T	R	U	L	T	R
Movement								

Minor Street:

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement	1U	NorthBound				4U	SouthBound		
	U	1	2	3		U	4	5	6
		L	T	R			L	T	R
Volume, V_x		12	85				52	7	
Flow Rate, v_x		13	92				57	8	

Minor Street:									
Approach Movement		WestBound				EastBound			
		7	8	9			10	11	12
		L	T	R			L	T	R
Volume, V_x							18	24	
Flow Rate, v_x							20	26	

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement	1U	NorthBound				4U	SouthBound		
	U	1	2	3		U	4	5	6
		L	T	R			L	T	R
Flow Rate, v_x		13	92				57	8	
Conflicting Flow, v_c, x		64							

Minor Street:									
Approach Movement		WestBound				EastBound			
		7	8	9			10	11	12
		L	T	R			L	T	R
Flow Rate, v_x							20	26	
Conflicting Flow, v_c, x							175	57	

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS												
Approach Movement	NB	1	4U	4	7	WestBound	8	9	10	EastBound	11	12
	1U	L	U	L	L	T	R	L	L	T	T	R
	U											
t_c, base												
Single Stage		4.1							7.1			6.2
Stage I												
Stage II												
t_c, HV		1.0							1.0			1.0
P_HV		0.03							0.03			0.03
t_c, G									0.2			0.1
G									0			0
t_3, LT		0.0							0.7			0.0
t_c												
Single Stage		4.13							6.43			6.23
Stage I												
Stage II												

FOLLOW-UP HEADWAYS												
Approach Movement	NB	1	4U	4	7	WestBound	8	9	10	EastBound	11	12
	1U	L	U	L	L	T	R	L	L	T	T	R
	U											
t_f, base		2.2							3.5			3.3
t_f, HV		0.9							0.9			0.9
P_HV		0.03							0.03			0.03
t_f		2.23							3.53			3.33

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT												
Approach Movement	NB	1	4U	4	7	WestBound	8	9	10	EastBound	11	12
	1U	L	U	L	L	T	R	L	L	T	T	R
	U											
v_c, x		64							175			57
t_c, x		4.13							6.43			6.23
t_f, x		2.23							3.53			3.33
c_p, x		1532							813			1007

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	NR	SR	WR	EB
-------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0		0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		64		
Potential Capacity, $c_{p,x}$		1532		
Pedestrian Impedance Factor, $p_{p,x}$		1.000		
Movement Capacity, $c_{m,x}$		1532		
Probability of Queue-free State, $p_{0,j}$		0.991		
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$			57	
Potential Capacity, $c_{p,x}$			1007	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Movement Capacity, $c_{m,x}$			1007	
Probability of Queue-free State, $p_{0,j}$			0.974	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$			175	
Potential Capacity, $c_{p,x}$			813	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$			0.991	
Movement Capacity, $c_{m,x}$			806	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES											
Approach		WestBound				EastBound					
Movement	7	8	9			10	11	12			
Lane Configuration						L		R			
Shared Flow Rate, $v_y$						20		26			
Movement Capacity, $c_{m,x}$						806		1007			
Shared Capacity, $c_{SH}$											

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS											
Approach		NB		SB		WestBound		EastBound			
Movement	1U	1	4U	4	7	8	9	10	11	12	
Flow Rate		13						20		26	
Movement Capacity		1532						806		1007	
Lane Configuration		L						L		R	
Shared Capacity											
Control Delay		7.4						9.6		8.7	

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach		NB		SB		WestBound		EastBound			
Movement	1U	1	4U	4	7	8	9	10	11	12	
Lane Configuration		L						L		R	
Flow Rate		13						20		26	
Lane Capacity		1532						806		1007	
$v/c$		0.01						0.02		0.03	
95% Queue Length		0.0						0.1		0.1	
Control Delay		7.4						9.6		8.7	



LOS  
Approach Delay  
Approach LOS  
Intersection Delay

A  
0.9

A

9.1  
A

A

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 1:16:53 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_PM\_NO BUILD\_Wortman-OSR-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Wortman Rd / OSR  
 Major Street Direction: North-South  
 East/West Street Name: Wortman Road  
 North/South Street Name: Old State Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound				SouthBound			
Approach Movement	1U	2	3	4U	4	5	6	
	U	L	T	R	U	L	T	R
Volume		22	74				115	28
Peak Hour Factor, PHF					0.92			
Hourly Flow Rate, HFR		24	80				125	30
Percent Heavy Vehicles		3						
Number of Lanes	0	1	1	0	0	0	1	1
Lane Configuration		L	T				T	R
Median Type	Undivided							
Median Storage								
RT channelized?								
Left-Turn Lane Storage								
Upstream Signal?	Not Present							

Minor Street:	WestBound			EastBound		
Approach Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume				15		18
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR				16		20
Percent Heavy Vehicles				3		3
Number of Lanes	0	0	0	1	0	1
Lane Configuration				L		R
RT channelized?						
Flared Approach   Storage					0	
Percent Grade						

Pedestrian Volumes and Adjustments

Approach Movement	NB	SB	WB	EB
	13	14	15	16
Flow (ped/hr)	0	0		0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Approach Movement	Delay, Queue Length, and Level of Service											
	1U	NB	2	4U	3	7	8	9	10	EastBound	11	12
	U	L	T	R	L	T	R	L	T	R	L	R
Flow Rate	24								16			20
Lane Capacity	1419								721			923
v/c	0.02								0.02			0.02
95% Queue Length	0.1								0.1			0.1
Control Delay	7.6								10.1			9.0
LOS	A								B			A
Approach Delay	1.7									9.5		
Approach LOS										A		
Intersction Delay	1.8											

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound				SouthBound			
Approach Priority Movement	1U	2	3	4U	4	5	6	
	U	L	T	R	U	L	T	R
Minor Street:								

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement	1U	NorthBound				4U	SouthBound		
	U	1	2	3		U	4	5	6
		L	T	R			L	T	R
Volume, V_x		22	74				115	28	
Flow Rate, v_x		24	80				125	30	

Minor Street:									
Approach Movement		WestBound				EastBound			
		7	8	9			10	11	12
		L	T	R			L	T	R
Volume, V_x							15	18	
Flow Rate, v_x							16	20	

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement	1U	NorthBound				4U	SouthBound		
	U	1	2	3		U	4	5	6
		L	T	R			L	T	R
Flow Rate, v_x		24	80				125	30	
Conflicting Flow, v_c, x		155							

Minor Street:									
Approach Movement		WestBound				EastBound			
		7	8	9			10	11	12
		L	T	R			L	T	R
Flow Rate, v_x							16	20	
Conflicting Flow, v_c, x							253	125	

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS										
Approach Movement		NB		SB		WestBound			EastBound	
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_c, base										
Single Stage		4.1						7.1		6.2
Stage I										
Stage II										
t_c, HV		1.0						1.0		1.0
P_HV		0.03						0.03		0.03
t_c, G								0.2		0.1
G								0		0
t_3, LT		0.0						0.7		0.0
t_c										
Single Stage		4.13						6.43		6.23
Stage I										
Stage II										

FOLLOW-UP HEADWAYS										
Approach Movement		NB		SB		WestBound			EastBound	
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_f, base		2.2						3.5		3.3
t_f, HV		0.9						0.9		0.9
P_HV		0.03						0.03		0.03
t_f		2.23						3.53		3.33

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach Movement		NB		SB		WestBound			EastBound	
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
v_c, x		155						253		125
t_c, x		4.13						6.43		6.23
t_f, x		2.23						3.53		3.33
c_p, x		1419						733		923

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0		0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		155		
Potential Capacity, $c_{p,x}$		1419		
Pedestrian Impedance Factor, $p_{p,x}$		1.000		
Movement Capacity, $c_{m,x}$		1419		
Probability of Queue-free State, $p_{0,j}$		0.983		
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$			125	
Potential Capacity, $c_{p,x}$			923	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Movement Capacity, $c_{m,x}$			923	
Probability of Queue-free State, $p_{0,j}$			0.979	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$			253	
Potential Capacity, $c_{p,x}$			733	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$			0.983	
Movement Capacity, $c_{m,x}$			721	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	WestBound			EastBound		
Movement	7	8	9	10	11	12
Lane Configuration				L		R
Shared Flow Rate, $v_y$				16		20
Movement Capacity, $c_{m,x}$				721		923
Shared Capacity, $c_{SH}$						

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		24						16		20
Movement Capacity		1419						721		923
Lane Configuration		L						L		R
Shared Capacity										
Control Delay		7.6						10.1		9.0

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration		L						L		R
Flow Rate		24						16		20
Lane Capacity		1419						721		923
$v/c$		0.02						0.02		0.02
95% Queue Length		0.1						0.1		0.1
Control Delay		7.6						10.1		9.0

LOS	A	B		A
Approach Delay	1.7		9.5	
Approach LOS			A	
Intersection Delay	1.8			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 1:22:34 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_AM\_NO BUILD\_Hillisdale-Walnut-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Hillisdale Rd / Walnut Rd  
 Major Street Direction: East-West  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: Walnut Avenue  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Volume		1	84	26		14	120	5	
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		1	91	28		15	130	5	
Percent Heavy Vehicles		3				3			
Number of Lanes	0	0	1	0	0	0	1	0	
Lane Configuration			LTR				LTR		
Median Type					Undivided				
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street:	NorthBound			SouthBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume	26	6	23	5	6	2
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR	28	7	25	5	7	2
Percent Heavy Vehicles	3	3	3	3	3	3
Number of Lanes	0	1	0	0	1	0
Lane Configuration		LTR			LTR	
RT channelized?						
Flared Approach   Storage	No			No		
Percent Grade		0			0	

Pedestrian Volumes and Adjustments

Approach	EB	WB	NB	SB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	EB	WB	NorthBound			SouthBound			
Movement	1U	4U	4	7	8	9	10	11	12
Lane Configuration			LTR		LTR		LTR		
Flow Rate	1	15	60		14				
Lane Capacity	1442	1462	751		656				
v/c	0.00	0.01	0.08		0.02				
95% Queue Length	0.0	0.0	0.3		0.1				
Control Delay	7.5	7.5	10.2		10.6				
LOS	A	A	B		B				
Approach Delay	0.1	0.8	10.2		10.6				
Approach LOS			B		B				
Intersection Delay	2.6								

Step 1: MOVEMENT PRIORITIES

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Priority									

Minor Street:

Approach Priority Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x		1	84	26			14	120	5
Flow Rate, v_x		1	91	28			15	130	5

Minor Street: Approach Movement	NorthBound				SouthBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Volume, V_x		26	6	23		5	6	2
Flow Rate, v_x		28	7	25		5	7	2

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		1	91	28			15	130	5
Conflicting Flow, v_c, x		136					120		

Minor Street: Approach Movement	NorthBound				SouthBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Flow Rate, v_x		28	7	25		5	7	2
Conflicting Flow, v_c, x		276	274	105		287	285	133

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_c, base											
Single Stage		4.1		4.1	7.1	6.5	6.2	7.1	6.5	6.2	
Stage I											
Stage II											
t_c, HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	
P_HV		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1	
G					0	0	0	0	0	0	
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
t_c											
Single Stage		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23	
Stage I											
Stage II											

FOLLOW-UP HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_f, base											
t_f, HV		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3	
P_HV		0.9		0.9	0.9	0.9	0.9	0.9	0.9	0.9	
t_f		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
c_p, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound		
					7 L	8 T	9 R	10 L	11 T	12 R
v_c, x		136		120	276	274	105	287	285	133
t_c, x		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23
t_f, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33
c_p, x		1442		1462	675	632	946	663	622	913

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	FR	WR	NR	SB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		136	120	
Potential Capacity, $c_{p,x}$		1442	1462	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		1442	1462	
Probability of Queue-free State, $p_{0,j}$		0.999	0.990	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$		0.999	0.989	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		105	133	
Potential Capacity, $c_{p,x}$		946	913	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		946	913	
Probability of Queue-free State, $p_{0,j}$		0.974	0.998	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$		274	285	
Potential Capacity, $c_{p,x}$		632	622	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Capacity Adjustment Factor, $f_x$		0.988	0.988	
Movement Capacity, $c_{m,x}$		624	615	
Probability of Queue-free State, $p_{0,j}$		0.990	0.989	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$		276	287	
Potential Capacity, $c_{p,x}$		675	663	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$		0.977	0.978	
Major L, Minor T Impedance Factor, $p'$		0.983	0.983	
Capacity Adjustment Factor, $f_x$		0.980	0.957	
Movement Capacity, $c_{m,x}$		662	635	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	NorthBound			SouthBound		
Movement	7	8	9	10	11	12
Lane Configuration		LTR			LTR	
Shared Flow Rate, $v_y$		60			14	
Movement Capacity, $c_{m,x}$	662	624	946	635	615	913
Shared Capacity, $c_{SH}$		751			656	

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB		WB		NorthBound			SouthBound		
	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		1		15	28	7	25	5	7	2
Movement Capacity		1442		1462	662	624	946	635	615	913
Lane Configuration						LTR			LTR	
Shared Capacity						751			656	
Control Delay		7.5		7.5		10.2			10.6	

CONTROL DELAY TO RANK 1 MOVEMENTS		
Approach	EB	WB
Movement	2	5
Number of Major Street Through Lanes, $N$	1	1
Proportion of Rank 1 vehicles not blocked, $p^*_{0,j}$	0.999	0.989
Delay to Major Left-turning Vehicles, $d_{MLT}$	7.5	7.5
Major Street Through Vehicles in Shared Lane, $v_{i1}$	91	130
Major Street Turning Vehicles in Shared Lane, $v_{i2}$	29	21
Saturation Flow Rate for Major Street Through, $s_{i1}$	1800	1800



Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

1500  
 0.0

1500  
 0.1

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement Lane Configuration	1U	EB 1	4U	WB 4	7	NorthBound 8	9	10	SouthBound 11	12
						LTR			LTR	
Flow Rate		1		15		60			14	
Lane Capacity		1442		1462		751			656	
v/c		0.00		0.01		0.08			0.02	
95% Queue Length		0.0		0.0		0.3			0.1	
Control Delay		7.5		7.5		10.2			10.6	
LOS		A		A		B			B	
Approach Delay		0.1		0.8		10.2			10.6	
Approach LOS						B			B	
Intersection Delay		2.6								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 4:15:41 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_PM\_NO BUILD\_Hillisdale-Walnut-x.txt  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Hillisdale Rd / Walnut Rd  
 Major Street Direction: East-West  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: Walnut Avenue  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Volume		14	185	28		12	110	26	
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		15	201	30		13	120	28	
Percent Heavy Vehicles		3				3			
Number of Lanes	0	0	1	0	0	0	1	0	
Lane Configuration			LTR				LTR		
Median Type					Undivided				
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street:	NorthBound			SouthBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume	36	12	39	8	9	3
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR	39	13	42	9	10	3
Percent Heavy Vehicles	3	3	3	3	3	3
Number of Lanes	0	1	0	0	1	0
Lane Configuration		LTR			LTR	
RT channelized?						
Flared Approach   Storage	No			No		
Percent Grade		0			0	

Pedestrian Volumes and Adjustments

Approach	EB	WB	NB	SB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	EB	WB	NorthBound			SouthBound			
Movement	1U	4U	4	7	8	9	10	11	12
Lane Configuration					LTR			LTR	
Flow Rate	15		13		95			22	
Lane Capacity	1428		1330		625			534	
v/c	0.01		0.01		0.15			0.04	
95% Queue Length	0.0		0.0		0.5			0.1	
Control Delay	7.5		7.7		11.8			12.0	
LOS	A		A		B			B	
Approach Delay	0.6		0.7		11.8			12.0	
Approach LOS					B			B	
Intersection Delay	3.1								

Step 1: MOVEMENT PRIORITIES

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Priority									

Minor Street:

Approach Priority Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x		14	185	28			12	110	26
Flow Rate, v_x		15	201	30			13	120	28

Minor Street: Approach Movement	NorthBound				SouthBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Volume, V_x		36	12	39		8	9	3
Flow Rate, v_x		39	13	42		9	10	3

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		15	201	30			13	120	28
Conflicting Flow, v_c, x		148					232		

Minor Street: Approach Movement	NorthBound				SouthBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Flow Rate, v_x		39	13	42		9	10	3
Conflicting Flow, v_c, x		413	421	216		434	422	134

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_c, base											
Single Stage		4.1		4.1	7.1	6.5	6.2	7.1	6.5	6.2	
Stage I											
Stage II											
t_c, HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	
P_HV		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1	
G					0	0	0	0	0	0	
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
t_c											
Single Stage		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23	
Stage I											
Stage II											

FOLLOW-UP HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_f, base											
t_f, HV		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3	
P_HV		0.9		0.9	0.9	0.9	0.9	0.9	0.9	0.9	
t_f		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
c_p, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound		
					7 L	8 T	9 R	10 L	11 T	12 R
v_c, x		148		232	413	421	216	434	422	134
t_c, x		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23
t_f, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33
c_p, x		1428		1330	548	523	821	530	522	913

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	FR	WR	NR	SB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		148	232	
Potential Capacity, $c_{p,x}$		1428	1330	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		1428	1330	
Probability of Queue-free State, $p_{0,j}$		0.989	0.990	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$		0.988	0.989	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		216	134	
Potential Capacity, $c_{p,x}$		821	913	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		821	913	
Probability of Queue-free State, $p_{0,j}$		0.948	0.996	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$		421	422	
Potential Capacity, $c_{p,x}$		523	522	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Capacity Adjustment Factor, $f_x$		0.977	0.977	
Movement Capacity, $c_{m,x}$		511	510	
Probability of Queue-free State, $p_{0,j}$		0.974	0.981	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$		413	434	
Potential Capacity, $c_{p,x}$		548	530	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$		0.958	0.952	
Major L, Minor T Impedance Factor, $p'$		0.968	0.963	
Capacity Adjustment Factor, $f_x$		0.965	0.914	
Movement Capacity, $c_{m,x}$		528	485	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	NorthBound			SouthBound		
Movement	7	8	9	10	11	12
Lane Configuration		LTR			LTR	
Shared Flow Rate, $v_y$		95			22	
Movement Capacity, $c_{m,x}$	528	511	821	485	510	913
Shared Capacity, $c_{SH}$		625			534	

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB			WB						
	1U	1	4U	4	7	8	9	10	11	12
Movement										
Flow Rate		15		13	39	13	42	9	10	3
Movement Capacity		1428		1330	528	511	821	485	510	913
Lane Configuration						LTR			LTR	
Shared Capacity						625			534	
Control Delay		7.5		7.7		11.8			12.0	

CONTROL DELAY TO RANK 1 MOVEMENTS		
Approach	EB	WB
Movement	2	5
Number of Major Street Through Lanes, $N$	1	1
Proportion of Rank 1 vehicles not blocked, $p^*_{0,j}$	0.988	0.989
Delay to Major Left-turning Vehicles, $d_{MLT}$	7.5	7.7
Major Street Through Vehicles in Shared Lane, $v_{i1}$	201	120
Major Street Turning Vehicles in Shared Lane, $v_{i2}$	46	41
Saturation Flow Rate for Major Street Through, $s_{i1}$	1800	1800

Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

1500  
 0.1

1500  
 0.1

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement Lane Configuration	1U	EB 1	4U	WB 4	7	NorthBound 8	9	10	SouthBound 11	12
						LTR			LTR	
Flow Rate		15		13		95			22	
Lane Capacity		1428		1330		625			534	
v/c		0.01		0.01		0.15			0.04	
95% Queue Length		0.0		0.0		0.5			0.1	
Control Delay		7.5		7.7		11.8			12.0	
LOS		A		A		B			B	
Approach Delay		0.6		0.7		11.8			12.0	
Approach LOS						B			B	
Intersection Delay		3.1								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 4:18:09 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_AM\_NO BUILD\_Hillisdale-US 41-x. xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Hillisdale Rd / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R
Volume	0	26	1229	54		0	43	1155	1
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		28	1336	59			47	1255	1
Percent Heavy Vehicles	3	3				3	3		
Number of Lanes	0	1	2	0		0	1	2	0
Lane Configuration		L	T	TR			L	T	TR
Median Type					Undivided				
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street: Approach Movement		WestBound 7 L	8 T	9 R		EastBound 10 L	11 T	12 R
Volume		124	10	54		0	28	18
Peak Hour Factor, PHF					0.92			
Hourly Flow Rate, HFR		135	11	59		0	30	20
Percent Heavy Vehicles		3	3	3		3	3	3
Number of Lanes		0	1	0		0	1	0
Lane Configuration			LTR				LTR	
RT channelized?								
Flared Approach   Storage		No				No		
Percent Grade			0				0	

Pedestrian Volumes and Adjustments

Approach Movement		NB 13	SB 14	WB 15	EB 16
Flow (ped/hr)		0	0	0	0
Lane Width (ft)					
Walking Speed (ft/sec)					
Pedestrian Blockage Factor, f <sub>pb</sub>					

Delay, Queue Length, and Level of Service

Approach Movement	1U U	NB 1 L	4U L	7 L	8 LTR	9 L	10 L	EastBound 11 LTR	12 L
Lane Configuration									
Flow Rate		28		47	204			50	
Lane Capacity		544		481				24	
v/c		0.05		0.10				2.04	
95% Queue Length		0.2		0.3				6.2	
Control Delay		12.0		13.3				823.9	
LOS		B		B				F	
Approach Delay		0.2		0.5				823.9	
Approach LOS								F	
Intersction Delay									

Step 1: MOVEMENT PRIORITIES

Major Street: Approach Priority Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R

Minor Street:

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U	NorthBound			4U	SouthBound			
	U	1	2	3		4	5	6	
		L	T	R		L	T	R	
Volume, V_x	0	26	1229	54		0	43	1155	1
Flow Rate, v_x		28	1336	59			47	1255	1

Minor Street: Approach Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Volume, V_x	124	10	54		0	28	18
Flow Rate, v_x	135	11	59		0	30	20

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U	NorthBound			4U	SouthBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Flow Rate, v_x		28	1336	59		47	1255	1
Conflicting Flow, v_c, x		1257				1395		

Minor Street: Approach Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Flow Rate, v_x	135	11	59		0	30	20
Conflicting Flow, v_c, x	2158	2772	697		2079	2801	628

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRI TICAL HEADWAYS Approach Movement	NB	SB			WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_c, base Single Stage		4.1		4.1	7.5	6.5	6.9	7.5	6.5	6.9
Stage I										
Stage II										
t_c, HV		2.0		2.0	2.0	2.0	2.0	2.0	2.0	2.0
P_HV		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1
G					0	0	0	0	0	0
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0
t_c Single Stage		4.16		4.16	7.56	6.56	6.96	7.56	6.56	6.96
Stage I										
Stage II										

FOLLOW-UP HEADWAYS Approach Movement	NB	SB			WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_f, base		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3
t_f, HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0
P_HV		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03
t_f		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	NB	SB			WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
v_c, x		1257		1395	2158	2772	697	2079	2801	628
t_c, x		4.16		4.16	7.56	6.56	6.96	7.56	6.56	6.96
t_f, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33
c_p, x		544		481	26	19	381	30	18	423

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	NR	SR	WR	EB
-------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		1257	1395	
Potential Capacity, $c_{p,x}$		544	481	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		544	481	
Probability of Queue-free State, $p_{0,j}$		0.948	0.903	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		697	628	
Potential Capacity, $c_{p,x}$		381	423	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		381	423	
Probability of Queue-free State, $p_{0,j}$		0.846	0.954	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$		2772	2801	
Potential Capacity, $c_{p,x}$		19	18	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Capacity Adjustment Factor, $f_x$		0.856	0.856	
Movement Capacity, $c_{m,x}$		16	15	
Probability of Queue-free State, $p_{0,j}$		0.317		
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$		2158	2079	
Potential Capacity, $c_{p,x}$		26	30	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$			0.271	
Major L, Minor T Impedance Factor, $p'$			0.406	
Capacity Adjustment Factor, $f_x$			0.343	
Movement Capacity, $c_{m,x}$			10	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES										
Approach	WestBound				EastBound					
Movement	7	8	9	10	11	12				
Lane Configuration		LTR			LTR					
Shared Flow Rate, $v_y$		204			50					
Movement Capacity, $c_{m,x}$		16	381	10	15	423				
Shared Capacity, $c_{SH}$					24					

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		28		47	135	11	59	0	30	20
Movement Capacity		544		481		16	381	10	15	423
Lane Configuration		L		L		LTR			LTR	
Shared Capacity									24	
Control Delay		12.0		13.3					823.9	

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration		L		L		LTR			LTR	
Flow Rate		28		47		204			50	
Lane Capacity		544		481					24	
$v/c$		0.05		0.10					2.04	
95% Queue Length		0.2		0.3					6.2	
Control Delay		12.0		13.3					823.9	



LOS  
Approach Delay  
Approach LOS  
Intersection Delay

B  
0.2

B  
0.5

F  
823.9  
F

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 4:20:22 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_PM\_NO BUILD\_Hillisdale-US 41-x. xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Hillisdale Rd / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R
Volume	0	26	1229	54		0	66	1315	1
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		28	1336	59			72	1429	1
Percent Heavy Vehicles	3	3				3	3		
Number of Lanes	0	1	2	0		0	1	2	0
Lane Configuration		L	T	TR			L	T	TR
Median Type					Undivided				
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street: Approach Movement	WestBound 7 L	8 T	9 R		EastBound 10 L	11 T	12 R
Volume	53	27	57		4	27	8
Peak Hour Factor, PHF				0.92			
Hourly Flow Rate, HFR	58	29	62		4	29	9
Percent Heavy Vehicles	3	3	3		3	3	3
Number of Lanes	0	1	0		0	1	0
Lane Configuration		LTR				LTR	
RT channelized?							
Flared Approach   Storage	No				No		
Percent Grade		0				0	

Pedestrian Volumes and Adjustments

Approach Movement	NB 13	SB 14	WB 15	EB 16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach Movement	1U U	NB 1 L	4U L	7 L	SB 4 L	8 LTR	9 L	10 L	EastBound 11 LTR	12 L
Lane Configuration										
Flow Rate		28			72		149		42	
Lane Capacity		466			481					
v/c		0.06			0.15					
95% Queue Length		0.2			0.5					
Control Delay		13.2			13.8					
LOS		B			B					
Approach Delay		0.3			0.7					
Approach LOS										
Intersection Delay										

Step 1: MOVEMENT PRIORITIES

Major Street: Approach Priority Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R

Minor Street:

Approach Priority Movement	WestBound				EastBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x	0	26	1229	54		0	66	1315	1
Flow Rate, v_x		28	1336	59			72	1429	1

Minor Street: Approach Movement	WestBound				EastBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Volume, V_x		53	27	57		4	27	8
Flow Rate, v_x		58	29	62		4	29	9

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		28	1336	59			72	1429	1
Conflicting Flow, v_c, x		1430					1395		

Minor Street: Approach Movement	WestBound				EastBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Flow Rate, v_x		58	29	62		4	29	9
Conflicting Flow, v_c, x		2295	2996	697		2313	3024	715

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	NB 1U U	1 L	SB 4U U	4 L	7 L	WestBound		9 R	10 L	EastBound	
						8 T	9 R			11 T	12 R
t_c, base											
Single Stage		4.1		4.1	7.5	6.5	6.9		7.5	6.5	6.9
Stage I											
Stage II											
t_c, HV		2.0		2.0	2.0	2.0	2.0		2.0	2.0	2.0
P_HV		0.03		0.03	0.03	0.03	0.03		0.03	0.03	0.03
t_c, G					0.2	0.2	0.1		0.2	0.2	0.1
G					0	0	0		0	0	0
t_3, LT		0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0
t_c											
Single Stage		4.16		4.16	7.56	6.56	6.96		7.56	6.56	6.96
Stage I											
Stage II											

FOLLOW-UP HEADWAYS Approach Movement	NB 1U U	1 L	SB 4U U	4 L	7 L	WestBound		9 R	10 L	EastBound	
						8 T	9 R			11 T	12 R
t_f, base											
t_f, HV		2.2		2.2	3.5	4.0	3.3		3.5	4.0	3.3
P_HV		1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0
t_f		0.03		0.03	0.03	0.03	0.03		0.03	0.03	0.03
c_p, x		2.23		2.23	3.53	4.03	3.33		3.53	4.03	3.33

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	NB 1U U	1 L	SB 4U U	4 L	7 L	WestBound		9 R	10 L	EastBound	
						8 T	9 R			11 T	12 R
v_c, x		1430		1395	2295	2996	697		2313	3024	715
t_c, x		4.16		4.16	7.56	6.56	6.96		7.56	6.56	6.96
t_f, x		2.23		2.23	3.53	4.03	3.33		3.53	4.03	3.33
c_p, x		466		481	21	13	381		20	13	371

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	NR	SR	WR	EB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		1430	1395	
Potential Capacity, $c_{p,x}$		466	481	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		466	481	
Probability of Queue-free State, $p_{0,j}$		0.939	0.851	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		697	715	
Potential Capacity, $c_{p,x}$		381	371	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		381	371	
Probability of Queue-free State, $p_{0,j}$		0.837	0.977	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$		2996	3024	
Potential Capacity, $c_{p,x}$		13	13	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Capacity Adjustment Factor, $f_x$		0.799	0.799	
Movement Capacity, $c_{m,x}$		11	10	
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$		2295	2313	
Potential Capacity, $c_{p,x}$		21	20	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES										
Approach	WestBound				EastBound					
Movement	7	8	9	10	11	12				
Lane Configuration		LTR			LTR					
Shared Flow Rate, $v_y$		149			42					
Movement Capacity, $c_{m,x}$		11	381		10	371				
Shared Capacity, $c_{SH}$										

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		28		72	58	29	62	4	29	9
Movement Capacity		466		481		11	381		10	371
Lane Configuration		L		L		LTR			LTR	
Shared Capacity										
Control Delay		13.2		13.8						

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound	
Movement	1U	1	4U	4	7	8	9	10	11
Lane Configuration		L		L		LTR			LTR
Flow Rate		28		72		149			42
Lane Capacity		466		481					
$v/c$		0.06		0.15					
95% Queue Length		0.2		0.5					
Control Delay		13.2		13.8					

LOS  
Approach Delay  
Approach LOS  
Intersection Delay

B  
0.3

B  
0.7

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 4:22:46 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_AM\_NO BUILD\_Hillisdale-Old State-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Hillisdale Rd / Old State  
 Major Street Direction: North-South  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: Old State Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound				SouthBound			
Approach	1U	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R
Volume		123	39			21	76	
Peak Hour Factor, PHF				0.92				
Hourly Flow Rate, HFR		134	42			23	83	
Percent Heavy Vehicles						3		
Number of Lanes	0	0	1	0	0	0	1	0
Lane Configuration				TR		LT		
Median Type	Undivided							
Median Storage								
RT channelized?								
Left-Turn Lane Storage								
Upstream Signal?	Not Present							

Minor Street:	WestBound			EastBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume	39		8			
Peak Hour Factor, PHF			0.92			
Hourly Flow Rate, HFR	42		9			
Percent Heavy Vehicles	3		3			
Number of Lanes	0	1	0	0	0	0
Lane Configuration		LR				
RT channelized?						
Flared Approach   Storage	No					
Percent Grade		0				

Pedestrian Volumes and Adjustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	NB	SB	WestBound	EastBound
Movement	1U	4U	7	8
Lane Configuration		LT	LR	
Flow Rate		23	51	
Lane Capacity		1394	720	
v/c		0.02	0.07	
95% Queue Length		0.0	0.2	
Control Delay		7.6	10.4	
LOS		A	B	
Approach Delay		1.8	10.4	
Approach LOS			B	
Intersection Delay	2.2			

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound				SouthBound			
Approach	1U	2	3	4U	4	5	6	
Priority	U	L	T	R	U	L	T	R
Movement								

Minor Street:

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:		NorthBound				SouthBound			
Approach Movement	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Volume, V_x		123	39				21	76	
Flow Rate, v_x		134	42				23	83	

Minor Street:		WestBound				EastBound		
Approach Movement		7	8	9		10	11	12
		L	T	R		L	T	R
Volume, V_x		39	8					
Flow Rate, v_x		42	9					

Step 3: CONFLICTING FLOW RATES

Major Street:		NorthBound				SouthBound			
Approach Movement	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Flow Rate, v_x		134	42				23	83	
Conflicting Flow, v_c, x							176		

Minor Street:		WestBound				EastBound		
Approach Movement		7	8	9		10	11	12
		L	T	R		L	T	R
Flow Rate, v_x		42	9					
Conflicting Flow, v_c, x		283	155					

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS												
Approach Movement	NB	1	4U	4	7	WestBound	8	9	10	EastBound	11	12
	1U	L	U	L	L	T	R	L	L	T	T	R
	U		U						L			R
t_c, base												
Single Stage				4.1	7.1			6.2				
Stage I												
Stage II												
t_c, HV				1.0	1.0		1.0					
P_HV				0.03	0.03		0.03					
t_c, G					0.2		0.1					
G					0		0					
t_3, LT				0.0	0.7		0.0					
t_c												
Single Stage				4.13	6.43		6.23					
Stage I												
Stage II												

FOLLOW-UP HEADWAYS												
Approach Movement	NB	1	4U	4	7	WestBound	8	9	10	EastBound	11	12
	1U	L	U	L	L	T	R	L	L	T	T	R
	U		U						L			R
t_f, base				2.2	3.5		3.3					
t_f, HV				0.9	0.9		0.9					
P_HV				0.03	0.03		0.03					
t_f				2.23	3.53		3.33					

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT												
Approach Movement	NB	1	4U	4	7	WestBound	8	9	10	EastBound	11	12
	1U	L	U	L	L	T	R	L	L	T	T	R
	U		U						L			R
v_c, x				176	283		155					
t_c, x				4.13	6.43		6.23					
t_f, x				2.23	3.53		3.33					
c_p, x				1394	705		888					

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	NR	SR	WR	EB
-------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$ Lane Width, $w$ Walking Speed, $S_p$ Pedestrian Blockage Factor, $f_{pb}$	0	0	0	
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$ Major L-Shared Probability Queue-free State, $p^*_{0,j}$			176 1394 1.000 1394 0.984 0.983	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$		155 888 1.000 888 0.990		
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Shared L/U Capacity, $c_{SH}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Major L, Minor T Adjusted Impedance Factor, $p''$ Major L, Minor T Impedance Factor, $p'$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$		283 705 1.000  0.983 693		

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES											
Approach	WestBound						EastBound				
Movement	7	8	9	10	11	12					
Lane Configuration		LR									
Shared Flow Rate, $v_y$		51									
Movement Capacity, $c_{m,x}$	693		888								
Shared Capacity, $c_{SH}$		720									

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS																
Approach	NB				SB				WestBound				EastBound			
Movement	1U	1	4U	5	4	7	8	9	10	11	12					
Flow Rate					23	42		9								
Movement Capacity					1394	693		888								
Lane Configuration					LT		LR									
Shared Capacity							720									
Control Delay					7.6		10.4									

CONTROL DELAY TO RANK 1 MOVEMENTS												
Approach	NB						SB					
Movement	2						5					
Number of Major Street Through Lanes, $N$	1						1					
Proportion of Rank 1 vehicles not blocked, $p^*_{0,j}$							0.983					
Delay to Major Left-turning Vehicles, $d_{MLT}$							7.6					
Major Street Through Vehicles in Shared Lane, $v_{i1}$							83					
Major Street Turning Vehicles in Shared Lane, $v_{i2}$							23					
Saturation Flow Rate for Major Street Through, $s_{i1}$	1800						1800					



Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

1500

0.1

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement	1U	NB 1	4U	SB 4	7	WestBound 8	9	10	EastBound 11	12
Lane Configuration				LT		LR				
Flow Rate				23		51				
Lane Capacity				1394		720				
v/c				0.02		0.07				
95% Queue Length				0.0		0.2				
Control Delay				7.6		10.4				
LOS				A		B				
Approach Delay				1.8		10.4				
Approach LOS						B				
Intersection Delay		2.2								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 4:27:07 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_PM\_NO BUILD\_Hillisdale-Old State-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Hillisdale Rd / Old State  
 Major Street Direction: North-South  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: Old State Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound				SouthBound			
Approach Movement	1U	2	3	4U	4	5	6	
	U	L	T	R	U	L	T	R
Volume			126	30		22	78	
Peak Hour Factor, PHF					0.92			
Hourly Flow Rate, HFR			137	33		24	85	
Percent Heavy Vehicles						3		
Number of Lanes	0	0	1	0	0	0	1	0
Lane Configuration				TR		LT		
Median Type					Undivided			
Median Storage								
RT channelized?								
Left-Turn Lane Storage								
Upstream Signal?					Not Present			

Minor Street:	WestBound			EastBound		
Approach Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	40		8			
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR	43		9			
Percent Heavy Vehicles	3		3			
Number of Lanes	0	1	0	0	0	0
Lane Configuration		LR				
RT channelized?						
Flared Approach   Storage	No					
Percent Grade		0				

Pedestrian Volumes and Adjustments

Approach Movement	NB	SB	WB	EB
	13	14	15	16
Flow (ped/hr)	0	0	0	
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f <sub>pb</sub>				

Delay, Queue Length, and Level of Service

Approach Movement	1U	NB	4U	SB	7	WestBound	8	9	10	EastBound	11	12
Lane Configuration				LT		LR						
Flow Rate				24		52						
Lane Capacity				1402		717						
v/c				0.02		0.07						
95% Queue Length				0.1		0.2						
Control Delay				7.6		10.4						
LOS				A		B						
Approach Delay				1.8		10.4						
Approach LOS						B						
Intersection Delay	2.2											

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound				SouthBound			
Approach Priority Movement	1U	2	3	4U	4	5	6	
	U	L	T	R	U	L	T	R
Minor Street:								

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement	1U	NorthBound				4U	SouthBound		
	U	1	2	3		U	4	5	6
		L	T	R			L	T	R
Volume, V_x			126	30			22	78	
Flow Rate, v_x			137	33			24	85	

Minor Street:									
Approach Movement		WestBound				EastBound			
		7	8	9			10	11	12
		L	T	R			L	T	R
Volume, V_x		40		8					
Flow Rate, v_x		43		9					

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement	1U	NorthBound				4U	SouthBound		
	U	1	2	3		U	4	5	6
		L	T	R			L	T	R
Flow Rate, v_x			137	33			24	85	
Conflicting Flow, v_c, x							170		

Minor Street:									
Approach Movement		WestBound				EastBound			
		7	8	9			10	11	12
		L	T	R			L	T	R
Flow Rate, v_x		43		9					
Conflicting Flow, v_c, x		286		153					

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS											
Approach Movement		NB		SB		WestBound			EastBound		
	1U	1		4U	4	7	8	9	10	11	12
	U	L		U	L	L	T	R	L	T	R
t_c, base											
Single Stage					4.1	7.1		6.2			
Stage I											
Stage II											
t_c, HV					1.0	1.0		1.0			
P_HV					0.03	0.03		0.03			
t_c, G						0.2		0.1			
G						0		0			
t_3, LT					0.0	0.7		0.0			
t_c											
Single Stage					4.13	6.43		6.23			
Stage I											
Stage II											

FOLLOW-UP HEADWAYS											
Approach Movement		NB		SB		WestBound			EastBound		
	1U	1		4U	4	7	8	9	10	11	12
	U	L		U	L	L	T	R	L	T	R
t_f, base					2.2	3.5		3.3			
t_f, HV					0.9	0.9		0.9			
P_HV					0.03	0.03		0.03			
t_f					2.23	3.53		3.33			

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT											
Approach Movement		NB		SB		WestBound			EastBound		
	1U	1		4U	4	7	8	9	10	11	12
	U	L		U	L	L	T	R	L	T	R
v_c, x					170	286		153			
t_c, x					4.13	6.43		6.23			
t_f, x					2.23	3.53		3.33			
c_p, x					1402	702		890			

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach				NR	SR	WR	EB
-------------------------------	--	--	--	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$			170	
Potential Capacity, $c_{p,x}$			1402	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Movement Capacity, $c_{m,x}$			1402	
Probability of Queue-free State, $p_{0,j}$			0.983	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$			0.982	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		153		
Potential Capacity, $c_{p,x}$		890		
Pedestrian Impedance Factor, $p_{p,x}$		1.000		
Movement Capacity, $c_{m,x}$		890		
Probability of Queue-free State, $p_{0,j}$		0.990		
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$		286		
Potential Capacity, $c_{p,x}$		702		
Pedestrian Impedance Factor, $p_{p,x}$		1.000		
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$		0.982		
Movement Capacity, $c_{m,x}$		690		

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES											
Approach	WestBound				EastBound						
Movement	7	8	9	10	11	12					
Lane Configuration		LR									
Shared Flow Rate, $v_y$		52									
Movement Capacity, $c_{m,x}$	690		890								
Shared Capacity, $c_{SH}$		717									

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS												
Approach	NB				SB							
	1U	1	4U	4	7	8	9	10	11	12		
Movement					WestBound			EastBound				
					LR							
Flow Rate							9					
Movement Capacity							890					
Lane Configuration					LR							
Shared Capacity							717					
Control Delay				7.6			10.4					

CONTROL DELAY TO RANK 1 MOVEMENTS												
Approach					NB				SB			
Movement					2				5			
Number of Major Street Through Lanes, $N$					1				1			
Proportion of Rank 1 vehicles not blocked, $p^*_{0,j}$									0.982			
Delay to Major Left-turning Vehicles, $d_{MLT}$									7.6			
Major Street Through Vehicles in Shared Lane, $v_{i1}$									85			
Major Street Turning Vehicles in Shared Lane, $v_{i2}$									24			
Saturation Flow Rate for Major Street Through, $s_{i1}$					1800				1800			

Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

1500

0.1

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement	1U	NB 1	4U	SB 4	7	WestBound 8	9	10	EastBound 11	12
Lane Configuration				LT		LR				
Flow Rate				24		52				
Lane Capacity				1402		717				
v/c				0.02		0.07				
95% Queue Length				0.1		0.2				
Control Delay				7.6		10.4				
LOS				A		B				
Approach Delay				1.8		10.4				
Approach LOS						B				
Intersection Delay		2.2								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 4:29:43 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis  
2045\_AM\_NO BUILD\_Radio-Walnut-x. xtw

File Name: 2045\_AM\_NO BUILD\_Radio-Walnut-x. xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Radio Ave / Walnut Rd  
 Major Street Direction: East-West  
 East/West Street Name: Radio Ave  
 North/South Street Name: Walnut Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	1U U	EastBound 1 L	2 T	3 R		4U U	WestBound 4 L	5 T	6 R
Volume		3	3	1			4	1	1
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		3	3	1			4	1	1
Percent Heavy Vehicles		3					3		
Number of Lanes	0	0	1	0		0	0	1	0
Lane Configuration			LTR					LTR	
Median Type					Undivided				
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street: Approach Movement		NorthBound 7 L	8 T	9 R			SouthBound 10 L	11 T	12 R
Volume		9	16	1			1	4	5
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		10	17	1			1	4	5
Percent Heavy Vehicles		3	3	3			3	3	3
Number of Lanes		0	1	0			0	1	0
Lane Configuration			LTR					LTR	
RT channelized?									
Flared Approach   Storage		No					No		
Percent Grade			0					0	

Pedestrian Volumes and Adjustments

Approach Movement		EB 13	WB 14	NB 15	SB 16
Flow (ped/hr)		0	0	0	0
Lane Width (ft)					
Walking Speed (ft/sec)					
Pedestrian Blockage Factor, f_pb					

Delay, Queue Length, and Level of Service

Approach Movement	1U U	EB 1	4U 4	WB 4	7 7	NorthBound 8 LTR	9 9	10 10	SouthBound 11 LTR	12 12
Lane Configuration						LTR			LTR	
Flow Rate		3		4		28			11	
Lane Capacity		1614		1611		907			972	
v/c		0.00		0.00		0.03			0.01	
95% Queue Length		0.0		0.0		0.1			0.0	
Control Delay		7.2		7.2		9.1			8.7	
LOS		A		A		A			A	
Approach Delay		3.1		4.8		9.1			8.7	
Approach LOS						A			A	
Intersection Delay		7.6								

Step 1: MOVEMENT PRIORITIES

Major Street: Approach Priority Movement	1U U	EastBound 1 L	2 T	3 R		4U U	WestBound 4 L	5 T	6 R
Minor Street:									

Approach Priority Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x		3	3	1		4	1	1	
Flow Rate, v_x		3	3	1		4	1	1	

Minor Street: Approach Movement	NorthBound				SouthBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Volume, V_x		9	16	1		1	4	5
Flow Rate, v_x		10	17	1		1	4	5

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		3	3	1		4	1	1	
Conflicting Flow, v_c, x		2				4			

Minor Street: Approach Movement	NorthBound				SouthBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Flow Rate, v_x		10	17	1		1	4	5
Conflicting Flow, v_c, x		26	21	4		30	21	2

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_c, base											
Single Stage		4.1		4.1	7.1	6.5	6.2	7.1	6.5	6.2	
Stage I											
Stage II											
t_c, HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	
P_HV		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1	
G					0	0	0	0	0	0	
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
t_c											
Single Stage		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23	
Stage I											
Stage II											

FOLLOW-UP HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_f, base											
t_f, HV		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3	
P_HV		0.9		0.9	0.9	0.9	0.9	0.9	0.9	0.9	
t_f		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
c_p, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound		
					7 L	8 T	9 R	10 L	11 T	12 R
v_c, x		2		4	26	21	4	30	21	2
t_c, x		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23
t_f, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33
c_p, x		1614		1611	983	870	1077	976	870	1080

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	FR	WR	NR	SB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate v_x Lane Width, w Walking Speed, S_p Pedestrian Blockage Factor, f_pb	0	0	0	0
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j Major L-Shared Probability Queue-free State, p*_0, j		2 1614 1.000 1614 0.998 0.998	4 1611 1.000 1611 0.997 0.997	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j		4 1077 1.000 1077 0.999	2 1080 1.000 1080 0.995	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Capacity Adjustment Factor, f_x Movement Capacity, c_m, x Shared L/U Capacity, c_SH Probability of Queue-free State, p_0, j				
Minor-Street Through Movements		8	11	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Capacity Adjustment Factor, f_x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j		21 870 1.000 0.995 866 0.980	21 870 1.000 0.995 866 0.995	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Major L, Minor T Adjusted Impedance Factor, p" Major L, Minor T Impedance Factor, p' Capacity Adjustment Factor, f_x Movement Capacity, c_m, x		26 983 1.000 0.990 0.993 0.988 970	30 976 1.000 0.975 0.981 0.980 957	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	NorthBound			SouthBound		
Movement	7	8	9	10	11	12
Lane Configuration		LTR			LTR	
Shared Flow Rate, v_y		28			11	
Movement Capacity, c_m, x	970	866	1077	957	866	1080
Shared Capacity, c_SH		907			972	

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		3		4	10	17	1	1	4	5
Movement Capacity		1614		1611	970	866	1077	957	866	1080
Lane Configuration						LTR			LTR	
Shared Capacity						907			972	
Control Delay		7.2		7.2		9.1			8.7	

CONTROL DELAY TO RANK 1 MOVEMENTS		
Approach	EB	WB
Movement	2	5
Number of Major Street Through Lanes, N	1	1
Proportion of Rank 1 vehicles not blocked, p*_0, j	0.998	0.997
Delay to Major Left-turning Vehicles, d_MLT	7.2	7.2
Major Street Through Vehicles in Shared Lane, v_i1	3	1
Major Street Turning Vehicles in Shared Lane, v_i2	4	5
Saturation Flow Rate for Major Street Through, s_i1	1800	1800



Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

1500  
 0.0

1500  
 0.0

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement Lane Configuration	1U	EB 1	4U	WB 4	7	NorthBound 8	9	10	SouthBound 11	12
						LTR			LTR	
Flow Rate		3		4		28			11	
Lane Capacity		1614		1611		907			972	
v/c		0.00		0.00		0.03			0.01	
95% Queue Length		0.0		0.0		0.1			0.0	
Control Delay		7.2		7.2		9.1			8.7	
LOS		A		A		A			A	
Approach Delay		3.1		4.8		9.1			8.7	
Approach LOS						A			A	
Intersection Delay		7.6								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 4:31:31 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_PM\_NO BUILD\_Radio-Walnut-x. xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Radio Ave / Walnut Rd  
 Major Street Direction: East-West  
 East/West Street Name: Radio Ave  
 North/South Street Name: Walnut Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	1U U	EastBound 1 L 2 T 3 R	3 R	4U U	WestBound 4 L 5 T 6 R
Volume		1	5	4	0 5 0
Peak Hour Factor, PHF				0.92	
Hourly Flow Rate, HFR		1	5	4	0 5 0
Percent Heavy Vehicles		3			3 5 0
Number of Lanes	0	0	1	0	0 1 0
Lane Configuration			LTR		LTR
Median Type				Undivided	
Median Storage					
RT channelized?					
Left-Turn Lane Storage					
Upstream Signal?				Not Present	

Minor Street: Approach Movement	NorthBound 7 L 8 T 9 R	9 R	SouthBound 10 L 11 T 12 R
Volume	8	13	1
Peak Hour Factor, PHF			0.92
Hourly Flow Rate, HFR	9	14	1
Percent Heavy Vehicles	3	3	3
Number of Lanes	0	1	0
Lane Configuration		LTR	LTR
RT channelized?			
Flared Approach   Storage	No		No
Percent Grade		0	0

Pedestrian Volumes and Adjustments

Approach Movement	EB 13	WB 14	NB 15	SB 16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach Movement	1U U	EB 1	4U U	4 R	7 R	NorthBound 8 LTR	9 R	10 R	SouthBound 11 LTR	12 R
Lane Configuration						LTR			LTR	
Flow Rate		1				24			16	
Lane Capacity		1609				1603			891	
v/c		0.00				0.03			0.02	
95% Queue Length		0.0				0.1			0.1	
Control Delay		7.2				7.2			9.1	
LOS		A				A			A	
Approach Delay		0.7				9.0			9.1	
Approach LOS						A			A	
Intersection Delay		6.6								

Step 1: MOVEMENT PRIORITIES

Major Street: Approach Priority Movement	1U U	EastBound 1 L 2 T 3 R	3 R	4U U	WestBound 4 L 5 T 6 R
Minor Street:					

Approach Priority Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x		1	5	4			0	5	0
Flow Rate, v_x		1	5	4			0	5	0

Minor Street: Approach Movement	NorthBound				SouthBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Volume, V_x		8	13	1		1	13	1
Flow Rate, v_x		9	14	1		1	14	1

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		1	5	4			0	5	0
Conflicting Flow, v_c, x		5					10		

Minor Street: Approach Movement	NorthBound				SouthBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Flow Rate, v_x		9	14	1		1	14	1
Conflicting Flow, v_c, x		23	15	8		23	17	5

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_c, base											
Single Stage		4.1		4.1	7.1	6.5	6.2	7.1	6.5	6.2	
Stage I											
Stage II											
t_c, HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	
P_HV		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1	
G					0	0	0	0	0	0	
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
t_c											
Single Stage		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23	
Stage I											
Stage II											

FOLLOW-UP HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_f, base											
t_f, HV		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3	
P_HV		0.9		0.9	0.9	0.9	0.9	0.9	0.9	0.9	
t_f		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
c_p, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound		
					7 L	8 T	9 R	10 L	11 T	12 R
v_c, x		5		10	23	15	8	23	17	5
t_c, x		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23
t_f, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33
c_p, x		1609		1603	987	877	1072	987	875	1075

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	FR	WR	NR	SB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate v_x Lane Width, w Walking Speed, S_p Pedestrian Blockage Factor, f_pb	0	0	0	0
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j Major L-Shared Probability Queue-free State, p*_0, j		5 1609 1.000 1609 0.999 0.999	10 1603 1.000 1603 1.000 1.000	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j		8 1072 1.000 1072 0.999	5 1075 1.000 1075 0.999	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Capacity Adjustment Factor, f_x Movement Capacity, c_m, x Shared L/U Capacity, c_SH Probability of Queue-free State, p_0, j				
Minor-Street Through Movements		8	11	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Capacity Adjustment Factor, f_x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j		15 877 1.000 0.999 876 0.984	17 875 1.000 0.999 874 0.984	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Major L, Minor T Adjusted Impedance Factor, p" Major L, Minor T Impedance Factor, p' Capacity Adjustment Factor, f_x Movement Capacity, c_m, x		23 987 1.000 0.983 0.987 0.986 973	23 987 1.000 0.983 0.987 0.986 973	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	NorthBound			SouthBound		
Movement	7	8	9	10	11	12
Lane Configuration		LTR			LTR	
Shared Flow Rate, v_y		24			16	
Movement Capacity, c_m, x	973	876	1072	973	874	1075
Shared Capacity, c_SH		917			891	

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		1		0	9	14	1	1	14	1
Movement Capacity		1609		1603	973	876	1072	973	874	1075
Lane Configuration						LTR			LTR	
Shared Capacity						917			891	
Control Delay		7.2		7.2		9.0			9.1	

CONTROL DELAY TO RANK 1 MOVEMENTS		
Approach	EB	WB
Movement	2	5
Number of Major Street Through Lanes, N	1	1
Proportion of Rank 1 vehicles not blocked, p*_0, j	0.999	1.000
Delay to Major Left-turning Vehicles, d_MLT	7.2	7.2
Major Street Through Vehicles in Shared Lane, v_i1	5	5
Major Street Turning Vehicles in Shared Lane, v_i2	5	0
Saturation Flow Rate for Major Street Through, s_i1	1800	1800

Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

1500  
 0.0

1500  
 0.0

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement Lane Configuration	1U	EB 1	4U	WB 4	7	NorthBound 8	9	10	SouthBound 11	12
						LTR			LTR	
Flow Rate		1		0		24			16	
Lane Capacity		1609		1603		917			891	
v/c		0.00		0.00		0.03			0.02	
95% Queue Length		0.0		0.0		0.1			0.1	
Control Delay		7.2		7.2		9.0			9.1	
LOS		A		A		A			A	
Approach Delay		0.7		0.0		9.0			9.1	
Approach LOS						A			A	
Intersection Delay		6.6								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 4:33:44 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis  
2045\_AM\_NO BUILD\_Radio-US 41-x.xtw

File Name: 2045\_AM\_NO BUILD\_Radio-US 41-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Radio Ave / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Radio Ave  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound						SouthBound		
Approach	1U	2	3	4U	4	5	6		
Movement	U	L	T	R	U	L	T	R	
Volume			988	5	0	0	1071		
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR			1074	5		0	1164		
Percent Heavy Vehicles					0	13			
Number of Lanes	0	0	2	0	0	1	2	0	
Lane Configuration			T	TR		L	T		
Median Type	Undivided								
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?	Not Present								

Minor Street:	WestBound			EastBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume	5		16			
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR	5		17			
Percent Heavy Vehicles	3		3			
Number of Lanes	1	0	1	0	0	0
Lane Configuration	L		R			
RT channelized?			No			
Flared Approach   Storage						
Percent Grade		0				

Pedestrian Volumes and Adjustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	NB	SB	WestBound	EastBound
Movement	1	4U	7	10
Lane Configuration	1U	4U	7	10
	1	4U	7	10
	1	4U	7	10
Flow Rate		0	5	17
Lane Capacity		581	88	484
v/c		0.00	0.06	0.04
95% Queue Length		0.0	0.2	0.1
Control Delay		11.2	48.8	12.7
LOS		B	E	B
Approach Delay		0.0	21.3	
Approach LOS			C	
Intersection Delay	0.2			

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound						SouthBound		
Approach	1U	2	3	4U	4	5	6		
Priority	U	L	T	R	U	L	T	R	
Movement	U	L	T	R	U	L	T	R	
Minor Street:									

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:								
Approach Movement	1U	NorthBound			4U	SouthBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Volume, V_x		988			0	1071		
Flow Rate, v_x		1074				1164		

Minor Street:								
Approach Movement	WestBound				EastBound			
		7	8	9		10	11	12
		L	T	R		L	T	R
Volume, V_x		5				16		
Flow Rate, v_x		5				17		

Step 3: CONFLICTING FLOW RATES

Major Street:								
Approach Movement	1U	NorthBound			4U	SouthBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Flow Rate, v_x		1074			5	1164		
Conflicting Flow, v_c, x						1079		

Minor Street:								
Approach Movement	WestBound				EastBound			
		7	8	9		10	11	12
		L	T	R		L	T	R
Flow Rate, v_x		5				17		
Conflicting Flow, v_c, x		1659				540		

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_c, base										
Single Stage				4.1	7.5		6.9			
Stage I										
Stage II										
t_c, HV				2.0	2.0		2.0			
P_HV				0.13	0.03		0.03			
t_c, G					0.2		0.1			
G					0		0			
t_3, LT				0.0	0.7		0.0			
t_c										
Single Stage				4.36	6.86		6.96			
Stage I										
Stage II										

FOLLOW-UP HEADWAYS										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_f, base				2.2	3.5		3.3			
t_f, HV				1.0	1.0		1.0			
P_HV				0.13	0.03		0.03			
t_f				2.33	3.53		3.33			

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
v_c, x				1079	1659		540			
t_c, x				4.36	6.86		6.96			
t_f, x				2.33	3.53		3.33			
c_p, x				581	88		484			

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$			1079	
Potential Capacity, $c_{p,x}$			581	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Movement Capacity, $c_{m,x}$			581	
Probability of Queue-free State, $p_{0,j}$			1.000	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		540		
Potential Capacity, $c_{p,x}$		484		
Pedestrian Impedance Factor, $p_{p,x}$		1.000		
Movement Capacity, $c_{m,x}$		484		
Probability of Queue-free State, $p_{0,j}$		0.964		
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$		1659		
Potential Capacity, $c_{p,x}$		88		
Pedestrian Impedance Factor, $p_{p,x}$		1.000		
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$		1.000		
Movement Capacity, $c_{m,x}$		88		

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES										
Approach	WestBound				EastBound					
Movement	7	8	9	10	11	12				
Lane Configuration	L		R							
Shared Flow Rate, $v_y$	5		17							
Movement Capacity, $c_{m,x}$	88		484							
Shared Capacity, $c_{SH}$										

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate	0	5					17			
Movement Capacity	581	88					484			
Lane Configuration	L	L					R			
Shared Capacity										
Control Delay	11.2	48.8					12.7			

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate	0	5					17			
Lane Capacity	581	88					484			
$v/c$	0.00	0.06					0.04			
95% Queue Length	0.0	0.2					0.1			
Control Delay	11.2	48.8					12.7			



LOS		B	E	B
Approach Delay		0.0		21.3
Approach LOS				C
Intersection Delay	0.2			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 4:36:41 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_PM\_NO BUILD\_Radio-US 41-x. xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Radio Ave / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Radio Ave  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound						SouthBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Volume			956	3	0	3	1276		
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR			1039	3		3	1387		
Percent Heavy Vehicles					0	13			
Number of Lanes	0	0	2	0	0	1	2	0	
Lane Configuration			T	TR		L	T		
Median Type					Undivided				
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street:	WestBound			EastBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume	1		5			
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR	1		5			
Percent Heavy Vehicles	3		3			
Number of Lanes	1	0	1	0	0	0
Lane Configuration	L		R			
RT channelized?			No			
Flared Approach   Storage						
Percent Grade		0				

Pedestrian Volumes and Adjustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	NB	SB	WestBound			EastBound			
Movement	1U	4U	4	7	8	9	10	11	12
Lane Configuration		L	L	L	R				
Flow Rate		3	1		5				
Lane Capacity		601	77		497				
v/c		0.01	0.01		0.01				
95% Queue Length		0.0	0.0		0.0				
Control Delay		11.0	52.6		12.3				
LOS		B	F		B				
Approach Delay		0.0		19.0					
Approach LOS				C					
Intersection Delay	0.1								

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound						SouthBound		
Approach	1U	1	2	3	4U	4	5	6	
Priority	U	L	T	R	U	L	T	R	

Minor Street:

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:							
Approach Movement	NorthBound				SouthBound		
	1U	2	3		4U	5	6
	U	L	T		U	L	T
Volume, V_x		956	3		0	3	1276
Flow Rate, v_x		1039	3			3	1387

Minor Street:							
Approach Movement	WestBound				EastBound		
			9				12
			R				R
Volume, V_x			5				
Flow Rate, v_x			5				

Step 3: CONFLICTING FLOW RATES

Major Street:							
Approach Movement	NorthBound				SouthBound		
	1U	2	3		4U	5	6
	U	L	T		U	L	T
Flow Rate, v_x		1039	3			3	1387
Conflicting Flow, v_c, x						1042	

Minor Street:							
Approach Movement	WestBound				EastBound		
			9				12
			R				R
Flow Rate, v_x			5				
Conflicting Flow, v_c, x		1741	521				

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_c, base										
Single Stage				4.1	7.5		6.9			
Stage I										
Stage II										
t_c, HV				2.0	2.0		2.0			
P_HV				0.13	0.03		0.03			
t_c, G					0.2		0.1			
G					0		0			
t_3, LT				0.0	0.7		0.0			
t_c										
Single Stage				4.36	6.86		6.96			
Stage I										
Stage II										

FOLLOW-UP HEADWAYS										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_f, base				2.2	3.5		3.3			
t_f, HV				1.0	1.0		1.0			
P_HV				0.13	0.03		0.03			
t_f				2.33	3.53		3.33			

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
v_c, x				1042	1741		521			
t_c, x				4.36	6.86		6.96			
t_f, x				2.33	3.53		3.33			
c_p, x				601	77		497			

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$			1042	
Potential Capacity, $c_{p,x}$			601	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Movement Capacity, $c_{m,x}$			601	
Probability of Queue-free State, $p_{0,j}$			0.995	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		521		
Potential Capacity, $c_{p,x}$		497		
Pedestrian Impedance Factor, $p_{p,x}$		1.000		
Movement Capacity, $c_{m,x}$		497		
Probability of Queue-free State, $p_{0,j}$		0.989		
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$		1741		
Potential Capacity, $c_{p,x}$		77		
Pedestrian Impedance Factor, $p_{p,x}$		1.000		
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$		0.995		
Movement Capacity, $c_{m,x}$		77		

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES										
Approach	WestBound							EastBound		
Movement	7	8	9		10	11	12			
Lane Configuration	L		R							
Shared Flow Rate, $v_y$	1		5							
Movement Capacity, $c_{m,x}$	77		497							
Shared Capacity, $c_{SH}$										

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate	3	1					5			
Movement Capacity	601	77					497			
Lane Configuration	L	L					R			
Shared Capacity										
Control Delay	11.0	52.6					12.3			

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration										
Flow Rate	3	1					5			
Lane Capacity	601	77					497			
$v/c$	0.01	0.01					0.01			
95% Queue Length	0.0	0.0					0.0			
Control Delay	11.0	52.6					12.3			

LOS		B	F	B
Approach Delay		0.0		19.0
Approach LOS				C
Intersection Delay	0.1			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 4:38:44 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_AM\_NO BUILD\_Campbell - Walnut-x. xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Campbell Rd / Walnut Rd  
 Major Street Direction: East-West  
 East/West Street Name: Campbell Road  
 North/South Street Name: Walnut Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Volume			5	3		1	16		
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR			5	3		1	17		
Percent Heavy Vehicles						3			
Number of Lanes	0	0	1	1	0	1	1	0	
Lane Configuration			T	R		L	T		
Median Type	Undivided								
Median Storage									
RT channelized?	No								
Left-Turn Lane Storage									
Upstream Signal?	Not Present								

Minor Street:	NorthBound			SouthBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume	17		3			
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR	18		3			
Percent Heavy Vehicles	3		3			
Number of Lanes	1	0	1	0	0	0
Lane Configuration	L		R			
RT channelized?			No			
Flared Approach   Storage						
Percent Grade		0				

Pedestrian Volumes and Adjustments

Approach	EB	WB	NB	SB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	EB	WB	NorthBound			SouthBound			
Movement	1	4U	4	7	8	9	10	11	12
Lane Configuration			L	L		R			
Flow Rate			1	18		3			
Lane Capacity			1605	987		1075			
v/c			0.00	0.02		0.00			
95% Queue Length			0.0	0.1		0.0			
Control Delay			7.2	8.7		8.4			
LOS			A	A		A			
Approach Delay			0.4		8.7				
Approach LOS					A				
Intersection Delay	4.0								

Step 1: MOVEMENT PRIORITIES

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Minor Street:									

Approach Priority Movement	NorthBound				SouthBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement	1U	EastBound				4U	WestBound		
	U	1	2	3		U	4	5	6
		L	T	R			L	T	R
Volume, V_x			5	3			1	16	
Flow Rate, v_x			5	3			1	17	

Minor Street:								
Approach Movement	NorthBound				SouthBound			
	7	8	9		10	11	12	
	L	T	R		L	T	R	
Volume, V_x	17		3					
Flow Rate, v_x	18		3					

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement	1U	EastBound				4U	WestBound		
	U	1	2	3		U	4	5	6
		L	T	R			L	T	R
Flow Rate, v_x			5	3			1	17	
Conflicting Flow, v_c, x							9		

Minor Street:								
Approach Movement	NorthBound				SouthBound			
	7	8	9		10	11	12	
	L	T	R		L	T	R	
Flow Rate, v_x	18		3					
Conflicting Flow, v_c, x	25		5					

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS														
Approach Movement	1U	EB	1	4U	WB	4	7	NorthBound	8	9	10	SouthBound	11	12
	U		L	U		L	L		T	R	L		T	R
t_c, base														
Single Stage							4.1		7.1				6.2	
Stage I														
Stage II														
t_c, HV							1.0		1.0				1.0	
P_HV							0.03		0.03				0.03	
t_c, G									0.2				0.1	
G									0				0	
t_3, LT							0.0		0.7				0.0	
t_c														
Single Stage							4.13		6.43				6.23	
Stage I														
Stage II														

FOLLOW-UP HEADWAYS														
Approach Movement	1U	EB	1	4U	WB	4	7	NorthBound	8	9	10	SouthBound	11	12
	U		L	U		L	L		T	R	L		T	R
t_f, base							2.2		3.5				3.3	
t_f, HV							0.9		0.9				0.9	
P_HV							0.03		0.03				0.03	
t_f							2.23		3.53				3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT														
Approach Movement	1U	EB	1	4U	WB	4	7	NorthBound	8	9	10	SouthBound	11	12
	U		L	U		L	L		T	R	L		T	R
v_c, x							9		25				5	
t_c, x							4.13		6.43				6.23	
t_f, x							2.23		3.53				3.33	
c_p, x							1605		988				1075	

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	FR	WR	NR	SB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$ Lane Width, $w$ Walking Speed, $S_p$ Pedestrian Blockage Factor, $f_{pb}$	0	0	0	
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$ Major L-Shared Probability Queue-free State, $p^*_{0,j}$			9 1605 1.000 1605 0.999	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$		5 1075 1.000 1075 0.997		
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Shared L/U Capacity, $c_{SH}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Major L, Minor T Adjusted Impedance Factor, $p''$ Major L, Minor T Impedance Factor, $p'$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$		25 988 1.000   0.999 987		

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES										
Approach	NorthBound					SouthBound				
Movement	7	8	9		10	11	12			
Lane Configuration	L		R							
Shared Flow Rate, $v_y$	18		3							
Movement Capacity, $c_{m,x}$	987		1075							
Shared Capacity, $c_{SH}$										

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate	1				18		3			
Movement Capacity	1605				987		1075			
Lane Configuration	L				L		R			
Shared Capacity										
Control Delay	7.2				8.7		8.4			

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration				L	L		R			
Flow Rate	1				18		3			
Lane Capacity	1605				987		1075			
$v/c$	0.00				0.02		0.00			
95% Queue Length	0.0				0.1		0.0			
Control Delay	7.2				8.7		8.4			



LOS		A	A		A
Approach Delay		0.4		8.7	
Approach LOS				A	
Intersection Delay	4.0				

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 4:41:18 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_PM\_NO BUILD\_Campbell - Walnut-x. xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Campbell Rd / Walnut Rd  
 Major Street Direction: East-West  
 East/West Street Name: Campbell Road  
 North/South Street Name: Walnut Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Volume			19	19		1	6		
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR			21	21		1	7		
Percent Heavy Vehicles						3			
Number of Lanes	0	0	1	1	0	1	1	0	
Lane Configuration			T	R		L	T		
Median Type	Undivided								
Median Storage									
RT channelized?	No								
Left-Turn Lane Storage									
Upstream Signal?	Not Present								

Minor Street:	NorthBound			SouthBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume	10		0			
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR	11		0			
Percent Heavy Vehicles	3		3			
Number of Lanes	1	0	1	0	0	0
Lane Configuration	L		R			
RT channelized?			No			
Flared Approach   Storage						
Percent Grade		0				

Pedestrian Volumes and Adjustments

Approach	EB	WB	NB	SB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	EB	WB	NorthBound			SouthBound			
Movement	1U	4U	4	7	8	9	10	11	12
Lane Configuration			L	L		R			
Flow Rate			1	11		0			
Lane Capacity			1561	982		1054			
v/c			0.00	0.01		0.00			
95% Queue Length			0.0	0.0		0.0			
Control Delay			7.3	8.7		8.4			
LOS			A	A		A			
Approach Delay			1.0		8.7				
Approach LOS					A				
Intersection Delay	1.7								

Step 1: MOVEMENT PRIORITIES

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Minor Street:									

Approach Priority Movement	NorthBound				SouthBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:								
Approach Movement	1U	EastBound			4U	WestBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Volume, V_x		19	19			1	6	
Flow Rate, v_x		21	21			1	7	

Minor Street:							
Approach Movement	NorthBound				SouthBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Volume, V_x	10		0				
Flow Rate, v_x	11		0				

Step 3: CONFLICTING FLOW RATES

Major Street:								
Approach Movement	1U	EastBound			4U	WestBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Flow Rate, v_x		21	21			1	7	
Conflicting Flow, v_c, x						41		

Minor Street:							
Approach Movement	NorthBound				SouthBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Flow Rate, v_x	11		0				
Conflicting Flow, v_c, x	29		21				

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS														
Approach Movement	1U	EB	1	4U	WB	4	7	NorthBound	8	9	10	SouthBound	11	12
	U		L	U		L	L		T	R	L		T	R
t_c, base														
Single Stage						4.1	7.1			6.2				
Stage I														
Stage II														
t_c, HV						1.0	1.0			1.0				
P_HV						0.03	0.03			0.03				
t_c, G							0.2			0.1				
G							0			0				
t_3, LT						0.0	0.7			0.0				
t_c														
Single Stage						4.13	6.43			6.23				
Stage I														
Stage II														

FOLLOW-UP HEADWAYS														
Approach Movement	1U	EB	1	4U	WB	4	7	NorthBound	8	9	10	SouthBound	11	12
	U		L	U		L	L		T	R	L		T	R
t_f, base						2.2	3.5			3.3				
t_f, HV						0.9	0.9			0.9				
P_HV						0.03	0.03			0.03				
t_f						2.23	3.53			3.33				

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT														
Approach Movement	1U	EB	1	4U	WB	4	7	NorthBound	8	9	10	SouthBound	11	12
	U		L	U		L	L		T	R	L		T	R
v_c, x						41	29			21				
t_c, x						4.13	6.43			6.23				
t_f, x						2.23	3.53			3.33				
c_p, x						1561	983			1054				

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	FR	WR	NR	SB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$ Lane Width, $w$ Walking Speed, $S_p$ Pedestrian Blockage Factor, $f_{pb}$	0	0	0	
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$ Major L-Shared Probability Queue-free State, $p^*_{0,j}$			41 1561 1.000 1561 0.999	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$		21 1054 1.000 1054 1.000		
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Shared L/U Capacity, $c_{SH}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Major L, Minor T Adjusted Impedance Factor, $p''$ Major L, Minor T Impedance Factor, $p'$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$		29 983 1.000   0.999 982		

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES										
Approach	NorthBound							SouthBound		
Movement	7	8	9		10	11	12			
Lane Configuration	L		R							
Shared Flow Rate, $v_y$	11		0							
Movement Capacity, $c_{m,x}$	982		1054							
Shared Capacity, $c_{SH}$										

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate	1				11		0			
Movement Capacity	1561				982		1054			
Lane Configuration	L				L		R			
Shared Capacity										
Control Delay	7.3				8.7		8.4			

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration				L	L		R			
Flow Rate	1				11		0			
Lane Capacity	1561				982		1054			
$v/c$	0.00				0.01		0.00			
95% Queue Length	0.0				0.0		0.0			
Control Delay	7.3				8.7		8.4			

LOS		A	A		A
Approach Delay		1.0		8.7	
Approach LOS				A	
Intersection Delay	1.7				

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 4:42:55 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_AM\_NO BUILD\_Campbell-OSR-x. xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Campbell Rd / OSR  
 Major Street Direction: East-West  
 East/West Street Name: Campbell Road  
 North/South Street Name: Old State Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	1U U	EastBound 1 L	2 T	3 R	4U U	WestBound 4 L	5 T	6 R
Volume		48	32				14	22
Peak Hour Factor, PHF					0.92			
Hourly Flow Rate, HFR		52	35				15	24
Percent Heavy Vehicles		3						
Number of Lanes	0	1	1	0	0	0	1	1
Lane Configuration		L	T				T	R
Median Type					Undivided			
Median Storage								
RT channelized?								No
Left-Turn Lane Storage								
Upstream Signal?					Not Present			

Minor Street: Approach Movement	NorthBound 7 L	8 T	9 R	SouthBound 10 L	11 T	12 R
Volume				18		83
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR				20		90
Percent Heavy Vehicles				3		3
Number of Lanes	0	0	0	1	0	1
Lane Configuration				L		R
RT channelized?						No
Flared Approach   Storage					0	
Percent Grade						

Pedestrian Volumes and Adjustments

Approach Movement	EB 13	WB 14	NB 15	SB 16
Flow (ped/hr)	0	0		0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach Movement	1U U	EB 1 L	4U 4	WB 4	7 7	NorthBound 8	9	10 10	SouthBound 11	12 12
Lane Configuration		L						L		R
Flow Rate		52						20		90
Lane Capacity		1564						807		1061
v/c		0.03						0.02		0.09
95% Queue Length		0.1						0.1		0.3
Control Delay		7.4						9.6		8.7
LOS		A						A		A
Approach Delay		4.4							8.9	
Approach LOS									A	
Intersection Delay		5.8								

Step 1: MOVEMENT PRIORITIES

Major Street: Approach Priority Movement	1U U	EastBound 1 L	2 T	3 R	4U U	WestBound 4 L	5 T	6 R
Minor Street:								

Approach Priority Movement	NorthBound				SouthBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U	EastBound		3		4U	WestBound		6
	U	1	2	R		U	4	5	R
		L	T				L	T	
Volume, V_x		48	32				14	22	
Flow Rate, v_x		52	35				15	24	

Minor Street: Approach Movement	NorthBound				SouthBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Volume, V_x					18		83
Flow Rate, v_x					20		90

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U	EastBound		3		4U	WestBound		6
	U	1	2	R		U	4	5	R
		L	T				L	T	
Flow Rate, v_x		52	35				15	24	
Conflicting Flow, v_c, x		39							

Minor Street: Approach Movement	NorthBound				SouthBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Flow Rate, v_x					20		90
Conflicting Flow, v_c, x					154		15

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS												
Approach Movement	EB	1	WB	4	7	NorthBound	8	9	SouthBound	10	11	12
	1U	L	4U	L	L		T	R		L	T	R
	U		U									
t_c, base												
Single Stage		4.1								7.1		6.2
Stage I												
Stage II												
t_c, HV		1.0								1.0		1.0
P_HV		0.03								0.03		0.03
t_c, G										0.2		0.1
G										0		0
t_3, LT		0.0								0.7		0.0
t_c												
Single Stage		4.13								6.43		6.23
Stage I												
Stage II												

FOLLOW-UP HEADWAYS												
Approach Movement	EB	1	WB	4	7	NorthBound	8	9	SouthBound	10	11	12
	1U	L	4U	L	L		T	R		L	T	R
	U		U									
t_f, base												
t_f, HV		2.2								3.5		3.3
P_HV		0.9								0.9		0.9
t_f		0.03								0.03		0.03
		2.23								3.53		3.33

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT												
Approach Movement	EB	1	WB	4	7	NorthBound	8	9	SouthBound	10	11	12
	1U	L	4U	L	L		T	R		L	T	R
	U		U									
v_c, x		39								154		15
t_c, x		4.13								6.43		6.23
t_f, x		2.23								3.53		3.33
c_p, x		1564								835		1061

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	FR	WR	NR	SB
-------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0		0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		39		
Potential Capacity, $c_{p,x}$		1564		
Pedestrian Impedance Factor, $p_{p,x}$		1.000		
Movement Capacity, $c_{m,x}$		1564		
Probability of Queue-free State, $p_{0,j}$		0.967		
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$			15	
Potential Capacity, $c_{p,x}$			1061	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Movement Capacity, $c_{m,x}$			1061	
Probability of Queue-free State, $p_{0,j}$			0.915	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$			154	
Potential Capacity, $c_{p,x}$			835	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$			0.967	
Movement Capacity, $c_{m,x}$			807	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	NorthBound			SouthBound		
Movement	7	8	9	10	11	12
Lane Configuration				L		R
Shared Flow Rate, $v_y$				20		90
Movement Capacity, $c_{m,x}$				807		1061
Shared Capacity, $c_{SH}$						

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		52						20		90
Movement Capacity		1564						807		1061
Lane Configuration		L						L		R
Shared Capacity										
Control Delay		7.4						9.6		8.7

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration		L						L		R
Flow Rate		52						20		90
Lane Capacity		1564						807		1061
$v/c$		0.03						0.02		0.09
95% Queue Length		0.1						0.1		0.3
Control Delay		7.4						9.6		8.7



LOS	A	A		A
Approach Delay	4.4		8.9	
Approach LOS			A	
Intersection Delay	5.8			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 4:49:05 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_PM\_NO BUILD\_Campbell-OSR-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Campbell Rd / OSR  
 Major Street Direction: East-West  
 East/West Street Name: Campbell Road  
 North/South Street Name: Old State Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Volume		57	22				12	9	
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		62	24				13	10	
Percent Heavy Vehicles		3							
Number of Lanes	0	1	1	0	0	0	1	1	
Lane Configuration		L	T				T	R	
Median Type					Undivided				
Median Storage									
RT channelized?								No	
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street:	NorthBound			SouthBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume				34		43
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR				37		47
Percent Heavy Vehicles				3		3
Number of Lanes	0	0	0	1	0	1
Lane Configuration				L		R
RT channelized?						No
Flared Approach   Storage					0	
Percent Grade						

Pedestrian Volumes and Adjustments

Approach	EB	WB	NB	SB
Movement	13	14	15	16
Flow (ped/hr)	0	0		0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	EB	NorthBound			SouthBound			
Movement	1U	4U	7	8	9	10	11	12
Lane Configuration	L	L	T	T	T	L	T	R
Flow Rate	62					37		47
Lane Capacity	1586					795		1064
v/c	0.04					0.05		0.04
95% Queue Length	0.1					0.1		0.1
Control Delay	7.4					9.7		8.5
LOS	A					A		A
Approach Delay	5.3						9.1	
Approach LOS							A	
Intersection Delay	6.3							

Step 1: MOVEMENT PRIORITIES

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Priority									

Minor Street:

Approach Priority Movement	NorthBound				SouthBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:								
Approach Movement	1U	EastBound			4U	WestBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Volume, V_x		57	22			12	9	
Flow Rate, v_x		62	24			13	10	

Minor Street:								
Approach Movement	NorthBound				SouthBound			
	7	8	9		10	11	12	
	L	T	R		L	T	R	
Volume, V_x					34		43	
Flow Rate, v_x					37		47	

Step 3: CONFLICTING FLOW RATES

Major Street:								
Approach Movement	1U	EastBound			4U	WestBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Flow Rate, v_x		62	24			13	10	
Conflicting Flow, v_c, x		23						

Minor Street:								
Approach Movement	NorthBound				SouthBound			
	7	8	9		10	11	12	
	L	T	R		L	T	R	
Flow Rate, v_x					37		47	
Conflicting Flow, v_c, x					161		13	

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS														
Approach Movement	1U	EB	1	4U	WB	4	7	NorthBound	8	9	SouthBound	10	11	12
	U		L	U		L	L		T	R	L	T	R	
t_c, base			4.1								7.1		6.2	
Single Stage														
Stage I														
Stage II														
t_c, HV			1.0								1.0		1.0	
P_HV			0.03								0.03		0.03	
t_c, G											0.2		0.1	
G											0		0	
t_3, LT			0.0								0.7		0.0	
t_c														
Single Stage			4.13								6.43		6.23	
Stage I														
Stage II														

FOLLOW-UP HEADWAYS														
Approach Movement	1U	EB	1	4U	WB	4	7	NorthBound	8	9	SouthBound	10	11	12
	U		L	U		L	L		T	R	L	T	R	
t_f, base			2.2								3.5		3.3	
t_f, HV			0.9								0.9		0.9	
P_HV			0.03								0.03		0.03	
t_f			2.23								3.53		3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT														
Approach Movement	1U	EB	1	4U	WB	4	7	NorthBound	8	9	SouthBound	10	11	12
	U		L	U		L	L		T	R	L	T	R	
v_c, x			23								161		13	
t_c, x			4.13								6.43		6.23	
t_f, x			2.23								3.53		3.33	
c_p, x			1586								828		1064	

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	FR	WR	NR	SB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0		0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		23		
Potential Capacity, $c_{p,x}$		1586		
Pedestrian Impedance Factor, $p_{p,x}$		1.000		
Movement Capacity, $c_{m,x}$		1586		
Probability of Queue-free State, $p_{0,j}$		0.961		
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$			13	
Potential Capacity, $c_{p,x}$			1064	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Movement Capacity, $c_{m,x}$			1064	
Probability of Queue-free State, $p_{0,j}$			0.956	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$			161	
Potential Capacity, $c_{p,x}$			828	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$			0.961	
Movement Capacity, $c_{m,x}$			795	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	NorthBound			SouthBound		
Movement	7	8	9	10	11	12
Lane Configuration				L		R
Shared Flow Rate, $v_y$				37		47
Movement Capacity, $c_{m,x}$				795		1064
Shared Capacity, $c_{SH}$						

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		62						37		47
Movement Capacity		1586						795		1064
Lane Configuration		L						L		R
Shared Capacity										
Control Delay		7.4						9.7		8.5

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration		L						L		R
Flow Rate		62						37		47
Lane Capacity		1586						795		1064
$v/c$		0.04						0.05		0.04
95% Queue Length		0.1						0.1		0.1
Control Delay		7.4						9.7		8.5

LOS	A	A		A
Approach Delay	5.3		9.1	
Approach LOS			A	
Intersection Delay	6.3			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 4:50:57 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_AM\_NO BUILD\_OSR-41-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Campbell-Old St Rd/ US 41  
 Major Street Direction: North-South  
 East/West Street Name: Campbell/OSR  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R
Volume	0	0	1220	14		0	4	1148	48
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		0	1326	15			4	1248	52
Percent Heavy Vehicles	13	13				13	13		
Number of Lanes	0	1	2	0		0	1	2	1
Lane Configuration		L	T	TR			L	T	R
Median Type					Undivided				
Median Storage									
RT channelized?									No
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street: Approach Movement		WestBound 7 L	8 T	9 R		EastBound 10 L	11 T	12 R
Volume		25	19	6		89	32	8
Peak Hour Factor, PHF					0.92			
Hourly Flow Rate, HFR		27	21	7		97	35	9
Percent Heavy Vehicles		3	3	3		3	3	3
Number of Lanes		0	1	0		0	1	0
Lane Configuration			LTR				LTR	
RT channelized?								
Flared Approach   Storage		No				No		
Percent Grade			0				0	

Pedestrian Volumes and Adjustments

Approach Movement		NB 13	SB 14	WB 15	EB 16
Flow (ped/hr)		0	0	0	0
Lane Width (ft)					
Walking Speed (ft/sec)					
Pedestrian Blockage Factor, f_pb					

Delay, Queue Length, and Level of Service

Approach Movement	1U U	NB 1 L	4U L	7 L	8 LTR	9 L	10 L	EastBound 11 LTR	12 R
Lane Configuration									
Flow Rate		0	4		54			140	
Lane Capacity		473	455					10	
v/c		0.00	0.01					14.54	
95% Queue Length		0.0	0.0					19.1	
Control Delay		12.6	13.0					6846.6	
LOS		B	B					F	
Approach Delay		0.0	0.0					6846.6	
Approach LOS								F	
Intersction Delay									

Step 1: MOVEMENT PRIORITIES

Major Street: Approach Priority Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R
Minor Street:									

Approach Priority Movement	WestBound				EastBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x	0	0	1220	14		0	4	1148	48
Flow Rate, v_x		0	1326	15			4	1248	52

Minor Street: Approach Movement	WestBound				EastBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Volume, V_x		25	19	6		89	32	8
Flow Rate, v_x		27	21	7		97	35	9

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		0	1326	15		4	1248	52	
Conflicting Flow, v_c, x		1300				1341			

Minor Street: Approach Movement	WestBound				EastBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Flow Rate, v_x		27	21	7		97	35	9
Conflicting Flow, v_c, x		1984	2642	671		1930	2598	624

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	1U U	NB	4U U	SB	4	7	WestBound		10	EastBound	
		1 L					8 T	9 R		11 T	12 R
t_c, base											
Single Stage		4.1		4.1	7.5	6.5	6.9	7.5	6.5	6.9	
Stage I											
Stage II											
t_c, HV		2.0		2.0	2.0	2.0	2.0	2.0	2.0	2.0	
P_HV		0.13		0.13	0.03	0.03	0.03	0.03	0.03	0.03	
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1	
G					0	0	0	0	0	0	
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
t_c											
Single Stage		4.36		4.36	7.56	6.56	6.96	7.56	6.56	6.96	
Stage I											
Stage II											

FOLLOW-UP HEADWAYS Approach Movement	1U U	NB	4U U	SB	4	7	WestBound		10	EastBound	
		1 L					8 T	9 R		11 T	12 R
t_f, base											
t_f, HV		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3	
P_HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	
t_f		0.13		0.13	0.03	0.03	0.03	0.03	0.03	0.03	
		2.33		2.33	3.53	4.03	3.33	3.53	4.03	3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	1U U	NB	4U U	SB	4	7	WestBound		10	EastBound	
		1 L					8 T	9 R		11 T	12 R
v_c, x		1300		1341	1984	2642	671	1930	2598	624	
t_c, x		4.36		4.36	7.56	6.56	6.96	7.56	6.56	6.96	
t_f, x		2.33		2.33	3.53	4.03	3.33	3.53	4.03	3.33	
c_p, x		473		455	36	23	397	39	24	426	

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	NR	SR	WR	EB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		1300	1341	
Potential Capacity, $c_{p,x}$		473	455	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		473	455	
Probability of Queue-free State, $p_{0,j}$		1.000	0.990	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		671	624	
Potential Capacity, $c_{p,x}$		397	426	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		397	426	
Probability of Queue-free State, $p_{0,j}$		0.984	0.980	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$		2642	2598	
Potential Capacity, $c_{p,x}$		23	24	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Capacity Adjustment Factor, $f_x$		0.990	0.990	
Movement Capacity, $c_{m,x}$		22	24	
Probability of Queue-free State, $p_{0,j}$		0.077		
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$		1984	1930	
Potential Capacity, $c_{p,x}$		36	39	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$			0.076	
Major L, Minor T Impedance Factor, $p'$			0.191	
Capacity Adjustment Factor, $f_x$			0.188	
Movement Capacity, $c_{m,x}$			7	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	WestBound			EastBound		
Movement	7	8	9	10	11	12
Lane Configuration		LTR			LTR	
Shared Flow Rate, $v_y$		54			140	
Movement Capacity, $c_{m,x}$		22	397	7	24	426
Shared Capacity, $c_{SH}$					10	

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		0		4	27	21	7	97	35	9
Movement Capacity		473		455		22	397	7	24	426
Lane Configuration		L		L		LTR			LTR	
Shared Capacity									10	
Control Delay		12.6		13.0					6846.6	

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration		L		L		LTR			LTR	
Flow Rate		0		4		54			140	
Lane Capacity		473		455					10	
$v/c$		0.00		0.01					14.54	
95% Queue Length		0.0		0.0					19.1	
Control Delay		12.6		13.0					6846.6	



LOS  
Approach Delay  
Approach LOS  
Intersection Delay

B  
0.0

B  
0.0

F  
6846.6  
F

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 4:53:49 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_AM\_NO BUILD\_OSR-41-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: w/EB LTR No-Build AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Campbell-Old St Rd/ US 41  
 Major Street Direction: North-South  
 East/West Street Name: Campbell/OSR  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R
Volume	0	0	1220	14		0	4	1148	48
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		0	1326	15			4	1248	52
Percent Heavy Vehicles	13	13				13	13		
Number of Lanes	0	1	2	0		0	1	2	1
Lane Configuration		L	T	TR			L	T	R
Median Type					Undivided				
Median Storage									
RT channelized?									No
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street: Approach Movement		WestBound 7 L	8 T	9 R		EastBound 10 L	11 T	12 R
Volume		25	19	6		89	32	8
Peak Hour Factor, PHF					0.92			
Hourly Flow Rate, HFR		27	21	7		97	35	9
Percent Heavy Vehicles		3	3	3		3	3	3
Number of Lanes		0	1	0		1	1	0
Lane Configuration			LTR			L		TR
RT channelized?								
Flared Approach   Storage		No				No		
Percent Grade			0				0	

Pedestrian Volumes and Adjustments

Approach Movement		NB 13	SB 14	WB 15	EB 16
Flow (ped/hr)		0	0	0	0
Lane Width (ft)					
Walking Speed (ft/sec)					
Pedestrian Blockage Factor, f_pb					

Delay, Queue Length, and Level of Service

Approach Movement Lane Configuration	1U U	NB 1 L	4U L	SB 4 L	7 L	WestBound 8 LTR	9 L	EastBound 10 L	11 L	12 TR
Flow Rate		0		4		54		97		43
Lane Capacity		473		455				7		29
v/c		0.00		0.01				13.06		1.47
95% Queue Length		0.0		0.0				13.8		5.0
Control Delay		12.6		13.0				6402.2		537.5
LOS		B		B				F		F
Approach Delay		0.0		0.0					4583.7	
Approach LOS									F	
Intersction Delay										

Step 1: MOVEMENT PRIORITIES

Major Street: Approach Priority Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R
Minor Street:									

Approach Priority Movement	WestBound				EastBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x	0	0	1220	14		0	4	1148	48
Flow Rate, v_x		0	1326	15			4	1248	52

Minor Street: Approach Movement	WestBound				EastBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Volume, V_x		25	19	6		89	32	8
Flow Rate, v_x		27	21	7		97	35	9

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		0	1326	15		4	1248	52	
Conflicting Flow, v_c, x		1300				1341			

Minor Street: Approach Movement	WestBound				EastBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Flow Rate, v_x		27	21	7		97	35	9
Conflicting Flow, v_c, x		1984	2642	671		1930	2598	624

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	1U U	NB	4U U	SB	4	7	WestBound		10	EastBound	
		1 L					8 T	9 R		11 T	12 R
t_c, base											
Single Stage		4.1		4.1	7.5	6.5	6.9	7.5	6.5	6.9	
Stage I											
Stage II											
t_c, HV		2.0		2.0	2.0	2.0	2.0	2.0	2.0	2.0	
P_HV		0.13		0.13	0.03	0.03	0.03	0.03	0.03	0.03	
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1	
G					0	0	0	0	0	0	
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
t_c											
Single Stage		4.36		4.36	7.56	6.56	6.96	7.56	6.56	6.96	
Stage I											
Stage II											

FOLLOW-UP HEADWAYS Approach Movement	1U U	NB	4U U	SB	4	7	WestBound		10	EastBound	
		1 L					8 T	9 R		11 T	12 R
t_f, base											
t_f, HV		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3	
P_HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	
t_f		0.13		0.13	0.03	0.03	0.03	0.03	0.03	0.03	
		2.33		2.33	3.53	4.03	3.33	3.53	4.03	3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	1U U	NB	4U U	SB	4	7	WestBound		10	EastBound	
		1 L					8 T	9 R		11 T	12 R
v_c, x		1300		1341	1984	2642	671	1930	2598	624	
t_c, x		4.36		4.36	7.56	6.56	6.96	7.56	6.56	6.96	
t_f, x		2.33		2.33	3.53	4.03	3.33	3.53	4.03	3.33	
c_p, x		473		455	36	23	397	39	24	426	

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	NR	SR	WR	EB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		1300	1341	
Potential Capacity, $c_{p,x}$		473	455	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		473	455	
Probability of Queue-free State, $p_{0,j}$		1.000	0.990	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		671	624	
Potential Capacity, $c_{p,x}$		397	426	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		397	426	
Probability of Queue-free State, $p_{0,j}$		0.984	0.980	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$		2642	2598	
Potential Capacity, $c_{p,x}$		23	24	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Capacity Adjustment Factor, $f_x$		0.990	0.990	
Movement Capacity, $c_{m,x}$		22	24	
Probability of Queue-free State, $p_{0,j}$		0.077		
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$		1984	1930	
Potential Capacity, $c_{p,x}$		36	39	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$			0.076	
Major L, Minor T Impedance Factor, $p'$			0.191	
Capacity Adjustment Factor, $f_x$			0.188	
Movement Capacity, $c_{m,x}$			7	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	WestBound			EastBound		
Movement	7	8	9	10	11	12
Lane Configuration		LTR		L		TR
Shared Flow Rate, $v_y$		54		97		43
Movement Capacity, $c_{m,x}$		22	397	7	24	426
Shared Capacity, $c_{SH}$						29

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		0		4	27	21	7	97	35	9
Movement Capacity		473		455		22	397	7	24	426
Lane Configuration		L		L		LTR		L		TR
Shared Capacity										29
Control Delay		12.6		13.0				6402.2		537.5

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration		L		L		LTR		L		TR
Flow Rate		0		4		54		97		43
Lane Capacity		473		455				7		29
$v/c$		0.00		0.01				13.06		1.47
95% Queue Length		0.0		0.0				13.8		5.0
Control Delay		12.6		13.0				6402.2		537.5

LOS  
Approach Delay  
Approach LOS  
Intersection Delay

B  
0.0

B  
0.0

F

4583.7  
F

F

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 4:56:20 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_PM\_NO BUILD\_OSR-41-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Campbell-Old St Rd/ US 41  
 Major Street Direction: North-South  
 East/West Street Name: Campbell/OSR  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R
Volume	0	0	1220	14		0	4	1148	48
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		0	1326	15			4	1248	52
Percent Heavy Vehicles	13	13				13	13		
Number of Lanes	0	1	2	0		0	1	2	1
Lane Configuration		L	T	TR			L	T	R
Median Type					Undivided				
Median Storage									
RT channelized?									No
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street: Approach Movement		WestBound 7 L	8 T	9 R		EastBound 10 L	11 T	12 R
Volume		31	25	5		48	19	5
Peak Hour Factor, PHF					0.92			
Hourly Flow Rate, HFR		34	27	5		52	21	5
Percent Heavy Vehicles		3	3	3		3	3	3
Number of Lanes		0	1	0		0	1	0
Lane Configuration			LTR				LTR	
RT channelized?								
Flared Approach   Storage		No				No		
Percent Grade			0				0	

Pedestrian Volumes and Adjustments

Approach Movement		NB 13	SB 14	WB 15	EB 16
Flow (ped/hr)		0	0	0	0
Lane Width (ft)					
Walking Speed (ft/sec)					
Pedestrian Blockage Factor, f_pb					

Delay, Queue Length, and Level of Service

Approach Movement	1U U	NB 1 L	4U L	7 L	8 LTR	9 L	10 L	EastBound 11 LTR	12 R
Lane Configuration									
Flow Rate		0	4		66			78	
Lane Capacity		473	455		14				
v/c		0.00	0.01		4.76				
95% Queue Length		0.0	0.0		9.2				
Control Delay		12.6	13.0		2234.5				
LOS		B	B		F				
Approach Delay		0.0	0.0		2234.5				
Approach LOS					F				
Intersction Delay									

Step 1: MOVEMENT PRIORITIES

Major Street: Approach Priority Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R
Minor Street:									

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U	NorthBound			4U	SouthBound			
	U	1	2	3		4	5	6	
		L	T	R		L	T	R	
Volume, V_x	0	0	1220	14		0	4	1148	48
Flow Rate, v_x		0	1326	15			4	1248	52

Minor Street: Approach Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Volume, V_x	31	25	5		48	19	5
Flow Rate, v_x	34	27	5		52	21	5

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U	NorthBound			4U	SouthBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Flow Rate, v_x	0	1326	15		4	1248	52	
Conflicting Flow, v_c, x		1300				1341		

Minor Street: Approach Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Flow Rate, v_x	34	27	5		52	21	5
Conflicting Flow, v_c, x	1977	2642	671		1933	2598	624

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRI TICAL HEADWAYS										
Approach Movement	NB	SB			WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_c, base		4.1		4.1	7.5	6.5	6.9	7.5	6.5	6.9
Single Stage										
Stage I										
Stage II										
t_c, HV		2.0		2.0	2.0	2.0	2.0	2.0	2.0	2.0
P_HV		0.13		0.13	0.03	0.03	0.03	0.03	0.03	0.03
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1
G					0	0	0	0	0	0
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0
t_c										
Single Stage		4.36		4.36	7.56	6.56	6.96	7.56	6.56	6.96
Stage I										
Stage II										

FOLLOW-UP HEADWAYS										
Approach Movement	NB	SB			WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_f, base		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3
t_f, HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0
P_HV		0.13		0.13	0.03	0.03	0.03	0.03	0.03	0.03
t_f		2.33		2.33	3.53	4.03	3.33	3.53	4.03	3.33

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach Movement	NB	SB			WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
v_c, x		1300		1341	1977	2642	671	1933	2598	624
t_c, x		4.36		4.36	7.56	6.56	6.96	7.56	6.56	6.96
t_f, x		2.33		2.33	3.53	4.03	3.33	3.53	4.03	3.33
c_p, x		473		455	36	23	397	39	24	426

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	NR	SR	WR	EB
-------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		1300	1341	
Potential Capacity, $c_{p,x}$		473	455	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		473	455	
Probability of Queue-free State, $p_{0,j}$		1.000	0.990	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		671	624	
Potential Capacity, $c_{p,x}$		397	426	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		397	426	
Probability of Queue-free State, $p_{0,j}$		0.986	0.987	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$		2642	2598	
Potential Capacity, $c_{p,x}$		23	24	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Capacity Adjustment Factor, $f_x$		0.990	0.990	
Movement Capacity, $c_{m,x}$		22	24	
Probability of Queue-free State, $p_{0,j}$			0.137	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$		1977	1933	
Potential Capacity, $c_{p,x}$		36	39	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$		0.136		
Major L, Minor T Impedance Factor, $p'$		0.266		
Capacity Adjustment Factor, $f_x$		0.263		
Movement Capacity, $c_{m,x}$		10		

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES										
Approach	WestBound				EastBound					
Movement	7	8	9	10	11	12				
Lane Configuration	LTR				LTR					
Shared Flow Rate, $v_y$		66			78					
Movement Capacity, $c_{m,x}$	10	22	397		24	426				
Shared Capacity, $c_{SH}$		14								

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		0	4	34	27	5	52	21	5	
Movement Capacity		473	455	10	22	397		24	426	
Lane Configuration		L	L		LTR			LTR		
Shared Capacity					14					
Control Delay		12.6	13.0		2234.5					

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration		L	L		LTR				LTR	
Flow Rate		0	4		66				78	
Lane Capacity		473	455		14					
$v/c$		0.00	0.01		4.76					
95% Queue Length		0.0	0.0		9.2					
Control Delay		12.6	13.0		2234.5					



LOS	B	B	F
Approach Delay	0.0	0.0	2234.5
Approach LOS			F
Intersection Delay			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 5:01:11 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_PM\_NO BUILD\_OSR-41-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: w/EB LTR No-Build PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Campbell-Old St Rd/ US 41  
 Major Street Direction: North-South  
 East/West Street Name: Campbell/OSR  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R
Volume	0	0	1220	14		0	4	1148	48
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		0	1326	15			4	1248	52
Percent Heavy Vehicles	13	13				13	13		
Number of Lanes	0	1	2	0		0	1	2	1
Lane Configuration		L	T	TR			L	T	R
Median Type					Undivided				
Median Storage									
RT channelized?									No
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street: Approach Movement		WestBound 7 L	8 T	9 R		EastBound 10 L	11 T	12 R
Volume		31	25	5		48	19	5
Peak Hour Factor, PHF					0.92			
Hourly Flow Rate, HFR		34	27	5		52	21	5
Percent Heavy Vehicles		3	3	3		3	3	3
Number of Lanes		0	1	0		1	1	0
Lane Configuration			LTR			L		TR
RT channelized?								
Flared Approach   Storage		No				No		
Percent Grade			0				0	

Pedestrian Volumes and Adjustments

Approach Movement		NB 13	SB 14	WB 15	EB 16
Flow (ped/hr)		0	0	0	0
Lane Width (ft)					
Walking Speed (ft/sec)					
Pedestrian Blockage Factor, f_pb					

Delay, Queue Length, and Level of Service

Approach Movement	1U U	NB 1 L	4U L	7 L	8 LTR	9 L	10 L	11 L	12 TR
Lane Configuration									
Flow Rate		0	4		66		52		26
Lane Capacity		473	455		14				30
v/c		0.00	0.01		4.76				0.88
95% Queue Length		0.0	0.0		9.2				2.9
Control Delay		12.6	13.0		2234.5				317.9
LOS		B	B		F				F
Approach Delay		0.0	0.0		2234.5				
Approach LOS					F				
Intersction Delay									

Step 1: MOVEMENT PRIORITIES

Major Street: Approach Priority Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R
Minor Street:									

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Maj or Street: Approach Movement	1U	NorthBound			4U	SouthBound			
	U	1	2	3		4	5	6	
		L	T	R		L	T	R	
Volume, V_x	0	0	1220	14		0	4	1148	48
Flow Rate, v_x		0	1326	15			4	1248	52

Minor Street: Approach Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Volume, V_x	31	25	5		48	19	5
Flow Rate, v_x	34	27	5		52	21	5

Step 3: CONFLICTING FLOW RATES

Maj or Street: Approach Movement	1U	NorthBound			4U	SouthBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Flow Rate, v_x	0	1326	15		4	1248	52	
Conflicting Flow, v_c, x		1300				1341		

Minor Street: Approach Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Flow Rate, v_x	34	27	5		52	21	5
Conflicting Flow, v_c, x	1977	2642	671		1933	2598	624

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS										
Approach Movement	NB	SB			WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_c, base		4.1		4.1	7.5	6.5	6.9	7.5	6.5	6.9
Single Stage										
Stage I										
Stage II										
t_c, HV		2.0		2.0	2.0	2.0	2.0	2.0	2.0	2.0
P_HV		0.13		0.13	0.03	0.03	0.03	0.03	0.03	0.03
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1
G					0	0	0	0	0	0
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0
t_c										
Single Stage		4.36		4.36	7.56	6.56	6.96	7.56	6.56	6.96
Stage I										
Stage II										

FOLLOW-UP HEADWAYS										
Approach Movement	NB	SB			WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_f, base		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3
t_f, HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0
P_HV		0.13		0.13	0.03	0.03	0.03	0.03	0.03	0.03
t_f		2.33		2.33	3.53	4.03	3.33	3.53	4.03	3.33

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach Movement	NB	SB			WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
v_c, x		1300		1341	1977	2642	671	1933	2598	624
t_c, x		4.36		4.36	7.56	6.56	6.96	7.56	6.56	6.96
t_f, x		2.33		2.33	3.53	4.03	3.33	3.53	4.03	3.33
c_p, x		473		455	36	23	397	39	24	426

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	NR	SR	WR	EB
-------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		1300	1341	
Potential Capacity, $c_{p,x}$		473	455	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		473	455	
Probability of Queue-free State, $p_{0,j}$		1.000	0.990	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		671	624	
Potential Capacity, $c_{p,x}$		397	426	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		397	426	
Probability of Queue-free State, $p_{0,j}$		0.986	0.987	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$		2642	2598	
Potential Capacity, $c_{p,x}$		23	24	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Capacity Adjustment Factor, $f_x$		0.990	0.990	
Movement Capacity, $c_{m,x}$		22	24	
Probability of Queue-free State, $p_{0,j}$			0.137	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$		1977	1933	
Potential Capacity, $c_{p,x}$		36	39	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$		0.136		
Major L, Minor T Impedance Factor, $p'$		0.266		
Capacity Adjustment Factor, $f_x$		0.263		
Movement Capacity, $c_{m,x}$		10		

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	WestBound			EastBound		
Movement	7	8	9	10	11	12
Lane Configuration		LTR		L		TR
Shared Flow Rate, $v_y$		66		52		26
Movement Capacity, $c_{m,x}$	10	22	397		24	426
Shared Capacity, $c_{SH}$		14				30

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
Movement										
Flow Rate		0	4	34	27	5	52	21	5	
Movement Capacity		473	455	10	22	397		24	426	
Lane Configuration		L	L		LTR		L		TR	
Shared Capacity					14				30	
Control Delay		12.6	13.0		2234.5				317.9	

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
Movement										
Lane Configuration										
Flow Rate		0	4			66		52		26
Lane Capacity		473	455			14				30
$v/c$		0.00	0.01			4.76				0.88
95% Queue Length		0.0	0.0			9.2				2.9
Control Delay		12.6	13.0			2234.5				317.9

LOS  
Approach Delay  
Approach LOS  
Intersection Delay

B  
0.0

B  
0.0

F  
2234.5  
F

F

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 5:02:41 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_AM\_NO BUILD\_Wortman-OSR-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Wortman Rd / OSR  
 Major Street Direction: North-South  
 East/West Street Name: Wortman Road  
 North/South Street Name: Old State Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound				SouthBound			
Approach Movement	1U	2	3	4U	4	5	6	
	U	L	T	R	U	L	T	R
Volume		16	110			67	9	
Peak Hour Factor, PHF				0.92				
Hourly Flow Rate, HFR		17	120			73	10	
Percent Heavy Vehicles		3						
Number of Lanes	0	1	1	0	0	0	1	1
Lane Configuration		L	T				T	R
Median Type	Undivided							
Median Storage								
RT channelized?								
Left-Turn Lane Storage								
Upstream Signal?	Not Present							

Minor Street:	WestBound			EastBound		
Approach Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume				23		31
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR				25		34
Percent Heavy Vehicles				3		3
Number of Lanes	0	0	0	1	0	1
Lane Configuration				L		R
RT channelized?						
Flared Approach   Storage					0	
Percent Grade						

Pedestrian Volumes and Adjustments

Approach Movement	NB	SB	WB	EB
	13	14	15	16
Flow (ped/hr)	0	0		0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach Movement	1U	NB	4U	SB	7	8	9	10	EastBound	11	12
	U	L	T	R	WestBound	8	9	L	11	R	
Lane Configuration		L	T	R				L		R	
Flow Rate	17							25		34	
Lane Capacity	1508							750		986	
v/c	0.01							0.03		0.03	
95% Queue Length	0.0							0.1		0.1	
Control Delay	7.4							10.0		8.8	
LOS	A							A		A	
Approach Delay	0.9								9.3		
Approach LOS									A		
Intersection Delay	2.4										

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound				SouthBound			
Approach Priority Movement	1U	2	3	4U	4	5	6	
	U	L	T	R	U	L	T	R
Minor Street:								

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement	1U	NorthBound				4U	SouthBound		
	U	1	2	3		U	4	5	6
		L	T	R			L	T	R
Volume, V_x		16	110				67	9	
Flow Rate, v_x		17	120				73	10	

Minor Street:									
Approach Movement		WestBound				EastBound			
		7	8	9			10	11	12
		L	T	R			L	T	R
Volume, V_x							23	31	
Flow Rate, v_x							25	34	

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement	1U	NorthBound				4U	SouthBound		
	U	1	2	3		U	4	5	6
		L	T	R			L	T	R
Flow Rate, v_x		17	120				73	10	
Conflicting Flow, v_c, x		83							

Minor Street:									
Approach Movement		WestBound				EastBound			
		7	8	9			10	11	12
		L	T	R			L	T	R
Flow Rate, v_x							25	34	
Conflicting Flow, v_c, x							227	73	

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS												
Approach Movement	NB	1	4U	4	7	WestBound	8	9	10	EastBound	11	12
	1U	L	U	L	L	T	R	L	L	T	T	R
	U											
t_c, base		4.1							7.1			6.2
Single Stage												
Stage I												
Stage II												
t_c, HV		1.0							1.0			1.0
P_HV		0.03							0.03			0.03
t_c, G									0.2			0.1
G									0			0
t_3, LT		0.0							0.7			0.0
t_c												
Single Stage		4.13							6.43			6.23
Stage I												
Stage II												

FOLLOW-UP HEADWAYS												
Approach Movement	NB	1	4U	4	7	WestBound	8	9	10	EastBound	11	12
	1U	L	U	L	L	T	R	L	L	T	T	R
	U											
t_f, base		2.2							3.5			3.3
t_f, HV		0.9							0.9			0.9
P_HV		0.03							0.03			0.03
t_f		2.23							3.53			3.33

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT												
Approach Movement	NB	1	4U	4	7	WestBound	8	9	10	EastBound	11	12
	1U	L	U	L	L	T	R	L	L	T	T	R
	U											
v_c, x		83							227			73
t_c, x		4.13							6.43			6.23
t_f, x		2.23							3.53			3.33
c_p, x		1508							759			986

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	NR	SR	WR	EB
-------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0		0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		83		
Potential Capacity, $c_{p,x}$		1508		
Pedestrian Impedance Factor, $p_{p,x}$		1.000		
Movement Capacity, $c_{m,x}$		1508		
Probability of Queue-free State, $p_{0,j}$		0.988		
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$			73	
Potential Capacity, $c_{p,x}$			986	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Movement Capacity, $c_{m,x}$			986	
Probability of Queue-free State, $p_{0,j}$			0.966	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$				227
Potential Capacity, $c_{p,x}$				759
Pedestrian Impedance Factor, $p_{p,x}$				1.000
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$				0.988
Movement Capacity, $c_{m,x}$				750

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES											
Approach		WestBound				EastBound					
Movement	7	8	9			10	11	12			
Lane Configuration						L		R			
Shared Flow Rate, $v_y$						25		34			
Movement Capacity, $c_{m,x}$						750		986			
Shared Capacity, $c_{SH}$											

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS											
Approach		NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12	
Flow Rate		17						25		34	
Movement Capacity		1508						750		986	
Lane Configuration		L						L		R	
Shared Capacity											
Control Delay		7.4						10.0		8.8	

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach		NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12	
Lane Configuration		L						L		R	
Flow Rate		17						25		34	
Lane Capacity		1508						750		986	
$v/c$		0.01						0.03		0.03	
95% Queue Length		0.0						0.1		0.1	
Control Delay		7.4						10.0		8.8	



LOS	A	A		A
Approach Delay	0.9		9.3	
Approach LOS			A	
Intersection Delay	2.4			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 5:12:45 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_PM\_NO BUILD\_Wortman-OSR-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Wortman Rd / OSR  
 Major Street Direction: North-South  
 East/West Street Name: Wortman Road  
 North/South Street Name: Old State Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound						SouthBound		
Approach	1U	2	3	4U	4	5	6		
Movement	U	L	T	R	U	L	T	R	
Volume		28	96				149	36	
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		30	104				162	39	
Percent Heavy Vehicles		3							
Number of Lanes	0	1	1	0	0	0	1	1	
Lane Configuration		L	T				T	R	
Median Type					Undivided				
Median Storage									
RT channelized?								No	
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street:	WestBound			EastBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume				19		23
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR				21		25
Percent Heavy Vehicles				3		3
Number of Lanes	0	0	0	1	0	1
Lane Configuration				L		R
RT channelized?						No
Flared Approach   Storage					0	
Percent Grade						

Pedestrian Volumes and Adjustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)	0	0		0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	NB	SB	WestBound	EastBound
Movement	1U	4U	7	10
Lane Configuration	L	T	R	L
Flow Rate	30			21
Lane Capacity	1365			650
v/c	0.02			0.03
95% Queue Length	0.1			0.1
Control Delay	7.7			10.7
LOS	A			B
Approach Delay	1.7			9.9
Approach LOS				A
Intersection Delay	1.8			

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound						SouthBound		
Approach	1U	2	3	4U	4	5	6		
Priority	U	L	T	R	U	L	T	R	
Minor Street:									

Approach	WestBound					EastBound		
Priority	7	8	9			10	11	12
Movement	L	T	R			L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach	NorthBound					SouthBound			
Movement	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Volume, V_x		28	96				149	36	
Flow Rate, v_x		30	104				162	39	

Minor Street:									
Approach	WestBound					EastBound			
Movement		7	8	9			10	11	12
		L	T	R			L	T	R
Volume, V_x							19	23	
Flow Rate, v_x							21	25	

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach	NorthBound					SouthBound			
Movement	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Flow Rate, v_x		30	104				162	39	
Conflicting Flow, v_c, x		201							

Minor Street:									
Approach	WestBound					EastBound			
Movement		7	8	9			10	11	12
		L	T	R			L	T	R
Flow Rate, v_x							21	25	
Conflicting Flow, v_c, x							327	162	

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS										
Approach	NB		SB			WestBound		EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_c, base		4.1						7.1		6.2
Single Stage										
Stage I										
Stage II										
t_c, HV		1.0						1.0		1.0
P_HV		0.03						0.03		0.03
t_c, G								0.2		0.1
G								0		0
t_3, LT		0.0						0.7		0.0
t_c										
Single Stage		4.13						6.43		6.23
Stage I										
Stage II										

FOLLOW-UP HEADWAYS										
Approach	NB		SB			WestBound		EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_f, base		2.2						3.5		3.3
t_f, HV		0.9						0.9		0.9
P_HV		0.03						0.03		0.03
t_f		2.23						3.53		3.33

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach	NB		SB			WestBound		EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
v_c, x		201						327		162
t_c, x		4.13						6.43		6.23
t_f, x		2.23						3.53		3.33
c_p, x		1365						665		880

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0		0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		201		
Potential Capacity, $c_{p,x}$		1365		
Pedestrian Impedance Factor, $p_{p,x}$		1.000		
Movement Capacity, $c_{m,x}$		1365		
Probability of Queue-free State, $p_{0,j}$		0.978		
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$			162	
Potential Capacity, $c_{p,x}$			880	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Movement Capacity, $c_{m,x}$			880	
Probability of Queue-free State, $p_{0,j}$			0.972	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$			327	
Potential Capacity, $c_{p,x}$			665	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$			0.978	
Movement Capacity, $c_{m,x}$			650	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES											
Approach		WestBound				EastBound					
Movement	7	8	9			10	11	12			
Lane Configuration						L		R			
Shared Flow Rate, $v_y$						21		25			
Movement Capacity, $c_{m,x}$						650		880			
Shared Capacity, $c_{SH}$											

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS											
Approach		NB		SB		WestBound		EastBound			
Movement	1U	1	4U	4	7	8	9	10	11	12	
Flow Rate		30						21		25	
Movement Capacity		1365						650		880	
Lane Configuration		L						L		R	
Shared Capacity											
Control Delay		7.7						10.7		9.2	

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach		NB		SB		WestBound		EastBound			
Movement	1U	1	4U	4	7	8	9	10	11	12	
Lane Configuration		L						L		R	
Flow Rate		30						21		25	
Lane Capacity		1365						650		880	
$v/c$		0.02						0.03		0.03	
95% Queue Length		0.1						0.1		0.1	
Control Delay		7.7						10.7		9.2	

LOS	A	B		A
Approach Delay	1.7		9.9	
Approach LOS			A	
Intersection Delay	1.8			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 5:14:46 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_AM\_5-Rin-out-Hillisdale-Walnut-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5\_Rin-out\_AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Hillisdale Rd / Walnut Rd  
 Major Street Direction: East-West  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: Walnut Avenue  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Volume		1	72	22		12	103	4	
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		1	78	24		13	112	4	
Percent Heavy Vehicles		3				3			
Number of Lanes	0	0	1	0	0	0	1	0	
Lane Configuration			LTR				LTR		
Median Type					Undivided				
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street:	NorthBound			SouthBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume	22	5	20	4	5	5
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR	24	5	22	4	5	5
Percent Heavy Vehicles	3	3	3	3	3	3
Number of Lanes	0	1	0	0	1	0
Lane Configuration		LTR			LTR	
RT channelized?						
Flared Approach   Storage	No			No		
Percent Grade		0			0	

Pedestrian Volumes and Adjustments

Approach	EB	WB	NB	SB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	EB	WB	NorthBound	SouthBound
Movement	1U	4U	7	10
Lane Configuration	1	4	8	11
	U	L	T	LTR
Flow Rate	1	13	51	15
Lane Capacity	1466	1484	786	740
v/c	0.00	0.01	0.06	0.02
95% Queue Length	0.0	0.0	0.2	0.1
Control Delay	7.5	7.4	9.9	10.0
LOS	A	A	A	A
Approach Delay	0.1	0.8	9.9	10.0
Approach LOS			A	A
Intersection Delay	2.6			

Step 1: MOVEMENT PRIORITIES

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Priority	U	L	T	R	U	L	T	R	

Minor Street:

Approach Priority Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x		1	72	22			12	103	4
Flow Rate, v_x		1	78	24			13	112	4

Minor Street: Approach Movement	NorthBound				SouthBound				
	7 L	8 T	9 R		10 L	11 T	12 R		
Volume, V_x		22	5	20			4	5	5
Flow Rate, v_x		24	5	22			4	5	5

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		1	78	24			13	112	4
Conflicting Flow, v_c, x		116					102		

Minor Street: Approach Movement	NorthBound				SouthBound				
	7 L	8 T	9 R		10 L	11 T	12 R		
Flow Rate, v_x		24	5	22			4	5	5
Conflicting Flow, v_c, x		238	235	90			246	245	114

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_c, base											
Single Stage		4.1		4.1	7.1	6.5	6.2	7.1	6.5	6.2	
Stage I											
Stage II											
t_c, HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	
P_HV		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1	
G					0	0	0	0	0	0	
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
t_c											
Single Stage		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23	
Stage I											
Stage II											

FOLLOW-UP HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_f, base											
t_f, HV		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3	
P_HV		0.9		0.9	0.9	0.9	0.9	0.9	0.9	0.9	
t_f		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
c_p, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound		
					7 L	8 T	9 R	10 L	11 T	12 R
v_c, x		116		102	238	235	90	246	245	114
t_c, x		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23
t_f, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33
c_p, x		1466		1484	714	664	965	705	656	936

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	FR	WR	NR	SB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		116	102	
Potential Capacity, $c_{p,x}$		1466	1484	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		1466	1484	
Probability of Queue-free State, $p_{0,j}$		0.999	0.991	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$		0.999	0.991	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		90	114	
Potential Capacity, $c_{p,x}$		965	936	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		965	936	
Probability of Queue-free State, $p_{0,j}$		0.977	0.994	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$		235	245	
Potential Capacity, $c_{p,x}$		664	656	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Capacity Adjustment Factor, $f_x$		0.990	0.990	
Movement Capacity, $c_{m,x}$		657	649	
Probability of Queue-free State, $p_{0,j}$		0.992	0.992	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$		238	246	
Potential Capacity, $c_{p,x}$		714	705	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$		0.982	0.982	
Major L, Minor T Impedance Factor, $p'$		0.986	0.986	
Capacity Adjustment Factor, $f_x$		0.980	0.964	
Movement Capacity, $c_{m,x}$		700	680	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	NorthBound			SouthBound		
Movement	7	8	9	10	11	12
Lane Configuration		LTR			LTR	
Shared Flow Rate, $v_y$		51			15	
Movement Capacity, $c_{m,x}$	700	657	965	680	649	936
Shared Capacity, $c_{SH}$		786			740	

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		1		13	24	5	22	4	5	5
Movement Capacity		1466		1484	700	657	965	680	649	936
Lane Configuration						LTR			LTR	
Shared Capacity						786			740	
Control Delay		7.5		7.4		9.9			10.0	

CONTROL DELAY TO RANK 1 MOVEMENTS		
Approach	EB	WB
Movement	2	5
Number of Major Street Through Lanes, $N$	1	1
Proportion of Rank 1 vehicles not blocked, $p^*_{0,j}$	0.999	0.991
Delay to Major Left-turning Vehicles, $d_{MLT}$	7.5	7.4
Major Street Through Vehicles in Shared Lane, $v_{i1}$	78	112
Major Street Turning Vehicles in Shared Lane, $v_{i2}$	25	17
Saturation Flow Rate for Major Street Through, $s_{i1}$	1800	1800



Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

1500  
 0.0

1500  
 0.1

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement Lane Configuration	1U	EB 1	4U	WB 4	7	NorthBound 8	9	10	SouthBound 11	12
						LTR			LTR	
Flow Rate		1		13		51			15	
Lane Capacity		1466		1484		786			740	
v/c		0.00		0.01		0.06			0.02	
95% Queue Length		0.0		0.0		0.2			0.1	
Control Delay		7.5		7.4		9.9			10.0	
LOS		A		A		A			A	
Approach Delay		0.1		0.8		9.9			10.0	
Approach LOS						A			A	
Intersection Delay		2.6								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 1:27:44 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_PM\_5-Rin-out\_Hillisdale-Walnut-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5\_Rin-out PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Hillisdale Rd / Walnut Rd  
 Major Street Direction: East-West  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: Walnut Avenue  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	1U U	EastBound 1 L	2 T	3 R		4U U	WestBound 4 L	5 T	6 R
Volume		12	143	22			9	85	20
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		13	155	24			10	92	22
Percent Heavy Vehicles		3					3		
Number of Lanes	0	0	1	0		0	0	1	0
Lane Configuration			LTR					LTR	
Median Type					Undivided				
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street: Approach Movement		NorthBound 7 L	8 T	9 R			SouthBound 10 L	11 T	12 R
Volume		28	9	30			6	7	3
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		30	10	33			7	8	3
Percent Heavy Vehicles		3	3	3			3	3	3
Number of Lanes		0	1	0			0	1	0
Lane Configuration			LTR					LTR	
RT channelized?									
Flared Approach   Storage		No					No		
Percent Grade			0					0	

Pedestrian Volumes and Adjustments

Approach Movement		EB 13		WB 14		NB 15		SB 16
Flow (ped/hr)		0		0		0		0
Lane Width (ft)								
Walking Speed (ft/sec)								
Pedestrian Blockage Factor, f_pb								

Delay, Queue Length, and Level of Service

Approach Movement	1U	EB 1	4U 4	7 7	NorthBound 8 LTR	9	10	SouthBound 11 LTR	12
Lane Configuration					LTR			LTR	
Flow Rate		13			10			73	17
Lane Capacity		1469			1390			701	623
v/c		0.01			0.01			0.10	0.03
95% Queue Length		0.0			0.0			0.3	0.1
Control Delay		7.5			7.6			10.7	10.9
LOS		A			A			B	B
Approach Delay		0.6			0.7			10.7	10.9
Approach LOS								B	B
Intersection Delay		2.9							

Step 1: MOVEMENT PRIORITIES

Major Street: Approach Priority Movement	1U U	EastBound 1 L	2 T	3 R		4U U	WestBound 4 L	5 T	6 R
Minor Street:									

Approach Priority Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x		12	143	22			9	85	20
Flow Rate, v_x		13	155	24			10	92	22

Minor Street: Approach Movement	NorthBound				SouthBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Volume, V_x		28	9	30		6	7	3
Flow Rate, v_x		30	10	33		7	8	3

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		13	155	24			10	92	22
Conflicting Flow, v_c, x		114					179		

Minor Street: Approach Movement	NorthBound				SouthBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Flow Rate, v_x		30	10	33		7	8	3
Conflicting Flow, v_c, x		322	327	167		338	328	103

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_c, base											
Single Stage		4.1		4.1	7.1	6.5	6.2	7.1	6.5	6.2	
Stage I											
Stage II											
t_c, HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	
P_HV		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1	
G					0	0	0	0	0	0	
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
t_c											
Single Stage		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23	
Stage I											
Stage II											

FOLLOW-UP HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_f, base											
t_f, HV		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3	
P_HV		0.9		0.9	0.9	0.9	0.9	0.9	0.9	0.9	
t_f		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
c_p, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound		
					7 L	8 T	9 R	10 L	11 T	12 R
v_c, x		114		179	322	327	167	338	328	103
t_c, x		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23
t_f, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33
c_p, x		1469		1390	629	590	874	614	589	949

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	FR	WR	NR	SB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate v_x	0	0	0	0
Lane Width, w				
Walking Speed, S_p				
Pedestrian Blockage Factor, f_pb				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, v_c, x		114	179	
Potential Capacity, c_p, x		1469	1390	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Movement Capacity, c_m, x		1469	1390	
Probability of Queue-free State, p_0, j		0.991	0.993	
Major L-Shared Probability Queue-free State, p*_0, j		0.990	0.992	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, v_c, x		167	103	
Potential Capacity, c_p, x		874	949	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Movement Capacity, c_m, x		874	949	
Probability of Queue-free State, p_0, j		0.963	0.997	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, v_c, x				
Potential Capacity, c_p, x				
Capacity Adjustment Factor, f_x				
Movement Capacity, c_m, x				
Shared L/U Capacity, c_SH				
Probability of Queue-free State, p_0, j				
Minor-Street Through Movements		8	11	
Conflicting Flow, v_c, x		327	328	
Potential Capacity, c_p, x		590	589	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Capacity Adjustment Factor, f_x		0.983	0.983	
Movement Capacity, c_m, x		579	579	
Probability of Queue-free State, p_0, j		0.983	0.987	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, v_c, x		322	338	
Potential Capacity, c_p, x		629	614	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, p"		0.970	0.966	
Major L, Minor T Impedance Factor, p'		0.977	0.974	
Capacity Adjustment Factor, f_x		0.974	0.938	
Movement Capacity, c_m, x		613	576	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	NorthBound			SouthBound		
Movement	7	8	9	10	11	12
Lane Configuration		LTR			LTR	
Shared Flow Rate, v_y		73			17	
Movement Capacity, c_m, x	613	579	874	576	579	949
Shared Capacity, c_SH		701			623	

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB			WB						
	1U	1	4U	4	7	8	9	10	11	12
Movement										
Flow Rate		13		10	30	10	33	7	8	3
Movement Capacity		1469		1390	613	579	874	576	579	949
Lane Configuration						LTR			LTR	
Shared Capacity						701			623	
Control Delay		7.5		7.6		10.7			10.9	

CONTROL DELAY TO RANK 1 MOVEMENTS		
Approach	EB	WB
Movement	2	5
Number of Major Street Through Lanes, N	1	1
Proportion of Rank 1 vehicles not blocked, p*_0, j	0.990	0.992
Delay to Major Left-turning Vehicles, d_MLT	7.5	7.6
Major Street Through Vehicles in Shared Lane, v_i1	155	92
Major Street Turning Vehicles in Shared Lane, v_i2	37	32
Saturation Flow Rate for Major Street Through, s_i1	1800	1800

Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

1500  
 0.1

1500  
 0.1

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement Lane Configuration	1U	EB 1	4U	WB 4	7	NorthBound 8	9	10	SouthBound 11	12
						LTR			LTR	
Flow Rate		13		10		73			17	
Lane Capacity		1469		1390		701			623	
v/c		0.01		0.01		0.10			0.03	
95% Queue Length		0.0		0.0		0.3			0.1	
Control Delay		7.5		7.6		10.7			10.9	
LOS		A		A		B			B	
Approach Delay		0.6		0.7		10.7			10.9	
Approach LOS						B			B	
Intersection Delay		2.9								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 1:31:21 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_AM\_5-Rin-out\_Hillisdale-US 41-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5\_Rin-out AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Hillisdale Rd / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R
Volume	0	11	631	25		0	33	888	0
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		12	686	27			36	965	0
Percent Heavy Vehicles	3	3				3	3		
Number of Lanes	0	1	2	0		0	1	2	0
Lane Configuration		L	T	TR			L	T	TR
Median Type					Undivided				
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street: Approach Movement	WestBound 7 L	8 T	9 R		EastBound 10 L	11 T	12 R
Volume	99	8	41		0	22	14
Peak Hour Factor, PHF				0.92			
Hourly Flow Rate, HFR	108	9	45		0	24	15
Percent Heavy Vehicles	3	3	3		3	3	3
Number of Lanes	0	1	0		0	1	0
Lane Configuration		LTR				LTR	
RT channelized?							
Flared Approach   Storage	No				No		
Percent Grade		0				0	

Pedestrian Volumes and Adjustments

Approach Movement	NB 13	SB 14	WB 15	EB 16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach Movement	1U U	NB 1 L	4U L	7 L	8 LTR	9 L	10 L	EastBound 11 LTR	12 L
Lane Configuration									
Flow Rate	12		36		161			39	
Lane Capacity	703		876		111			115	
v/c	0.02		0.04		1.45			0.34	
95% Queue Length	0.1		0.1		11.5			1.4	
Control Delay	10.2		9.3		318.1			51.9	
LOS	B		A		F			F	
Approach Delay	0.2		0.3		318.1			51.9	
Approach LOS					F			F	
Intersection Delay	27.9								

Step 1: MOVEMENT PRIORITIES

Major Street: Approach Priority Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R
Minor Street:									

Approach Priority Movement	WestBound				EastBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x	0	11	631	25		0	33	888	0
Flow Rate, v_x		12	686	27			36	965	0

Minor Street: Approach Movement	WestBound				EastBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Volume, V_x		99	8	41		0	22	14
Flow Rate, v_x		108	9	45		0	24	15

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		12	686	27		36	965	0	
Conflicting Flow, v_c, x		965				713			

Minor Street: Approach Movement	WestBound				EastBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Flow Rate, v_x		108	9	45		0	24	15
Conflicting Flow, v_c, x		1290	1760	357		1408	1774	483

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	NB 1U U	1 L	SB 4U U	4 L	7 L	WestBound		9 R	10 L	EastBound	
						8 T				11 T	12 R
t_c, base											
Single Stage		4.1		4.1	7.5	6.5	6.9		7.5	6.5	6.9
Stage I											
Stage II											
t_c, HV		2.0		2.0	2.0	2.0	2.0		2.0	2.0	2.0
P_HV		0.03		0.03	0.03	0.03	0.03		0.03	0.03	0.03
t_c, G					0.2	0.2	0.1		0.2	0.2	0.1
G					0	0	0		0	0	0
t_3, LT		0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0
t_c											
Single Stage		4.16		4.16	7.56	6.56	6.96		7.56	6.56	6.96
Stage I											
Stage II											

FOLLOW-UP HEADWAYS Approach Movement	NB 1U U	1 L	SB 4U U	4 L	7 L	WestBound		9 R	10 L	EastBound	
						8 T				11 T	12 R
t_f, base											
t_f, HV		2.2		2.2	3.5	4.0	3.3		3.5	4.0	3.3
P_HV		1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0
t_f		0.03		0.03	0.03	0.03	0.03		0.03	0.03	0.03
c_p, x		2.23		2.23	3.53	4.03	3.33		3.53	4.03	3.33

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	NB 1U U	1 L	SB 4U U	4 L	7 L	WestBound		9 R	10 L	EastBound	
						8 T				11 T	12 R
v_c, x		965		713	1290	1760	357		1408	1774	483
t_c, x		4.16		4.16	7.56	6.56	6.96		7.56	6.56	6.96
t_f, x		2.23		2.23	3.53	4.03	3.33		3.53	4.03	3.33
c_p, x		703		876	120	83	637		98	81	527

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	NR	SR	WR	EB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate v_x	0	0	0	0
Lane Width, w				
Walking Speed, S_p				
Pedestrian Blockage Factor, f_pb				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, v_c, x		965	713	
Potential Capacity, c_p, x		703	876	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Movement Capacity, c_m, x		703	876	
Probability of Queue-free State, p_0, j		0.983	0.959	
Major L-Shared Probability Queue-free State, p*_0, j				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, v_c, x		357	483	
Potential Capacity, c_p, x		637	527	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Movement Capacity, c_m, x		637	527	
Probability of Queue-free State, p_0, j		0.930	0.971	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, v_c, x				
Potential Capacity, c_p, x				
Capacity Adjustment Factor, f_x				
Movement Capacity, c_m, x				
Shared L/U Capacity, c_SH				
Probability of Queue-free State, p_0, j				
Minor-Street Through Movements		8	11	
Conflicting Flow, v_c, x		1760	1774	
Potential Capacity, c_p, x		83	81	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Capacity Adjustment Factor, f_x		0.943	0.943	
Movement Capacity, c_m, x		78	76	
Probability of Queue-free State, p_0, j		0.889	0.687	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, v_c, x		1290	1408	
Potential Capacity, c_p, x		120	98	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, p"		0.648	0.838	
Major L, Minor T Impedance Factor, p'		0.727	0.875	
Capacity Adjustment Factor, f_x		0.706	0.814	
Movement Capacity, c_m, x		85	80	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES										
Approach	WestBound				EastBound					
Movement	7	8	9	10	11	12				
Lane Configuration		LTR			LTR					
Shared Flow Rate, v_y		161			39					
Movement Capacity, c_m, x	85	78	637	80	76	527				
Shared Capacity, c_SH		111			115					

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		12		36	108	9	45	0	24	15
Movement Capacity		703		876	85	78	637	80	76	527
Lane Configuration		L		L		LTR			LTR	
Shared Capacity						111			115	
Control Delay		10.2		9.3		318.1			51.9	

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound	
Movement	1U	1	4U	4	7	8	9	10	11
Lane Configuration		L		L		LTR			LTR
Flow Rate		12		36		161			39
Lane Capacity		703		876		111			115
v/c		0.02		0.04		1.45			0.34
95% Queue Length		0.1		0.1		11.5			1.4
Control Delay		10.2		9.3		318.1			51.9



LOS	B	A	F	F
Approach Delay	0.2	0.3	318.1	51.9
Approach LOS			F	F
Intersection Delay	27.9			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 1:34:37 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_PM\_NO BUILD\_Hillisdale-US 41-x. xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5\_Rin-out PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Hillisdale Rd / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R
Volume	0	20	958	42		0	53	1025	1
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		22	1041	46			58	1114	1
Percent Heavy Vehicles	3	3				3	3		
Number of Lanes	0	1	2	0		0	1	2	0
Lane Configuration		L	T	TR			L	T	TR
Median Type					Undivided				
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street: Approach Movement	WestBound 7 L	8 T	9 R		EastBound 10 L	11 T	12 R
Volume	42	21	44		3	21	6
Peak Hour Factor, PHF				0.92			
Hourly Flow Rate, HFR	46	23	48		3	23	7
Percent Heavy Vehicles	3	3	3		3	3	3
Number of Lanes	0	1	0		0	1	0
Lane Configuration		LTR				LTR	
RT channelized?							
Flared Approach   Storage	No				No		
Percent Grade		0				0	

Pedestrian Volumes and Adjustments

Approach Movement	NB 13	SB 14	WB 15	EB 16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach Movement	1U U	NB 1 L	4U L	7 L	8 LTR	9 L	10 L	EastBound 11 LTR	12 L
Lane Configuration									
Flow Rate		22	58		116			33	
Lane Capacity		616	632		34			34	
v/c		0.04	0.09		3.46			0.97	
95% Queue Length		0.1	0.3		13.6			3.4	
Control Delay		11.1	11.3		1352.3			320.6	
LOS		B	B		F			F	
Approach Delay		0.2	0.6		1352.3			320.6	
Approach LOS					F			F	
Intersction Delay		69.4							

Step 1: MOVEMENT PRIORITIES

Major Street: Approach Priority Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R
Minor Street:									

Approach Priority Movement	WestBound				EastBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x	0	20	958	42		0	53	1025	1
Flow Rate, v_x		22	1041	46			58	1114	1

Minor Street: Approach Movement	WestBound				EastBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Volume, V_x		42	21	44		3	21	6
Flow Rate, v_x		46	23	48		3	23	7

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		22	1041	46			58	1114	1
Conflicting Flow, v_c, x		1115					1087		

Minor Street: Approach Movement	WestBound				EastBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Flow Rate, v_x		46	23	48		3	23	7
Conflicting Flow, v_c, x		1791	2338	543		1805	2360	558

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	NB 1U U	1 L	SB 4U U	4 L	7 L	WestBound		9 R	10 L	EastBound	
						8 T				11 T	12 R
t_c, base											
Single Stage		4.1		4.1	7.5	6.5	6.9		7.5	6.5	6.9
Stage I											
Stage II											
t_c, HV		2.0		2.0	2.0	2.0	2.0		2.0	2.0	2.0
P_HV		0.03		0.03	0.03	0.03	0.03		0.03	0.03	0.03
t_c, G					0.2	0.2	0.1		0.2	0.2	0.1
G					0	0	0		0	0	0
t_3, LT		0.0		0.0	0.0	0.0	0.0		0.0	0.0	0.0
t_c											
Single Stage		4.16		4.16	7.56	6.56	6.96		7.56	6.56	6.96
Stage I											
Stage II											

FOLLOW-UP HEADWAYS Approach Movement	NB 1U U	1 L	SB 4U U	4 L	7 L	WestBound		9 R	10 L	EastBound	
						8 T				11 T	12 R
t_f, base											
t_f, HV		2.2		2.2	3.5	4.0	3.3		3.5	4.0	3.3
P_HV		1.0		1.0	1.0	1.0	1.0		1.0	1.0	1.0
t_f		0.03		0.03	0.03	0.03	0.03		0.03	0.03	0.03
		2.23		2.23	3.53	4.03	3.33		3.53	4.03	3.33

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	NB 1U U	1 L	SB 4U U	4 L	7 L	WestBound		9 R	10 L	EastBound	
						8 T				11 T	12 R
v_c, x		1115		1087	1791	2338	543		1805	2360	558
t_c, x		4.16		4.16	7.56	6.56	6.96		7.56	6.56	6.96
t_f, x		2.23		2.23	3.53	4.03	3.33		3.53	4.03	3.33
c_p, x		616		632	50	36	481		49	34	471

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	NR	SR	WR	EB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		1115	1087	
Potential Capacity, $c_{p,x}$		616	632	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		616	632	
Probability of Queue-free State, $p_{0,j}$		0.965	0.909	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		543	558	
Potential Capacity, $c_{p,x}$		481	471	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		481	471	
Probability of Queue-free State, $p_{0,j}$		0.901	0.986	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$		2338	2360	
Potential Capacity, $c_{p,x}$		36	34	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Capacity Adjustment Factor, $f_x$		0.877	0.877	
Movement Capacity, $c_{m,x}$		31	30	
Probability of Queue-free State, $p_{0,j}$		0.269	0.244	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$		1791	1805	
Potential Capacity, $c_{p,x}$		50	49	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$		0.214	0.236	
Major L, Minor T Impedance Factor, $p'$		0.350	0.372	
Capacity Adjustment Factor, $f_x$		0.345	0.335	
Movement Capacity, $c_{m,x}$		17	16	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	WestBound			EastBound		
Movement	7	8	9	10	11	12
Lane Configuration		LTR			LTR	
Shared Flow Rate, $v_y$		116			33	
Movement Capacity, $c_{m,x}$	17	31	481	16	30	471
Shared Capacity, $c_{SH}$		34			34	

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		22		58	46	23	48	3	23	7
Movement Capacity		616		632	17	31	481	16	30	471
Lane Configuration		L		L		LTR			LTR	
Shared Capacity						34			34	
Control Delay		11.1		11.3			1352.3			320.6

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration		L		L		LTR			LTR	
Flow Rate		22		58		116			33	
Lane Capacity		616		632		34			34	
$v/c$		0.04		0.09		3.46			0.97	
95% Queue Length		0.1		0.3		13.6			3.4	
Control Delay		11.1		11.3		1352.3			320.6	

LOS	B	B	F	F
Approach Delay	0.2	0.6	1352.3	320.6
Approach LOS			F	F
Intersection Delay	69.4			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 1:37:13 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis  
 File Name: 2020\_AM\_5-Rin-out-Hillisdale-OSR-x.txt  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5\_R-in-out AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Hillisdale Rd / Old State  
 Major Street Direction: North-South  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: Old State Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound				SouthBound			
Approach Movement	1U	2	3	4U	4	5	6	
	U	L	T	R	U	L	T	R
Volume		97	23			17	60	
Peak Hour Factor, PHF				0.92				
Hourly Flow Rate, HFR		105	25			18	65	
Percent Heavy Vehicles						3		
Number of Lanes	0	0	1	0	0	0	1	0
Lane Configuration				TR		LT		
Median Type	Undivided							
Median Storage								
RT channelized?								
Left-Turn Lane Storage								
Upstream Signal?	Not Present							

Minor Street:	WestBound			EastBound		
Approach Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	31		6			
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR	34		7			
Percent Heavy Vehicles	3		3			
Number of Lanes	0	1	0	0	0	0
Lane Configuration		LR				
RT channelized?						
Flared Approach   Storage	No					
Percent Grade		0				

Pedestrian Volumes and Adjustments

Approach Movement	NB	SB	WB	EB
	13	14	15	16
Flow (ped/hr)	0	0	0	
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach Movement	1U	NB	4U	SB	7	WestBound	8	9	10	EastBound	11	12
Lane Configuration						LR						
Flow Rate			18			40						
Lane Capacity			1449			780						
v/c			0.01			0.05						
95% Queue Length			0.0			0.2						
Control Delay			7.5			9.9						
LOS			A			A						
Approach Delay			1.7			9.9						
Approach LOS						A						
Intersection Delay	2.1											

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound				SouthBound			
Approach Priority Movement	1U	2	3	4U	4	5	6	
	U	L	T	R	U	L	T	R
Minor Street:								

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement	1U	NorthBound				4U	SouthBound		
	U	1	2	3		U	4	5	6
		L	T	R			L	T	R
Volume, V_x		97				17		60	
Flow Rate, v_x		105				18		65	

Minor Street:									
Approach Movement		WestBound				EastBound			
		7	8	9		10	11	12	
		L	T	R		L	T	R	
Volume, V_x		31				6			
Flow Rate, v_x		34				7			

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement	1U	NorthBound				4U	SouthBound		
	U	1	2	3		U	4	5	6
		L	T	R			L	T	R
Flow Rate, v_x		105				25		18	
Conflicting Flow, v_c, x						130		65	

Minor Street:									
Approach Movement		WestBound				EastBound			
		7	8	9		10	11	12	
		L	T	R		L	T	R	
Flow Rate, v_x		34				7			
Conflicting Flow, v_c, x		220				118			

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS											
Approach Movement		NB		SB		WestBound			EastBound		
	1U	1		4U	4	7	8	9	10	11	12
	U	L		U	L	L	T	R	L	T	R
t_c, base											
Single Stage					4.1	7.1		6.2			
Stage I											
Stage II											
t_c, HV					1.0	1.0		1.0			
P_HV					0.03	0.03		0.03			
t_c, G						0.2		0.1			
G						0		0			
t_3, LT					0.0	0.7		0.0			
t_c											
Single Stage					4.13	6.43		6.23			
Stage I											
Stage II											

FOLLOW-UP HEADWAYS											
Approach Movement		NB		SB		WestBound			EastBound		
	1U	1		4U	4	7	8	9	10	11	12
	U	L		U	L	L	T	R	L	T	R
t_f, base					2.2	3.5		3.3			
t_f, HV					0.9	0.9		0.9			
P_HV					0.03	0.03		0.03			
t_f					2.23	3.53		3.33			

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT											
Approach Movement		NB		SB		WestBound			EastBound		
	1U	1		4U	4	7	8	9	10	11	12
	U	L		U	L	L	T	R	L	T	R
v_c, x					130	220		118			
t_c, x					4.13	6.43		6.23			
t_f, x					2.23	3.53		3.33			
c_p, x					1449	766		931			

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach		NR		SR		WR		EB
-------------------------------	--	----	--	----	--	----	--	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$ Lane Width, $w$ Walking Speed, $S_p$ Pedestrian Blockage Factor, $f_{pb}$	0	0	0	
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$ Major L-Shared Probability Queue-free State, $p^*_{0,j}$			130 1449 1.000 1449 0.987 0.987	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$		118 931 1.000 931 0.993		
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Shared L/U Capacity, $c_{SH}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Major L, Minor T Adjusted Impedance Factor, $p''$ Major L, Minor T Impedance Factor, $p'$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$		220 766 1.000   0.987 756		

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES											
Approach	WestBound				EastBound						
Movement	7	8	9	10	11	12					
Lane Configuration		LR									
Shared Flow Rate, $v_y$		40									
Movement Capacity, $c_{m,x}$	756		931								
Shared Capacity, $c_{SH}$		780									

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS											
Approach	NB				SB						
Movement	1U	1	4U	4	7	8	9	10	11	12	
						LR					
Flow Rate							7				
Movement Capacity							931				
Lane Configuration						LR					
Shared Capacity							780				
Control Delay				7.5			9.9				

CONTROL DELAY TO RANK 1 MOVEMENTS												
Approach					NB				SB			
Movement					2				5			
Number of Major Street Through Lanes, $N$					1				1			
Proportion of Rank 1 vehicles not blocked, $p^*_{0,j}$									0.987			
Delay to Major Left-turning Vehicles, $d_{MLT}$									7.5			
Major Street Through Vehicles in Shared Lane, $v_{i1}$									65			
Major Street Turning Vehicles in Shared Lane, $v_{i2}$									18			
Saturation Flow Rate for Major Street Through, $s_{i1}$					1800				1800			



Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

1500

0.1

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement	1U	NB 1	4U	SB 4	7	WestBound 8	9	10	EastBound 11	12
Lane Configuration				LT		LR				
Flow Rate				18		40				
Lane Capacity				1449		780				
v/c				0.01		0.05				
95% Queue Length				0.0		0.2				
Control Delay				7.5		9.9				
LOS				A		A				
Approach Delay				1.7		9.9				
Approach LOS						A				
Intersection Delay		2.1								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 1:39:43 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_PM\_5-Rin-out\_Hillisdale-Old State-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: No-Build PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Hillisdale Rd / Old State  
 Major Street Direction: North-South  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: Old State Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound				SouthBound			
Approach	1U	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R
Volume		94	30			9	123	
Peak Hour Factor, PHF				0.92				
Hourly Flow Rate, HFR		102	33			10	134	
Percent Heavy Vehicles						3		
Number of Lanes	0	0	1	0	0	0	1	0
Lane Configuration				TR		LT		
Median Type					Undivided			
Median Storage								
RT channelized?								
Left-Turn Lane Storage								
Upstream Signal?					Not Present			

Minor Street:	WestBound			EastBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume	16		11			
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR	17		12			
Percent Heavy Vehicles	3		3			
Number of Lanes	0	1	0	0	0	0
Lane Configuration		LR				
RT channelized?						
Flared Approach   Storage	No					
Percent Grade		0				

Pedestrian Volumes and Adjustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	NB	SB	WestBound	EastBound
Movement	1U	4U	7	8
Lane Configuration		LT	LR	
Flow Rate		10	29	
Lane Capacity		1443	786	
v/c		0.01	0.04	
95% Queue Length		0.0	0.1	
Control Delay		7.5	9.8	
LOS		A	A	
Approach Delay		0.6	9.8	
Approach LOS			A	
Intersection Delay	1.2			

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound				SouthBound			
Approach	1U	2	3	4U	4	5	6	
Priority	U	L	T	R	U	L	T	R
Movement								

Minor Street:

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:							
Approach Movement	NorthBound				SouthBound		
	1U	2	3		4U	5	6
	U	L	T		U	L	R
Volume, V_x		94	30			9	123
Flow Rate, v_x		102	33			10	134

Minor Street:							
Approach Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Volume, V_x		16	11				
Flow Rate, v_x		17	12				

Step 3: CONFLICTING FLOW RATES

Major Street:							
Approach Movement	NorthBound				SouthBound		
	1U	2	3		4U	5	6
	U	L	T		U	L	R
Flow Rate, v_x		102	33			10	134
Conflicting Flow, v_c, x						135	

Minor Street:							
Approach Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Flow Rate, v_x		17	12				
Conflicting Flow, v_c, x		272	118				

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_c, base										
Single Stage				4.1	7.1		6.2			
Stage I										
Stage II										
t_c, HV				1.0	1.0		1.0			
P_HV				0.03	0.03		0.03			
t_c, G					0.2		0.1			
G					0		0			
t_3, LT				0.0	0.7		0.0			
t_c										
Single Stage				4.13	6.43		6.23			
Stage I										
Stage II										

FOLLOW-UP HEADWAYS										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_f, base				2.2	3.5		3.3			
t_f, HV				0.9	0.9		0.9			
P_HV				0.03	0.03		0.03			
t_f				2.23	3.53		3.33			

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
v_c, x				135	272		118			
t_c, x				4.13	6.43		6.23			
t_f, x				2.23	3.53		3.33			
c_p, x				1443	716		931			

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$ Lane Width, $w$ Walking Speed, $S_p$ Pedestrian Blockage Factor, $f_{pb}$	0	0	0	
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$ Major L-Shared Probability Queue-free State, $p^*_{0,j}$			135 1443 1.000 1443 0.993 0.993	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$		118 931 1.000 931 0.987		
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Shared L/U Capacity, $c_{SH}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Major L, Minor T Adjusted Impedance Factor, $p''$ Major L, Minor T Impedance Factor, $p'$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$		272 716 1.000   0.993 710		

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES											
Approach	WestBound				EastBound						
Movement	7	8	9	10	11	12					
Lane Configuration		LR									
Shared Flow Rate, $v_y$		29									
Movement Capacity, $c_{m,x}$	710		931								
Shared Capacity, $c_{SH}$		786									

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS												
Approach	NB				SB							
	1U	1	4U	4	7	8	9	10	11	12		
Flow Rate							12					
Movement Capacity					710		931					
Lane Configuration						LR						
Shared Capacity						786						
Control Delay				7.5		9.8						

CONTROL DELAY TO RANK 1 MOVEMENTS												
Approach					NB				SB			
Movement					2				5			
Number of Major Street Through Lanes, $N$					1				1			
Proportion of Rank 1 vehicles not blocked, $p^*_{0,j}$									0.993			
Delay to Major Left-turning Vehicles, $d_{MLT}$									7.5			
Major Street Through Vehicles in Shared Lane, $v_{i1}$									134			
Major Street Turning Vehicles in Shared Lane, $v_{i2}$									10			
Saturation Flow Rate for Major Street Through, $s_{i1}$					1800				1800			

Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

1500

0.1

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement Lane Configuration	NB 1U	1	4U SB	4 LT	7	WestBound 8 LR	9	10	EastBound 11	12
Flow Rate				10		29				
Lane Capacity				1443		786				
v/c				0.01		0.04				
95% Queue Length				0.0		0.1				
Control Delay				7.5		9.8				
LOS				A		A				
Approach Delay				0.6		9.8				
Approach LOS						A				
Intersection Delay		1.2								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 1:41:32 PM

HCS7 Two-Way Stop-Control Text Report

File Name: TWO-WAY STOP CONTROL (TWSC) Analysis  
 2020\_AM\_5-Rin-out\_Radio-Walnut-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5-Rin-out AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Radio Ave / Walnut Rd  
 Major Street Direction: East-West  
 East/West Street Name: Radio Ave  
 North/South Street Name: Walnut Road  
 Analysis Time Period (hrs): 0.25

Vehi cle Vol umes and Adj ustments

Major Street: Approach Movement	1U U	EastBound 1 L	2 T	3 R		4U U	WestBound 4 L	5 T	6 R
Volume		2	2	3			4	0	1
Peak Hour Factor, PHF					0.92				
Hourly Flow Rtae, HFR		2	2	3			4	0	1
Percent Heavy Vehicles		3					3		
Number of Lanes	0	0	1	0		0	0	1	0
Lane Configuration			LTR					LTR	
Median Type					Undivided				
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street: Approach Movement		NorthBound 7 L	8 T	9 R			SouthBound 10 L	11 T	12 R
Volume		7	12	1			1	3	4
Peak Hour Factor, PHF					0.92				
Hourly Flow Rtae, HFR		8	13	1			1	3	4
Percent Heavy Vehicles		3	3	3			3	3	3
Number of Lanes		0	1	0			0	1	0
Lane Configuration			LTR					LTR	
RT channelized?									
Flared Approach   Storage		No					No		
Percent Grade			0					0	

Pedestri an Vol umes and Adj ustments

Approach Movement		EB 13		WB 14		NB 15		SB 16
Flow (ped/hr)		0		0		0		0
Lane Width (ft)								
Walking Speed (ft/sec)								
Pedestrian Blockage Factor, f_pb								

Del ay, Queue Length, and Level of Service

Approach Movement	1U U	EB 1	4U 4	WB 4	7 7	NorthBound 8 LTR	9 9	10 10	SouthBound 11 LTR	12 12
Lane Configuration						LTR			LTR	
Flow Rate		2		4		22			9	
Lane Capacity		1615		1609		915			978	
v/c		0.00		0.00		0.02			0.01	
95% Queue Length		0.0		0.0		0.1			0.0	
Control Delay		7.2		7.2		9.0			8.7	
LOS		A		A		A			A	
Approach Delay		2.1		5.8		9.0			8.7	
Approach LOS						A			A	
Intersction Delay		7.3								

Step 1: MOVEMENT PRI OR TI ES

Major Street: Approach Priority Movement	1U U	EastBound 1 L	2 T	3 R		4U U	WestBound 4 L	5 T	6 R
Minor Street:									

Approach Priority Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x		2	2	3		4	0	1	
Flow Rate, v_x		2	2	3		4	0	1	

Minor Street: Approach Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R
Volume, V_x	7	12	1	1	3	4	
Flow Rate, v_x	8	13	1	1	3	4	

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		2	2	3		4	0	1	
Conflicting Flow, v_c, x		1				5			

Minor Street: Approach Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R
Flow Rate, v_x	8	13	1	1	3	4	
Conflicting Flow, v_c, x	21	18	4	24	19	1	

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound		
					7 L	8 T	9 R	10 L	11 T	12 R
t_c, base										
Single Stage		4.1		4.1	7.1	6.5	6.2	7.1	6.5	6.2
Stage I										
Stage II										
t_c, HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0
P_HV		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1
G					0	0	0	0	0	0
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0
t_c										
Single Stage		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23
Stage I										
Stage II										

FOLLOW-UP HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound		
					7 L	8 T	9 R	10 L	11 T	12 R
t_f, base										
t_f, HV		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3
P_HV		0.9		0.9	0.9	0.9	0.9	0.9	0.9	0.9
t_f		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03
c_p, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound		
					7 L	8 T	9 R	10 L	11 T	12 R
v_c, x		1		5	21	18	4	24	19	1
t_c, x		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23
t_f, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33
c_p, x		1615		1609	989	874	1077	984	873	1081

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	FR	WR	NR	SB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate v_x	0	0	0	0
Lane Width, w				
Walking Speed, S_p				
Pedestrian Blockage Factor, f_pb				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, v_c, x		1	5	
Potential Capacity, c_p, x		1615	1609	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Movement Capacity, c_m, x		1615	1609	
Probability of Queue-free State, p_0, j		0.999	0.997	
Major L-Shared Probability Queue-free State, p*_0, j		0.999	0.997	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, v_c, x		4	1	
Potential Capacity, c_p, x		1077	1081	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Movement Capacity, c_m, x		1077	1081	
Probability of Queue-free State, p_0, j		0.999	0.996	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, v_c, x				
Potential Capacity, c_p, x				
Capacity Adjustment Factor, f_x				
Movement Capacity, c_m, x				
Shared L/U Capacity, c_SH				
Probability of Queue-free State, p_0, j				
Minor-Street Through Movements		8	11	
Conflicting Flow, v_c, x		18	19	
Potential Capacity, c_p, x		874	873	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Capacity Adjustment Factor, f_x		0.996	0.996	
Movement Capacity, c_m, x		871	869	
Probability of Queue-free State, p_0, j		0.985	0.996	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, v_c, x		21	24	
Potential Capacity, c_p, x		989	984	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, p"		0.992	0.981	
Major L, Minor T Impedance Factor, p'		0.994	0.986	
Capacity Adjustment Factor, f_x		0.990	0.985	
Movement Capacity, c_m, x		979	969	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	NorthBound			SouthBound		
Movement	7	8	9	10	11	12
Lane Configuration		LTR			LTR	
Shared Flow Rate, v_y		22			9	
Movement Capacity, c_m, x	979	871	1077	969	869	1081
Shared Capacity, c_SH		915			978	

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		2		4	8	13	1	1	3	4
Movement Capacity		1615		1609	979	871	1077	969	869	1081
Lane Configuration						LTR			LTR	
Shared Capacity						915			978	
Control Delay		7.2		7.2		9.0			8.7	

CONTROL DELAY TO RANK 1 MOVEMENTS		
Approach	EB	WB
Movement	2	5
Number of Major Street Through Lanes, N	1	1
Proportion of Rank 1 vehicles not blocked, p*_0, j	0.999	0.997
Delay to Major Left-turning Vehicles, d_MLT	7.2	7.2
Major Street Through Vehicles in Shared Lane, v_i1	2	0
Major Street Turning Vehicles in Shared Lane, v_i2	5	5
Saturation Flow Rate for Major Street Through, s_i1	1800	1800



Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

1500  
 0.0

1500  
 0.0

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement Lane Configuration	1U	EB 1	4U WB 4	7	NorthBound 8 LTR	9	10	SouthBound 11 LTR	12
Flow Rate		2	4		22			9	
Lane Capacity		1615	1609		915			978	
v/c		0.00	0.00		0.02			0.01	
95% Queue Length		0.0	0.0		0.1			0.0	
Control Delay		7.2	7.2		9.0			8.7	
LOS		A	A		A			A	
Approach Delay		2.1	5.8		9.0			8.7	
Approach LOS					A			A	
Intersction Delay		7.3							

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 1:45:32 PM

HCS7 Two-Way Stop-Control Text Report

File Name: TWO-WAY STOP CONTROL (TWSC) Analysis  
 2020\_PM\_5-Rin-out\_Radio-Walnut-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5-Rin-out PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Radio Ave / Walnut Rd  
 Major Street Direction: East-West  
 East/West Street Name: Radio Ave  
 North/South Street Name: Walnut Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Volume		0	3	0		2	3	3	
Peak Hour Factor, PHF		0.92							
Hourly Flow Rate, HFR		0	3	0		2	3	3	
Percent Heavy Vehicles		3				3			
Number of Lanes	0	0	1	0	0	0	1	0	
Lane Configuration			LTR				LTR		
Median Type		Undivided							
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?		Not Present							

Minor Street:	NorthBound			SouthBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume	6	10	1	2	10	2
Peak Hour Factor, PHF	0.92					
Hourly Flow Rate, HFR	7	11	1	2	11	2
Percent Heavy Vehicles	3	3	3	3	3	3
Number of Lanes	0	1	0	0	1	0
Lane Configuration		LTR			LTR	
RT channelized?						
Flared Approach   Storage	No			No		
Percent Grade		0			0	

Pedestrian Volumes and Adjustments

Approach	EB	WB	NB	SB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f <sub>pb</sub>				

Delay, Queue Length, and Level of Service

Approach	EB	WB	NorthBound			SouthBound		
Movement	1U	4U	7	8	9	10	11	12
Lane Configuration				LTR			LTR	
Flow Rate	0	2	18			15		
Lane Capacity	1608	1612	921			916		
v/c	0.00	0.00	0.02			0.02		
95% Queue Length	0.0	0.0	0.1			0.1		
Control Delay	7.2	7.2	9.0			9.0		
LOS	A	A	A			A		
Approach Delay	0.0	1.8	9.0			9.0		
Approach LOS			A			A		
Intersection Delay	7.0							

Step 1: MOVEMENT PRIORITIES

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Minor Street:									

Approach Priority Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x		0	3	0			2	3	3
Flow Rate, v_x		0	3	0			2	3	3

Minor Street: Approach Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R
Volume, V_x	6	10	1		2	10	2
Flow Rate, v_x	7	11	1		2	11	2

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		0	3	0			2	3	3
Conflicting Flow, v_c, x		7					3		

Minor Street: Approach Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R
Flow Rate, v_x	7	11	1		2	11	2
Conflicting Flow, v_c, x	19	14	3		18	13	5

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound		
					7 L	8 T	9 R	10 L	11 T	12 R
t_c, base										
Single Stage		4.1		4.1	7.1	6.5	6.2	7.1	6.5	6.2
Stage I										
Stage II										
t_c, HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0
P_HV		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1
G					0	0	0	0	0	0
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0
t_c										
Single Stage		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23
Stage I										
Stage II										

FOLLOW-UP HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound		
					7 L	8 T	9 R	10 L	11 T	12 R
t_f, base										
t_f, HV		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3
P_HV		0.9		0.9	0.9	0.9	0.9	0.9	0.9	0.9
t_f		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03
c_p, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound		
					7 L	8 T	9 R	10 L	11 T	12 R
v_c, x		7		3	19	14	3	18	13	5
t_c, x		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23
t_f, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33
c_p, x		1608		1612	992	878	1078	993	880	1075

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	FR	WR	NR	SB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		7	3	
Potential Capacity, $c_{p,x}$		1608	1612	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		1608	1612	
Probability of Queue-free State, $p_{0,j}$		1.000	0.999	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$		1.000	0.999	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		3	5	
Potential Capacity, $c_{p,x}$		1078	1075	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		1078	1075	
Probability of Queue-free State, $p_{0,j}$		0.999	0.998	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$		14	13	
Potential Capacity, $c_{p,x}$		878	880	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Capacity Adjustment Factor, $f_x$		0.999	0.999	
Movement Capacity, $c_{m,x}$		877	879	
Probability of Queue-free State, $p_{0,j}$		0.988	0.988	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$		19	18	
Potential Capacity, $c_{p,x}$		992	993	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$		0.986	0.986	
Major L, Minor T Impedance Factor, $p'$		0.990	0.990	
Capacity Adjustment Factor, $f_x$		0.988	0.989	
Movement Capacity, $c_{m,x}$		980	982	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	NorthBound			SouthBound		
Movement	7	8	9	10	11	12
Lane Configuration		LTR			LTR	
Shared Flow Rate, $v_y$		18			15	
Movement Capacity, $c_{m,x}$	980	877	1078	982	879	1075
Shared Capacity, $c_{SH}$		921			916	

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB		WB		NorthBound			SouthBound		
	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		0		2	7	11	1	2	11	2
Movement Capacity		1608		1612	980	877	1078	982	879	1075
Lane Configuration						LTR			LTR	
Shared Capacity						921			916	
Control Delay		7.2		7.2		9.0			9.0	

CONTROL DELAY TO RANK 1 MOVEMENTS		
Approach	EB	WB
Movement	2	5
Number of Major Street Through Lanes, $N$	1	1
Proportion of Rank 1 vehicles not blocked, $p^*_{0,j}$	1.000	0.999
Delay to Major Left-turning Vehicles, $d_{MLT}$	7.2	7.2
Major Street Through Vehicles in Shared Lane, $v_{i1}$	3	3
Major Street Turning Vehicles in Shared Lane, $v_{i2}$	0	5
Saturation Flow Rate for Major Street Through, $s_{i1}$	1800	1800

Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

1500  
 0.0

1500  
 0.0

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement Lane Configuration	1U	EB 1	4U	WB 4	7	NorthBound 8	9	10	SouthBound 11	12
						LTR			LTR	
Flow Rate		0		2		18			15	
Lane Capacity		1608		1612		921			916	
v/c		0.00		0.00		0.02			0.02	
95% Queue Length		0.0		0.0		0.1			0.1	
Control Delay		7.2		7.2		9.0			9.0	
LOS		A		A		A			A	
Approach Delay		0.0		1.8		9.0			9.0	
Approach LOS						A			A	
Intersection Delay		7.0								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 1:48:08 PM

HCS7 Two-Way Stop-Control Text Report

File Name: TWO-WAY STOP CONTROL (TWSC) Analysis  
 2020\_AM\_5-Rin-out\_Radio-US 41-x. xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5-Rin-out AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Radio Ave / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Radio Ave  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehi cl e Vol umes and Adj ustments

Major Street:	NorthBound			SouthBound		
Approach	1U	2	3	4U	5	6
Movement	U	L T	R	U	L T	R
Volume		851	4		922	
Peak Hour Factor, PHF				0.92		
Hourly Flow Rtae, HFR		925	4		1002	
Percent Heavy Vehicles						
Number of Lanes	0	0	2	0	0	2
Lane Configuration		T	TR		T	
Median Type	Undivided					
Median Storage						
RT channelized?						
Left-Turn Lane Storage						
Upstream Signal ?	Not Present					

Minor Street:	WestBound			EastBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume			14			
Peak Hour Factor, PHF				0.92		
Hourly Flow Rtae, HFR			15			
Percent Heavy Vehicles			3			
Number of Lanes	0	0	1	0	0	0
Lane Configuration			R			
RT channelized?			No			
Flared Approach   Storage						
Percent Grade		0				

Pedestri an Vol umes and Adj ustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Del ay, Queue Length, and Level of Service

Approach	NB	SB	WestBound	EastBound
Movement	1U	4U	7	8
Lane Configuration			9 R	10
Flow Rate				15
Lane Capacity				542
v/c				0.03
95% Queue Length				0.1
Control Delay				11.8
LOS				B
Approach Delay				11.8
Approach LOS				B
Intersction Delay	0.1			

Step 1: MOVEMENT PRI OR I T I E S

Major Street:	NorthBound			SouthBound		
Approach	1U	2	3	4U	5	6
Pri or i ty	U	L T	R	U	L T	R

Minor Street:

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:							
Approach Movement	NorthBound				SouthBound		
	1U	2	3		4U	5	6
	U	L	T		U	L	R
Volume, V_x	851				922		
Flow Rate, v_x	925				1002		

Minor Street:							
Approach Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Volume, V_x	14						
Flow Rate, v_x	15						

Step 3: CONFLICTING FLOW RATES

Major Street:							
Approach Movement	NorthBound				SouthBound		
	1U	2	3		4U	5	6
	U	L	T		U	L	R
Flow Rate, v_x	925				1002		
Conflicting Flow, v_c, x	4				4		

Minor Street:							
Approach Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Flow Rate, v_x	15				465		
Conflicting Flow, v_c, x	465				15		

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS											
Approach Movement	NB		SB		WestBound			EastBound			
	1U	1	4U	4	7	8	9	10	11	12	
	U	L	U	L	L	T	R	L	T	R	
t_c, base							6.9				
Single Stage							6.9				
Stage I											
Stage II											
t_c, HV							2.0				
P_HV							0.03				
t_c, G							0.1				
G							0				
t_3, LT							0.0				
t_c							6.96				
Single Stage							6.96				
Stage I											
Stage II											

FOLLOW-UP HEADWAYS											
Approach Movement	NB		SB		WestBound			EastBound			
	1U	1	4U	4	7	8	9	10	11	12	
	U	L	U	L	L	T	R	L	T	R	
t_f, base							3.3				
t_f, HV							1.0				
P_HV							0.03				
t_f							3.33				

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT											
Approach Movement	NB		SB		WestBound			EastBound			
	1U	1	4U	4	7	8	9	10	11	12	
	U	L	U	L	L	T	R	L	T	R	
v_c, x							465				
t_c, x							6.96				
t_f, x							3.33				
c_p, x							542				

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$ Lane Width, $w$ Walking Speed, $S_p$ Pedestrian Blockage Factor, $f_{pb}$	0	0	0	
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$ Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$		465 542 1.000 542 0.972		
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Shared L/U Capacity, $c_{SH}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Major L, Minor T Adjusted Impedance Factor, $p''$ Major L, Minor T Impedance Factor, $p'$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$				

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES											
Approach	WestBound						EastBound				
Movement	7	8	9	10	11	12					
Lane Configuration			R								
Shared Flow Rate, $v_y$			15								
Movement Capacity, $c_{m,x}$			542								
Shared Capacity, $c_{SH}$											

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS											
Approach	NB		SB		WestBound			EastBound			
Movement	1U	1	4U	4	7	8	9	10	11	12	
Flow Rate							15				
Movement Capacity							542				
Lane Configuration							R				
Shared Capacity											
Control Delay							11.8				

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound			
Movement	1U	1	4U	4	7	8	9	10	11	12	
Lane Configuration							R				
Flow Rate							15				
Lane Capacity							542				
$v/c$							0.03				
95% Queue Length							0.1				
Control Delay							11.8				





HCS7 Two-Way Stop-Control Text Report

File Name: TWO-WAY STOP CONTROL (TWSC) Analysis  
 2020\_PM\_5-Rin-out\_Radio-US 41-x. xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5-Rin-out PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Radio Ave / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Radio Ave  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehi cl e Vol umes and Adj ustments

Major Street:	NorthBound			SouthBound		
Approach	1U	2	3	4U	5	6
Movement	U	L T	R	U	L T	R
Volume		822	3		1097	
Peak Hour Factor, PHF				0.92		
Hourly Flow Rtae, HFR		893	3		1192	
Percent Heavy Vehicles						
Number of Lanes	0	0	2	0	0	2
Lane Configuration		T	TR		T	
Median Type	Undivided					
Median Storage						
RT channelized?						
Left-Turn Lane Storage						
Upstream Signal ?	Not Present					

Minor Street:	WestBound			EastBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume			4			
Peak Hour Factor, PHF				0.92		
Hourly Flow Rtae, HFR			4			
Percent Heavy Vehicles			3			
Number of Lanes	0	0	1	0	0	0
Lane Configuration			R			
RT channelized?			No			
Flared Approach   Storage						
Percent Grade		0				

Pedestri an Vol umes and Adj ustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Del ay, Queue Length, and Level of Service

Approach	NB	SB	WestBound	EastBound
Movement	1U	4U	7	8
Lane Configuration			R	
Flow Rate				4
Lane Capacity				555
v/c				0.01
95% Queue Length				0.0
Control Delay				11.5
LOS				B
Approach Delay				11.5
Approach LOS				B
Interscti on Del ay	0.0			

Step 1: MOVEMENT PRI ORI TI ES

Major Street:	NorthBound			SouthBound		
Approach	1U	2	3	4U	5	6
Pri ority	U	L T	R	U	L T	R

Minor Street:

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement	NorthBound				SouthBound				
	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Volume, V_x	822			3		1097			
Flow Rate, v_x	893			3		1192			

Minor Street:							
Approach Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Volume, V_x	4				4		
Flow Rate, v_x	4				4		

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement	NorthBound				SouthBound				
	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Flow Rate, v_x	893			3		1192			
Conflicting Flow, v_c, x	893			3		1192			

Minor Street:							
Approach Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Flow Rate, v_x	4				448		
Conflicting Flow, v_c, x	4				448		

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS											
Approach Movement	NB		SB		WestBound			EastBound			
	1U	1	4U	4	7	8	9	10	11	12	
	U	L	U	L	L	T	R	L	T	R	
t_c, base							6.9				
Single Stage							6.9				
Stage I							6.9				
Stage II							6.9				
t_c, HV							2.0				
P_HV							0.03				
t_c, G							0.1				
G							0				
t_3, LT							0.0				
t_c							6.96				
Single Stage							6.96				
Stage I							6.96				
Stage II							6.96				

FOLLOW-UP HEADWAYS											
Approach Movement	NB		SB		WestBound			EastBound			
	1U	1	4U	4	7	8	9	10	11	12	
	U	L	U	L	L	T	R	L	T	R	
t_f, base							3.3				
t_f, HV							1.0				
P_HV							0.03				
t_f							3.33				

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT											
Approach Movement	NB		SB		WestBound			EastBound			
	1U	1	4U	4	7	8	9	10	11	12	
	U	L	U	L	L	T	R	L	T	R	
v_c, x							448				
t_c, x							6.96				
t_f, x							3.33				
c_p, x							555				

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$ Lane Width, $w$ Walking Speed, $S_p$ Pedestrian Blockage Factor, $f_{pb}$	0	0	0	
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$ Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$		448 555 1.000 555 0.992		
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Shared L/U Capacity, $c_{SH}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Major L, Minor T Adjusted Impedance Factor, $p''$ Major L, Minor T Impedance Factor, $p'$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$				

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES											
Approach	WestBound						EastBound				
Movement	7	8	9	10	11	12					
Lane Configuration			R								
Shared Flow Rate, $v_y$			4								
Movement Capacity, $c_{m,x}$			555								
Shared Capacity, $c_{SH}$											

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS											
Approach	NB		SB		WestBound			EastBound			
Movement	1U	1	4U	4	7	8	9	10	11	12	
Flow Rate							4				
Movement Capacity							555				
Lane Configuration							R				
Shared Capacity											
Control Delay							11.5				

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound			
Movement	1U	1	4U	4	7	8	9	10	11	12	
Lane Configuration							R				
Flow Rate							4				
Lane Capacity							555				
$v/c$							0.01				
95% Queue Length							0.0				
Control Delay							11.5				



HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_AM\_5-Rin-out\_Campbell-Walnut-x.txt  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5-Rin-out AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Campbell Rd / Walnut Rd  
 Major Street Direction: East-West  
 East/West Street Name: Campbell Road  
 North/South Street Name: Walnut Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Volume			4	2		1	12		
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR			4	2		1	13		
Percent Heavy Vehicles						3			
Number of Lanes	0	0	1	1	0	1	1	0	
Lane Configuration			T	R		L	T		
Median Type	Undivided								
Median Storage									
RT channelized?	No								
Left-Turn Lane Storage									
Upstream Signal?	Not Present								

Minor Street:	NorthBound			SouthBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume	14		2			
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR	15		2			
Percent Heavy Vehicles	3		3			
Number of Lanes	1	0	1	0	0	0
Lane Configuration	L		R			
RT channelized?			No			
Flared Approach   Storage						
Percent Grade		0				

Pedestrian Volumes and Adjustments

Approach	EB	WB	NB	SB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f <sub>pb</sub>				

Delay, Queue Length, and Level of Service

Approach	EB	NorthBound			SouthBound			
Movement	1U	4U	7	8	9	10	11	12
Lane Configuration		L	L	R	R			
Flow Rate		1	15		2			
Lane Capacity		1608	994		1076			
v/c		0.00	0.02		0.00			
95% Queue Length		0.0	0.0		0.0			
Control Delay		7.2	8.7		8.4			
LOS		A	A		A			
Approach Delay		0.6		8.6				
Approach LOS				A				
Intersection Delay	4.2							

Step 1: MOVEMENT PRIORITIES

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Priority									

Minor Street:

Approach Priority Movement	NorthBound				SouthBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:								
Approach Movement	1U	EastBound			4U	WestBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Volume, V_x		4	2			1	12	
Flow Rate, v_x		4	2			1	13	

Minor Street:							
Approach Movement	NorthBound				SouthBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Volume, V_x	14		2				
Flow Rate, v_x	15		2				

Step 3: CONFLICTING FLOW RATES

Major Street:								
Approach Movement	1U	EastBound			4U	WestBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Flow Rate, v_x		4	2			1	13	
Conflicting Flow, v_c, x						7		

Minor Street:							
Approach Movement	NorthBound				SouthBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Flow Rate, v_x	15		2				
Conflicting Flow, v_c, x	20		4				

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS														
Approach Movement	1U	EB	1	4U	WB	4	7	NorthBound	8	9	10	SouthBound	11	12
	U		L	U		L	L		T	R	L		T	R
t_c, base														
Single Stage						4.1	7.1			6.2				
Stage I														
Stage II														
t_c, HV						1.0	1.0			1.0				
P_HV						0.03	0.03			0.03				
t_c, G							0.2			0.1				
G							0			0				
t_3, LT						0.0	0.7			0.0				
t_c														
Single Stage						4.13	6.43			6.23				
Stage I														
Stage II														

FOLLOW-UP HEADWAYS														
Approach Movement	1U	EB	1	4U	WB	4	7	NorthBound	8	9	10	SouthBound	11	12
	U		L	U		L	L		T	R	L		T	R
t_f, base						2.2	3.5			3.3				
t_f, HV						0.9	0.9			0.9				
P_HV						0.03	0.03			0.03				
t_f						2.23	3.53			3.33				

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT														
Approach Movement	1U	EB	1	4U	WB	4	7	NorthBound	8	9	10	SouthBound	11	12
	U		L	U		L	L		T	R	L		T	R
v_c, x						7	20			4				
t_c, x						4.13	6.43			6.23				
t_f, x						2.23	3.53			3.33				
c_p, x						1608	995			1076				

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	FR	WR	NR	SB

Movement	13	14	15	16
Pedestrian Flow Rate v_x Lane Width, w Walking Speed, S_p Pedestrian Blockage Factor, f_pb	0	0	0	
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j Major L-Shared Probability Queue-free State, p*_0, j			7 1608 1.000 1608 0.999	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j		4 1076 1.000 1076 0.998		
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Capacity Adjustment Factor, f_x Movement Capacity, c_m, x Shared L/U Capacity, c_SH Probability of Queue-free State, p_0, j				
Minor-Street Through Movements		8	11	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Capacity Adjustment Factor, f_x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Major L, Minor T Adjusted Impedance Factor, p" Major L, Minor T Impedance Factor, p' Capacity Adjustment Factor, f_x Movement Capacity, c_m, x		20 995 1.000   0.999 994		

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES										
Approach	NorthBound							SouthBound		
Movement	7	8	9		10	11	12			
Lane Configuration	L		R							
Shared Flow Rate, v_y	15		2							
Movement Capacity, c_m, x	994		1076							
Shared Capacity, c_SH										

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate					1	15		2		
Movement Capacity					1608	994		1076		
Lane Configuration					L	L		R		
Shared Capacity										
Control Delay					7.2	8.7		8.4		

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration					L	L		R		
Flow Rate					1	15		2		
Lane Capacity					1608	994		1076		
v/c					0.00	0.02		0.00		
95% Queue Length					0.0	0.0		0.0		
Control Delay					7.2	8.7		8.4		



LOS		A	A		A
Approach Delay		0.6		8.6	
Approach LOS				A	
Intersection Delay	4.2				

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 1:55:50 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_PM\_5-Rin-out\_Campbell-Walnut-x.txt  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5-Rin-out PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Campbell Rd / Walnut Rd  
 Major Street Direction: East-West  
 East/West Street Name: Campbell Road  
 North/South Street Name: Walnut Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Volume			15	17		1	5		
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR			16	18		1	5		
Percent Heavy Vehicles						3			
Number of Lanes	0	0	1	1	0	1	1	0	
Lane Configuration			T	R		L	T		
Median Type	Undivided								
Median Storage									
RT channelized?	No								
Left-Turn Lane Storage									
Upstream Signal?	Not Present								

Minor Street:	NorthBound			SouthBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume	8		0			
Peak Hour Factor, PHF						0.92
Hourly Flow Rate, HFR	9		0			
Percent Heavy Vehicles	3		3			
Number of Lanes	1	0	1	0	0	0
Lane Configuration	L		R			
RT channelized?			No			
Flared Approach   Storage						
Percent Grade		0				

Pedestrian Volumes and Adjustments

Approach	EB	WB	NB	SB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	EB	WB	NorthBound			SouthBound		
Movement	1U	4U	7	8	9	10	11	12
Lane Configuration		L	L	R	R			
Flow Rate		1	9		0			
Lane Capacity		1570	989		1060			
v/c		0.00	0.01		0.00			
95% Queue Length		0.0	0.0		0.0			
Control Delay		7.3	8.7		8.4			
LOS		A	A		A			
Approach Delay		1.2		8.7				
Approach LOS				A				
Intersection Delay	1.7							

Step 1: MOVEMENT PRIORITIES

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Priority									

Minor Street:

Approach Priority Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement	1U U	EastBound		3 R		4U U	WestBound		6 R
		1 L	2 T				4 L	5 T	
Volume, V_x		15		17		1		5	
Flow Rate, v_x		16		18		1		5	

Minor Street:							
Approach Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R
Volume, V_x	8		0		1		5
Flow Rate, v_x	9		0		1		5

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement	1U U	EastBound		3 R		4U U	WestBound		6 R
		1 L	2 T				4 L	5 T	
Flow Rate, v_x		16		18		1		5	
Conflicting Flow, v_c, x						35			

Minor Street:							
Approach Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R
Flow Rate, v_x	9		0		1		5
Conflicting Flow, v_c, x	24		16				

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS												
Approach Movement	1U U	EB L	1	4U U	WB L	4	7 L	NorthBound 8 T	9 R	10 L	SouthBound 11 T	12 R
t_c, base												
Single Stage						4.1	7.1		6.2			
Stage I												
Stage II												
t_c, HV						1.0	1.0		1.0			
P_HV						0.03	0.03		0.03			
t_c, G							0.2		0.1			
G							0		0			
t_3, LT						0.0	0.7		0.0			
t_c												
Single Stage						4.13	6.43		6.23			
Stage I												
Stage II												

FOLLOW-UP HEADWAYS												
Approach Movement	1U U	EB L	1	4U U	WB L	4	7 L	NorthBound 8 T	9 R	10 L	SouthBound 11 T	12 R
t_f, base												
t_f, HV						2.2	3.5		3.3			
P_HV						0.9	0.9		0.9			
t_f						0.03	0.03		0.03			
c_p, x						2.23	3.53		3.33			

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT												
Approach Movement	1U U	EB L	1	4U U	WB L	4	7 L	NorthBound 8 T	9 R	10 L	SouthBound 11 T	12 R
v_c, x												
t_c, x						35	24		16			
t_f, x						4.13	6.43		6.23			
c_p, x						2.23	3.53		3.33			
						1570	990		1060			

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	FR	WR	NR	SB

Movement	13	14	15	16
Pedestrian Flow Rate v_x Lane Width, w Walking Speed, S_p Pedestrian Blockage Factor, f_pb	0	0	0	
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j Major L-Shared Probability Queue-free State, p*_0, j			35 1570 1.000 1570 0.999	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j		16 1060 1.000 1060 1.000		
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Capacity Adjustment Factor, f_x Movement Capacity, c_m, x Shared L/U Capacity, c_SH Probability of Queue-free State, p_0, j				
Minor-Street Through Movements		8	11	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Capacity Adjustment Factor, f_x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Major L, Minor T Adjusted Impedance Factor, p" Major L, Minor T Impedance Factor, p' Capacity Adjustment Factor, f_x Movement Capacity, c_m, x		24 990 1.000   0.999 989		

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES										
Approach	NorthBound				SouthBound					
Movement	7	8	9		10	11	12			
Lane Configuration	L		R							
Shared Flow Rate, v_y	9		0							
Movement Capacity, c_m, x	989		1060							
Shared Capacity, c_SH										

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate	1				9		0			
Movement Capacity	1570				989		1060			
Lane Configuration	L				L		R			
Shared Capacity										
Control Delay	7.3				8.7		8.4			

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration				L	L		R			
Flow Rate	1				9		0			
Lane Capacity	1570				989		1060			
v/c	0.00				0.01		0.00			
95% Queue Length	0.0				0.0		0.0			
Control Delay	7.3				8.7		8.4			

LOS		A	A		A
Approach Delay		1.2		8.7	
Approach LOS				A	
Intersection Delay	1.7				

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 1:58:34 PM

HCS7 Two-Way Stop-Control Text Report

File Name: TWO-WAY STOP CONTROL (TWSC) Analysis  
 2020\_AM\_5-Rin-out\_Campbell-OSR-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5-Rin-out AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Campbell Rd / OSR  
 Major Street Direction: East-West  
 East/West Street Name: Campbell Road  
 North/South Street Name: Old State Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	1U U	EastBound 1 L	2 T	3 R	4U U	WestBound 4 L	5 T	6 R
Volume		37	25				11	17
Peak Hour Factor, PHF					0.92			
Hourly Flow Rate, HFR		40	27				12	18
Percent Heavy Vehicles		3						
Number of Lanes	0	1	1	0	0	0	1	1
Lane Configuration		L	T				T	R
Median Type					Undivided			
Median Storage								
RT channelized?								No
Left-Turn Lane Storage								
Upstream Signal?					Not Present			

Minor Street: Approach Movement	NorthBound 7 L	8 T	9 R	SouthBound 10 L	11 T	12 R
Volume				14		64
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR				15		70
Percent Heavy Vehicles				3		3
Number of Lanes	0	0	0	1	0	1
Lane Configuration				L		R
RT channelized?						No
Flared Approach   Storage					0	
Percent Grade						

Pedestrian Volumes and Adjustments

Approach Movement	EB 13	WB 14	NB 15	SB 16
Flow (ped/hr)	0	0		0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach Movement Lane Configuration	1U U	EB 1 L	4U U	4 T	7 R	NorthBound 8	9	SouthBound 10 L	11 T	12 R
Flow Rate		40						15		70
Lane Capacity		1576						851		1066
v/c		0.03						0.02		0.07
95% Queue Length		0.1						0.1		0.2
Control Delay		7.3						9.3		8.6
LOS		A						A		A
Approach Delay		4.4							8.7	
Approach LOS									A	
Intersection Delay		5.7								

Step 1: MOVEMENT PRIORITIES

Major Street: Approach Priority Movement	1U U	EastBound 1 L	2 T	3 R	4U U	WestBound 4 L	5 T	6 R
Minor Street:								

Approach Priority Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	EastBound		3 R		4U U	WestBound		6 R
		1 L	2 T				4 L	5 T	
Volume, V_x		37	25				11	17	
Flow Rate, v_x		40	27				12	18	

Minor Street: Approach Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R
Volume, V_x					14		64
Flow Rate, v_x					15		70

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	EastBound		3 R		4U U	WestBound		6 R
		1 L	2 T				4 L	5 T	
Flow Rate, v_x		40	27				12	18	
Conflicting Flow, v_c, x		30							

Minor Street: Approach Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R
Flow Rate, v_x					15		70
Conflicting Flow, v_c, x					120		12

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	EB	WB			NorthBound			SouthBound		
	1U U	1 L	4U U	4 L	7 L	8 T	9 R	10 L	11 T	12 R
t_c, base										
Single Stage		4.1						7.1		6.2
Stage I										
Stage II										
t_c, HV		1.0						1.0		1.0
P_HV		0.03						0.03		0.03
t_c, G								0.2		0.1
G								0		0
t_3, LT		0.0						0.7		0.0
t_c										
Single Stage		4.13						6.43		6.23
Stage I										
Stage II										

FOLLOW-UP HEADWAYS Approach Movement	EB	WB			NorthBound			SouthBound		
	1U U	1 L	4U U	4 L	7 L	8 T	9 R	10 L	11 T	12 R
t_f, base		2.2						3.5		3.3
t_f, HV		0.9						0.9		0.9
P_HV		0.03						0.03		0.03
t_f		2.23						3.53		3.33

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	EB	WB			NorthBound			SouthBound		
	1U U	1 L	4U U	4 L	7 L	8 T	9 R	10 L	11 T	12 R
v_c, x		30						120		12
t_c, x		4.13						6.43		6.23
t_f, x		2.23						3.53		3.33
c_p, x		1576						874		1066

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	FR	WR	NR	SB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0		0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		30		
Potential Capacity, $c_{p,x}$		1576		
Pedestrian Impedance Factor, $p_{p,x}$		1.000		
Movement Capacity, $c_{m,x}$		1576		
Probability of Queue-free State, $p_{0,j}$		0.974		
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$			12	
Potential Capacity, $c_{p,x}$			1066	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Movement Capacity, $c_{m,x}$			1066	
Probability of Queue-free State, $p_{0,j}$			0.935	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$			120	
Potential Capacity, $c_{p,x}$			874	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$			0.974	
Movement Capacity, $c_{m,x}$			851	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES											
Approach	NorthBound						SouthBound				
Movement	7	8	9				10	11	12		
Lane Configuration							L			R	
Shared Flow Rate, $v_y$							15			70	
Movement Capacity, $c_{m,x}$							851			1066	
Shared Capacity, $c_{SH}$											

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS												
Approach	EB			WB			NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12		
Flow Rate		40						15		70		
Movement Capacity		1576						851		1066		
Lane Configuration		L						L		R		
Shared Capacity												
Control Delay		7.3						9.3		8.6		

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	EB			WB			NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12		
Lane Configuration		L						L		R		
Flow Rate		40						15		70		
Lane Capacity		1576						851		1066		
$v/c$		0.03						0.02		0.07		
95% Queue Length		0.1						0.1		0.2		
Control Delay		7.3						9.3		8.6		



LOS	A	A		A
Approach Delay	4.4		8.7	
Approach LOS			A	
Intersection Delay	5.7			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 2:00:30 PM

HCS7 Two-Way Stop-Control Text Report

File Name: TWO-WAY STOP CONTROL (TWSC) Analysis  
 2020\_PM\_5-Rin-out\_Campbell-OSR-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5-Rin-out PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Campbell Rd / OSR  
 Major Street Direction: East-West  
 East/West Street Name: Campbell Road  
 North/South Street Name: Old State Road  
 Analysis Time Period (hrs): 0.25

Vehi cle Vol umes and Adj ustments

Major Street: Approach Movement	1U U	EastBound 1 L	2 T	3 R	4U U	WestBound 4 L	5 T	6 R
Volume		44	17				9	7
Peak Hour Factor, PHF					0.92			
Hourly Flow Rtae, HFR		48	18				10	8
Percent Heavy Vehicles		3						
Number of Lanes	0	1	1	0	0	0	1	1
Lane Configuration		L	T				T	R
Median Type					Undivided			
Median Storage								
RT channelized?								No
Left-Turn Lane Storage								
Upstream Signal?					Not Present			

Minor Street: Approach Movement	NorthBound 7 L	8 T	9 R	SouthBound 10 L	11 T	12 R
Volume				26		33
Peak Hour Factor, PHF				0.92		
Hourly Flow Rtae, HFR				28		36
Percent Heavy Vehicles				3		3
Number of Lanes	0	0	0	1	0	1
Lane Configuration				L		R
RT channelized?						No
Flared Approach   Storage					0	
Percent Grade						

Pedestri an Vol umes and Adj ustments

Approach Movement	EB 13	WB 14	NB 15	SB 16
Flow (ped/hr)	0	0		0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Del ay, Queue Length, and Level of Service

Approach Movement	1U U	EB 1 L	4U 4	WB 4	7 7	NorthBound 8	9	10 L	SouthBound 11	12 R
Lane Configuration		L						L		R
Flow Rate		48						28		36
Lane Capacity		1593						843		1069
v/c		0.03						0.03		0.03
95% Queue Length		0.1						0.1		0.1
Control Delay		7.3						9.4		8.5
LOS		A						A		A
Approach Delay		5.3							8.9	
Approach LOS									A	
Intersction Delay		6.2								

Step 1: MOVEMENT PRI OR TI ES

Major Street: Approach Priority Movement	1U U	EastBound 1 L	2 T	3 R	4U U	WestBound 4 L	5 T	6 R
Minor Street:								

Approach Priority Movement	NorthBound				SouthBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement	1U	EastBound		3		4U	WestBound		
	U	1	2	R		U	4	5	6
		L	T				L	T	R
Volume, V_x		44	17				9	7	
Flow Rate, v_x		48	18				10	8	

Minor Street:								
Approach Movement	NorthBound				SouthBound			
	7	8	9		10	11	12	
	L	T	R		L	T	R	
Volume, V_x					26		33	
Flow Rate, v_x					28		36	

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement	1U	EastBound		3		4U	WestBound		
	U	1	2	R		U	4	5	6
		L	T				L	T	R
Flow Rate, v_x		48	18				10	8	
Conflicting Flow, v_c, x		17							

Minor Street:								
Approach Movement	NorthBound				SouthBound			
	7	8	9		10	11	12	
	L	T	R		L	T	R	
Flow Rate, v_x					28		36	
Conflicting Flow, v_c, x					124		10	

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS														
Approach Movement	1U	EB	1	4U	WB	4	7	NorthBound	8	9	SouthBound	10	11	12
	U		L	U		L	L		T	R	L	T	R	
t_c, base			4.1								7.1		6.2	
Single Stage														
Stage I														
Stage II														
t_c, HV			1.0								1.0		1.0	
P_HV			0.03								0.03		0.03	
t_c, G											0.2		0.1	
G											0		0	
t_3, LT			0.0								0.7		0.0	
t_c														
Single Stage			4.13								6.43		6.23	
Stage I														
Stage II														

FOLLOW-UP HEADWAYS														
Approach Movement	1U	EB	1	4U	WB	4	7	NorthBound	8	9	SouthBound	10	11	12
	U		L	U		L	L		T	R	L	T	R	
t_f, base			2.2								3.5		3.3	
t_f, HV			0.9								0.9		0.9	
P_HV			0.03								0.03		0.03	
t_f			2.23								3.53		3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT														
Approach Movement	1U	EB	1	4U	WB	4	7	NorthBound	8	9	SouthBound	10	11	12
	U		L	U		L	L		T	R	L	T	R	
v_c, x			17								124		10	
t_c, x			4.13								6.43		6.23	
t_f, x			2.23								3.53		3.33	
c_p, x			1593								869		1069	

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	FR	WR	NR	SB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0		0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		17		
Potential Capacity, $c_{p,x}$		1593		
Pedestrian Impedance Factor, $p_{p,x}$		1.000		
Movement Capacity, $c_{m,x}$		1593		
Probability of Queue-free State, $p_{0,j}$		0.970		
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$			10	
Potential Capacity, $c_{p,x}$			1069	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Movement Capacity, $c_{m,x}$			1069	
Probability of Queue-free State, $p_{0,j}$			0.966	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$			124	
Potential Capacity, $c_{p,x}$			869	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$			0.970	
Movement Capacity, $c_{m,x}$			843	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	NorthBound			SouthBound		
Movement	7	8	9	10	11	12
Lane Configuration				L		R
Shared Flow Rate, $v_y$				28		36
Movement Capacity, $c_{m,x}$				843		1069
Shared Capacity, $c_{SH}$						

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		48						28		36
Movement Capacity		1593						843		1069
Lane Configuration		L						L		R
Shared Capacity										
Control Delay		7.3						9.4		8.5

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration		L						L		R
Flow Rate		48						28		36
Lane Capacity		1593						843		1069
$v/c$		0.03						0.03		0.03
95% Queue Length		0.1						0.1		0.1
Control Delay		7.3						9.4		8.5

LOS	A	A		A
Approach Delay	5.3		8.9	
Approach LOS			A	
Intersection Delay	6.2			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 2:02:03 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_AM\_5-Rin-out\_OSR-US 41-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5\_Rin-out AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Old State Rd / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R
Volume	0	0	981	11		0	3	920	38
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		0	1066	12			3	1000	41
Percent Heavy Vehicles	3	3				3	3		
Number of Lanes	0	1	2	0		0	1	2	0
Lane Configuration		L	T	TR			L	T	TR
Median Type					Undivided				
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street: Approach Movement	WestBound 7 L	8 T	9 R		EastBound 10 L	11 T	12 R
Volume	20	15	5		69	25	6
Peak Hour Factor, PHF				0.92			
Hourly Flow Rate, HFR	22	16	5		75	27	7
Percent Heavy Vehicles	3	3	3		3	3	3
Number of Lanes	0	1	0		0	1	0
Lane Configuration		LTR				LTR	
RT channelized?							
Flared Approach   Storage	No				No		
Percent Grade		0				0	

Pedestrian Volumes and Adjustments

Approach Movement	NB 13	SB 14	WB 15	EB 16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach Movement	1U U	NB 1 L	4U L	7 L	8 LTR	9 L	10 L	EastBound 11 LTR	12 L
Lane Configuration		L	L	L	LTR	L	L	LTR	L
Flow Rate	0		3		43			109	
Lane Capacity	658		637		49			56	
v/c	0.00		0.01		0.89			1.94	
95% Queue Length	0.0		0.0		3.7			10.5	
Control Delay	10.5		10.7		228.0			600.0	
LOS	B		B		F			F	
Approach Delay	0.0		0.0		228.0			600.0	
Approach LOS					F			F	
Intersction Delay	33.0								

Step 1: MOVEMENT PRIORITIES

Major Street: Approach Priority Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R
Minor Street:									

Approach Priority Movement	WestBound				EastBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x	0	0	981	11		0	3	920	38
Flow Rate, v_x		0	1066	12			3	1000	41

Minor Street: Approach Movement	WestBound				EastBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Volume, V_x		20	15	5		69	25	6
Flow Rate, v_x		22	16	5		75	27	7

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		0	1066	12		3	1000	41	
Conflicting Flow, v_c, x		1041				1078			

Minor Street: Approach Movement	WestBound				EastBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Flow Rate, v_x		22	16	5		75	27	7
Conflicting Flow, v_c, x		1592	2120	539		1568	2105	521

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	1U U	NB	4U U	SB	4	7	WestBound		10	EastBound	
		1 L					8 T	9 R		11 T	12 R
t_c, base											
Single Stage		4.1		4.1	7.5	6.5	6.9	7.5	6.5	6.9	
Stage I											
Stage II											
t_c, HV		2.0		2.0	2.0	2.0	2.0	2.0	2.0	2.0	
P_HV		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1	
G					0	0	0	0	0	0	
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
t_c											
Single Stage		4.16		4.16	7.56	6.56	6.96	7.56	6.56	6.96	
Stage I											
Stage II											

FOLLOW-UP HEADWAYS Approach Movement	1U U	NB	4U U	SB	4	7	WestBound		10	EastBound	
		1 L					8 T	9 R		11 T	12 R
t_f, base											
t_f, HV		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3	
P_HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	
t_f		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
c_p, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	1U U	NB	4U U	SB	4	7	WestBound		10	EastBound	
		1 L					8 T	9 R		11 T	12 R
v_c, x		1041		1078	1592	2120	539	1568	2105	521	
t_c, x		4.16		4.16	7.56	6.56	6.96	7.56	6.56	6.96	
t_f, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33	
c_p, x		658		637	71	49	484	74	50	498	

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	NR	SR	WR	EB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		1041	1078	
Potential Capacity, $c_{p,x}$		658	637	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		658	637	
Probability of Queue-free State, $p_{0,j}$		1.000	0.995	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		539	521	
Potential Capacity, $c_{p,x}$		484	498	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		484	498	
Probability of Queue-free State, $p_{0,j}$		0.989	0.987	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$		2120	2105	
Potential Capacity, $c_{p,x}$		49	50	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Capacity Adjustment Factor, $f_x$		0.995	0.995	
Movement Capacity, $c_{m,x}$		49	50	
Probability of Queue-free State, $p_{0,j}$		0.666	0.455	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$		1592	1568	
Potential Capacity, $c_{p,x}$		71	74	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$		0.453	0.663	
Major L, Minor T Impedance Factor, $p'$		0.567	0.738	
Capacity Adjustment Factor, $f_x$		0.560	0.730	
Movement Capacity, $c_{m,x}$		40	54	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	WestBound			EastBound		
Movement	7	8	9	10	11	12
Lane Configuration		LTR			LTR	
Shared Flow Rate, $v_y$		43			109	
Movement Capacity, $c_{m,x}$	40	49	484	54	50	498
Shared Capacity, $c_{SH}$		49			56	

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		0		3	22	16	5	75	27	7
Movement Capacity		658		637	40	49	484	54	50	498
Lane Configuration		L		L		LTR			LTR	
Shared Capacity						49			56	
Control Delay		10.5		10.7		228.0			600.0	

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration		L		L		LTR			LTR	
Flow Rate		0		3		43			109	
Lane Capacity		658		637		49			56	
$v/c$		0.00		0.01		0.89			1.94	
95% Queue Length		0.0		0.0		3.7			10.5	
Control Delay		10.5		10.7		228.0			600.0	



LOS	B	B	F	F
Approach Delay	0.0	0.0	228.0	600.0
Approach LOS			F	F
Intersection Delay	33.0			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/21/2019 4:17:20 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis  
 File Name: 2020\_AM\_R-in-out\_Old State-Wortman.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5-Rin-out AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Old State Rd / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Wortman Road  
 North/South Street Name: Old State Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound				SouthBound			
Approach	1U	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R
Volume		12	85				52	7
Peak Hour Factor, PHF					0.92			
Hourly Flow Rtae, HFR		13	92				57	8
Percent Heavy Vehicles		0						
Number of Lanes	0	0	1	0	0	0	1	0
Lane Configuration		LT						TR
Median Type					Undivided			
Median Storage								
RT channelized?								
Left-Turn Lane Storage								
Upstream Signal?					Not Present			

Minor Street:	WestBound			EastBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume				18		24
Peak Hour Factor, PHF				0.92		
Hourly Flow Rtae, HFR				20		26
Percent Heavy Vehicles				0		3
Number of Lanes	0	0	0	0	1	0
Lane Configuration					LR	
RT channelized?						
Flared Approach   Storage				No		
Percent Grade					0	

Pedestrian Volumes and Adjustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)	0	0		0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Approach	Delay, Queue Length, and Level of Service										
	NB	WestBound			EastBound						
Movement	1U	4U	4	7	8	9	10	11	12		
Lane Configuration	LT							LR			
Flow Rate	13							46			
Lane Capacity	1551							909			
v/c	0.01							0.05			
95% Queue Length	0.0							0.2			
Control Delay	7.3							9.2			
LOS	A							A			
Approach Delay	1.0							9.2			
Approach LOS								A			
Intersction Delay	2.4										

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound				SouthBound			
Approach	1U	2	3	4U	4	5	6	
Priorty	U	L	T	R	U	L	T	R
Movement								
Minor Street:								

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:								
Approach Movement	1U	NorthBound				SouthBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Volume, V_x		12	85			52	7	
Flow Rate, v_x		13	92			57	8	

Minor Street:								
Approach Movement		WestBound				EastBound		
		7	8	9		10	11	12
		L	T	R		L	T	R
Volume, V_x						18	24	
Flow Rate, v_x						20	26	

Step 3: CONFLICTING FLOW RATES

Major Street:								
Approach Movement	1U	NorthBound				SouthBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Flow Rate, v_x		13	92			57	8	
Conflicting Flow, v_c, x		64						

Minor Street:								
Approach Movement		WestBound				EastBound		
		7	8	9		10	11	12
		L	T	R		L	T	R
Flow Rate, v_x						20	26	
Conflicting Flow, v_c, x						179	60	

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS											
Approach Movement		NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12	
	U	L	U	L	L	T	R	L	T	R	
t_c, base											
Single Stage		4.1						7.1		6.2	
Stage I											
Stage II											
t_c, HV		1.0						1.0		1.0	
P_HV		0.00						0.00		0.03	
t_c, G								0.2		0.1	
G								0		0	
t_3, LT		0.0						0.7		0.0	
t_c											
Single Stage		4.10						6.40		6.23	
Stage I											
Stage II											

FOLLOW-UP HEADWAYS											
Approach Movement		NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12	
	U	L	U	L	L	T	R	L	T	R	
t_f, base		2.2						3.5		3.3	
t_f, HV		0.9						0.9		0.9	
P_HV		0.00						0.00		0.03	
t_f		2.20						3.50		3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT											
Approach Movement		NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12	
	U	L	U	L	L	T	R	L	T	R	
v_c, x		64						179		60	
t_c, x		4.10						6.40		6.23	
t_f, x		2.20						3.50		3.33	
c_p, x		1551						815		1002	

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate v_x Lane Width, w Walking Speed, S_p Pedestrian Blockage Factor, f_pb	0	0		0
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j Major L-Shared Probability Queue-free State, p*_0, j		64 1551 1.000 1551 0.992 0.991		
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j			60 1002 1.000 1002 0.974	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Capacity Adjustment Factor, f_x Movement Capacity, c_m, x Shared L/U Capacity, c_SH Probability of Queue-free State, p_0, j				
Minor-Street Through Movements		8	11	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Capacity Adjustment Factor, f_x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Major L, Minor T Adjusted Impedance Factor, p" Major L, Minor T Impedance Factor, p' Capacity Adjustment Factor, f_x Movement Capacity, c_m, x			179 815 1.000   0.991 808	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	7	WestBound	9	10	EastBound	12
Movement		8			11	
Lane Configuration					LR	
Shared Flow Rate, v_y Movement Capacity, c_m, x Shared Capacity, c_SH				808	46 909	1002

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB			SB			WestBound		EastBound	
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		13						20		26
Movement Capacity		1551						808		1002
Lane Configuration		LT							LR	
Shared Capacity									909	
Control Delay		7.3							9.2	

CONTROL DELAY TO RANK 1 MOVEMENTS			
Approach	NB		SB
Movement	2		5
Number of Major Street Through Lanes, N	1		1
Proportion of Rank 1 vehicles not blocked, p*_0, j	0.991		
Delay to Major Left-turning Vehicles, d_MLT	7.3		
Major Street Through Vehicles in Shared Lane, v_i1	92		
Major Street Turning Vehicles in Shared Lane, v_i2	13		
Saturation Flow Rate for Major Street Through, s_i1	1800		1800

Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

0.1

1500

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement Lane Configuration	NB 1U	1 LT	4U SB	4	7	WestBound 8	9	10	EastBound 11	12
Flow Rate		13							46	
Lane Capacity		1551							909	
v/c		0.01							0.05	
95% Queue Length		0.0							0.2	
Control Delay		7.3							9.2	
LOS		A							A	
Approach Delay		1.0							9.2	
Approach LOS									A	
Intersction Delay		2.4								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 2:42:00 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis  
 File Name: 2020\_PM\_R-in-out\_Old State-Wortman.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5-Rin-out PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Old State Rd / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Wortman Road  
 North/South Street Name: Old State Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	1U U	NorthBound 1 L	2 T	3 R	4U U	SouthBound 4 L	5 T	6 R
Volume		22	74				115	28
Peak Hour Factor, PHF					0.92			
Hourly Flow Rate, HFR		24	80				125	30
Percent Heavy Vehicles		0						
Number of Lanes	0	0	1	0	0	0	1	0
Lane Configuration		LT						TR
Median Type					Undivided			
Median Storage								
RT channelized?								
Left-Turn Lane Storage								
Upstream Signal?					Not Present			

Minor Street: Approach Movement	WestBound 7 L	8 T	9 R	EastBound 10 L	11 T	12 R
Volume				15		18
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR				16		20
Percent Heavy Vehicles				0		3
Number of Lanes	0	0	0	0	1	0
Lane Configuration					LR	
RT channelized?						
Flared Approach   Storage				No		
Percent Grade					0	

Pedestrian Volumes and Adjustments

Approach Movement	NB 13	SB 14	WB 15	EB 16
Flow (ped/hr)	0	0		0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Approach Movement	1U U	NB 1 L	2 T	4U U	3 R	7 8	9	10	EastBound 11 L	12 R
Lane Configuration		LT							LR	
Flow Rate		24							36	
Lane Capacity		1437							806	
v/c		0.02							0.04	
95% Queue Length		0.1							0.1	
Control Delay		7.5							9.7	
LOS		A							A	
Approach Delay		1.8							9.7	
Approach LOS									A	
Intersection Delay		1.8								

Step 1: MOVEMENT PRIORITIES

Major Street: Approach Priority Movement	1U U	NorthBound 1 L	2 T	3 R	4U U	SouthBound 4 L	5 T	6 R
Minor Street:								

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:								
Approach Movement	1U	NorthBound			4U	SouthBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Volume, V_x		22	74			115	28	
Flow Rate, v_x		24	80			125	30	

Minor Street:								
Approach Movement		WestBound				EastBound		
		7	8	9		10	11	12
		L	T	R		L	T	R
Volume, V_x						15	18	
Flow Rate, v_x						16	20	

Step 3: CONFLICTING FLOW RATES

Major Street:								
Approach Movement	1U	NorthBound			4U	SouthBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Flow Rate, v_x		24	80			125	30	
Conflicting Flow, v_c, x		155						

Minor Street:								
Approach Movement		WestBound				EastBound		
		7	8	9		10	11	12
		L	T	R		L	T	R
Flow Rate, v_x						16	20	
Conflicting Flow, v_c, x						268	140	

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS										
Approach Movement		NB		SB		WestBound			EastBound	
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_c, base										
Single Stage		4.1						7.1		6.2
Stage I										
Stage II										
t_c, HV		1.0						1.0		1.0
P_HV		0.00						0.00		0.03
t_c, G								0.2		0.1
G								0		0
t_3, LT		0.0						0.7		0.0
t_c										
Single Stage		4.10						6.40		6.23
Stage I										
Stage II										

FOLLOW-UP HEADWAYS										
Approach Movement		NB		SB		WestBound			EastBound	
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_f, base		2.2						3.5		3.3
t_f, HV		0.9						0.9		0.9
P_HV		0.00						0.00		0.03
t_f		2.20						3.50		3.33

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach Movement		NB		SB		WestBound			EastBound	
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
v_c, x		155						268		140
t_c, x		4.10						6.40		6.23
t_f, x		2.20						3.50		3.33
c_p, x		1437						725		905

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate v_x Lane Width, w Walking Speed, S_p Pedestrian Blockage Factor, f_pb	0	0		0
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j Major L-Shared Probability Queue-free State, p*_0, j		155 1437 1.000 1437 0.983 0.983		
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j			140 905 1.000 905 0.978	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Capacity Adjustment Factor, f_x Movement Capacity, c_m, x Shared L/U Capacity, c_SH Probability of Queue-free State, p_0, j				
Minor-Street Through Movements		8	11	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Capacity Adjustment Factor, f_x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Major L, Minor T Adjusted Impedance Factor, p" Major L, Minor T Impedance Factor, p' Capacity Adjustment Factor, f_x Movement Capacity, c_m, x			268 725 1.000   0.983 712	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES											
Approach		7	WestBound	8	9		10	EastBound	11	12	
Movement									LR		
Lane Configuration											
Shared Flow Rate, v_y Movement Capacity, c_m, x Shared Capacity, c_SH							712	36	806	905	

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS															
Approach			NB	1	4U	SB	4	7	WestBound	8	9	10	EastBound	11	12
Movement															
Flow Rate Movement Capacity Lane Configuration Shared Capacity Control Delay				24 1437 LT 7.5								16 712			20 905
													LR 806 9.7		

CONTROL DELAY TO RANK 1 MOVEMENTS															
Approach								NB				SB			
Movement								2				5			
Number of Major Street Through Lanes, N Proportion of Rank 1 vehicles not blocked, p*_0, j Delay to Major Left-turning Vehicles, d_MLT Major Street Through Vehicles in Shared Lane, v_i1 Major Street Turning Vehicles in Shared Lane, v_i2 Saturation Flow Rate for Major Street Through, s_i1										1 0.983 7.5 80 24 1800			1 1800		



Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

0.1

1500

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement Lane Configuration	NB 1U	1 LT	4U SB	4	7	WestBound 8	9	10	EastBound 11	12
Flow Rate		24							36	
Lane Capacity		1437							806	
v/c		0.02							0.04	
95% Queue Length		0.1							0.1	
Control Delay		7.5							9.7	
LOS		A							A	
Approach Delay		1.8							9.7	
Approach LOS									A	
Intersction Delay		1.8								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 2:46:13 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_AM\_2w5-Rin-out\_Hillisdale-Walnut-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 2w & 5\_Rin-out\_AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Hillisdale Rd / Walnut Rd  
 Major Street Direction: East-West  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: Walnut Avenue  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Volume		1	72	22		12	103	4	
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		1	78	24		13	112	4	
Percent Heavy Vehicles		3				3			
Number of Lanes	0	0	1	0	0	0	1	0	
Lane Configuration			LTR				LTR		
Median Type					Undivided				
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street:	NorthBound			SouthBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume	22	5	20	4	5	5
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR	24	5	22	4	5	5
Percent Heavy Vehicles	3	3	3	3	3	3
Number of Lanes	0	1	0	0	1	0
Lane Configuration		LTR			LTR	
RT channelized?						
Flared Approach   Storage	No			No		
Percent Grade		0			0	

Pedestrian Volumes and Adjustments

Approach	EB	WB	NB	SB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	EB	WB	NorthBound			SouthBound		
Movement	1U	4U	7	8	9	10	11	12
Lane Configuration				LTR			LTR	
Flow Rate	1	13	51			15		
Lane Capacity	1466	1484	786			740		
v/c	0.00	0.01	0.06			0.02		
95% Queue Length	0.0	0.0	0.2			0.1		
Control Delay	7.5	7.4	9.9			10.0		
LOS	A	A	A			A		
Approach Delay	0.1	0.8	9.9			10.0		
Approach LOS			A			A		
Intersection Delay	2.6							

Step 1: MOVEMENT PRIORITIES

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Minor Street:									

Approach Priority Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x		1	72	22			12	103	4
Flow Rate, v_x		1	78	24			13	112	4

Minor Street: Approach Movement	NorthBound				SouthBound				
	7 L	8 T	9 R		10 L	11 T	12 R		
Volume, V_x		22	5	20			4	5	5
Flow Rate, v_x		24	5	22			4	5	5

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		1	78	24			13	112	4
Conflicting Flow, v_c, x		116					102		

Minor Street: Approach Movement	NorthBound				SouthBound				
	7 L	8 T	9 R		10 L	11 T	12 R		
Flow Rate, v_x		24	5	22			4	5	5
Conflicting Flow, v_c, x		238	235	90			246	245	114

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_c, base											
Single Stage		4.1		4.1	7.1	6.5	6.2	7.1	6.5	6.2	
Stage I											
Stage II											
t_c, HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	
P_HV		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1	
G					0	0	0	0	0	0	
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
t_c											
Single Stage		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23	
Stage I											
Stage II											

FOLLOW-UP HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_f, base											
t_f, HV		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3	
P_HV		0.9		0.9	0.9	0.9	0.9	0.9	0.9	0.9	
t_f		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
c_p, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound		
					7 L	8 T	9 R	10 L	11 T	12 R
v_c, x		116		102	238	235	90	246	245	114
t_c, x		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23
t_f, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33
c_p, x		1466		1484	714	664	965	705	656	936

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	FR	WR	NR	SB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate v_x Lane Width, w Walking Speed, S_p Pedestrian Blockage Factor, f_pb	0	0	0	0
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j Major L-Shared Probability Queue-free State, p*_0, j		116 1466 1.000 1466 0.999 0.999	102 1484 1.000 1484 0.991 0.991	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j		90 965 1.000 965 0.977	114 936 1.000 936 0.994	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Capacity Adjustment Factor, f_x Movement Capacity, c_m, x Shared L/U Capacity, c_SH Probability of Queue-free State, p_0, j				
Minor-Street Through Movements		8	11	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Capacity Adjustment Factor, f_x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j		235 664 1.000 0.990 657 0.992	245 656 1.000 0.990 649 0.992	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Major L, Minor T Adjusted Impedance Factor, p" Major L, Minor T Impedance Factor, p' Capacity Adjustment Factor, f_x Movement Capacity, c_m, x		238 714 1.000 0.982 0.986 0.980 700	246 705 1.000 0.982 0.986 0.964 680	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	NorthBound			SouthBound		
Movement	7	8	9	10	11	12
Lane Configuration		LTR			LTR	
Shared Flow Rate, v_y		51			15	
Movement Capacity, c_m, x	700	657	965	680	649	936
Shared Capacity, c_SH		786			740	

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		1		13	24	5	22	4	5	5
Movement Capacity		1466		1484	700	657	965	680	649	936
Lane Configuration						LTR			LTR	
Shared Capacity						786			740	
Control Delay		7.5		7.4		9.9			10.0	

CONTROL DELAY TO RANK 1 MOVEMENTS		
Approach	EB	WB
Movement	2	5
Number of Major Street Through Lanes, N	1	1
Proportion of Rank 1 vehicles not blocked, p*_0, j	0.999	0.991
Delay to Major Left-turning Vehicles, d_MLT	7.5	7.4
Major Street Through Vehicles in Shared Lane, v_i1	78	112
Major Street Turning Vehicles in Shared Lane, v_i2	25	17
Saturation Flow Rate for Major Street Through, s_i1	1800	1800

Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

1500  
 0.0

1500  
 0.1

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement Lane Configuration	1U	EB 1	4U	WB 4	7	NorthBound 8	9	10	SouthBound 11	12
						LTR			LTR	
Flow Rate		1		13		51			15	
Lane Capacity		1466		1484		786			740	
v/c		0.00		0.01		0.06			0.02	
95% Queue Length		0.0		0.0		0.2			0.1	
Control Delay		7.5		7.4		9.9			10.0	
LOS		A		A		A			A	
Approach Delay		0.1		0.8		9.9			10.0	
Approach LOS						A			A	
Intersction Delay		2.6								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 2:51:08 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_PM\_2w5-Rin-out\_Hillisdale-Walnut-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 2w5\_Rin-out PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Hillisdale Rd / Walnut Rd  
 Major Street Direction: East-West  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: Walnut Avenue  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	1U U	EastBound 1 L 2 T	3 R	4U U	WestBound 4 L 5 T	6 R
Volume		12	143	22	9	85
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR		13	155	24	10	92
Percent Heavy Vehicles		3			3	
Number of Lanes	0	0	1	0	0	1
Lane Configuration			LTR			LTR
Median Type				Undivided		
Median Storage						
RT channelized?						
Left-Turn Lane Storage						
Upstream Signal?				Not Present		

Minor Street: Approach Movement	NorthBound 7 L 8 T	9 R	SouthBound 10 L 11 T	12 R
Volume	28	9	30	6
Peak Hour Factor, PHF			0.92	
Hourly Flow Rate, HFR	30	10	33	7
Percent Heavy Vehicles	3	3	3	3
Number of Lanes	0	1	0	0
Lane Configuration		LTR		LTR
RT channelized?				
Flared Approach   Storage	No		No	
Percent Grade		0		0

Pedestrian Volumes and Adjustments

Approach Movement	EB 13	WB 14	NB 15	SB 16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach Movement	1U U	EB 1	4U 4	WB 4	7 7	NorthBound 8 LTR	9	10	SouthBound 11 LTR	12
Lane Configuration						LTR			LTR	
Flow Rate		13		10		73			17	
Lane Capacity		1469		1390		701			623	
v/c		0.01		0.01		0.10			0.03	
95% Queue Length		0.0		0.0		0.3			0.1	
Control Delay		7.5		7.6		10.7			10.9	
LOS		A		A		B			B	
Approach Delay		0.6		0.7		10.7			10.9	
Approach LOS						B			B	
Intersection Delay		2.9								

Step 1: MOVEMENT PRIORITIES

Major Street: Approach Priority Movement	1U U	EastBound 1 L 2 T	3 R	4U U	WestBound 4 L 5 T	6 R
Minor Street:						

Approach Priority Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x		12	143	22		9	85	20	
Flow Rate, v_x		13	155	24		10	92	22	

Minor Street: Approach Movement	NorthBound				SouthBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Volume, V_x		28	9	30		6	7	3
Flow Rate, v_x		30	10	33		7	8	3

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		13	155	24		10	92	22	
Conflicting Flow, v_c, x		114				179			

Minor Street: Approach Movement	NorthBound				SouthBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Flow Rate, v_x		30	10	33		7	8	3
Conflicting Flow, v_c, x		322	327	167		338	328	103

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_c, base											
Single Stage		4.1		4.1	7.1	6.5	6.2	7.1	6.5	6.2	
Stage I											
Stage II											
t_c, HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	
P_HV		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1	
G					0	0	0	0	0	0	
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
t_c											
Single Stage		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23	
Stage I											
Stage II											

FOLLOW-UP HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_f, base											
t_f, HV		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3	
P_HV		0.9		0.9	0.9	0.9	0.9	0.9	0.9	0.9	
t_f		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
c_p, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound		
					7 L	8 T	9 R	10 L	11 T	12 R
v_c, x		114		179	322	327	167	338	328	103
t_c, x		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23
t_f, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33
c_p, x		1469		1390	629	590	874	614	589	949

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	FR	WR	NR	SB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate v_x	0	0	0	0
Lane Width, w				
Walking Speed, S_p				
Pedestrian Blockage Factor, f_pb				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, v_c, x		114	179	
Potential Capacity, c_p, x		1469	1390	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Movement Capacity, c_m, x		1469	1390	
Probability of Queue-free State, p_0, j		0.991	0.993	
Major L-Shared Probability Queue-free State, p*_0, j		0.990	0.992	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, v_c, x		167	103	
Potential Capacity, c_p, x		874	949	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Movement Capacity, c_m, x		874	949	
Probability of Queue-free State, p_0, j		0.963	0.997	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, v_c, x				
Potential Capacity, c_p, x				
Capacity Adjustment Factor, f_x				
Movement Capacity, c_m, x				
Shared L/U Capacity, c_SH				
Probability of Queue-free State, p_0, j				
Minor-Street Through Movements		8	11	
Conflicting Flow, v_c, x		327	328	
Potential Capacity, c_p, x		590	589	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Capacity Adjustment Factor, f_x		0.983	0.983	
Movement Capacity, c_m, x		579	579	
Probability of Queue-free State, p_0, j		0.983	0.987	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, v_c, x		322	338	
Potential Capacity, c_p, x		629	614	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, p"		0.970	0.966	
Major L, Minor T Impedance Factor, p'		0.977	0.974	
Capacity Adjustment Factor, f_x		0.974	0.938	
Movement Capacity, c_m, x		613	576	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	NorthBound			SouthBound		
Movement	7	8	9	10	11	12
Lane Configuration		LTR			LTR	
Shared Flow Rate, v_y		73			17	
Movement Capacity, c_m, x	613	579	874	576	579	949
Shared Capacity, c_SH		701			623	

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		13		10	30	10	33	7	8	3
Movement Capacity		1469		1390	613	579	874	576	579	949
Lane Configuration						LTR			LTR	
Shared Capacity						701			623	
Control Delay		7.5		7.6		10.7			10.9	

CONTROL DELAY TO RANK 1 MOVEMENTS		
Approach	EB	WB
Movement	2	5
Number of Major Street Through Lanes, N	1	1
Proportion of Rank 1 vehicles not blocked, p*_0, j	0.990	0.992
Delay to Major Left-turning Vehicles, d_MLT	7.5	7.6
Major Street Through Vehicles in Shared Lane, v_i1	155	92
Major Street Turning Vehicles in Shared Lane, v_i2	37	32
Saturation Flow Rate for Major Street Through, s_i1	1800	1800



Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

1500  
 0.1

1500  
 0.1

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement Lane Configuration	1U	EB 1	4U	WB 4	7	NorthBound 8	9	10	SouthBound 11	12
						LTR			LTR	
Flow Rate		13		10		73			17	
Lane Capacity		1469		1390		701			623	
v/c		0.01		0.01		0.10			0.03	
95% Queue Length		0.0		0.0		0.3			0.1	
Control Delay		7.5		7.6		10.7			10.9	
LOS		A		A		B			B	
Approach Delay		0.6		0.7		10.7			10.9	
Approach LOS						B			B	
Intersction Delay		2.9								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 2:52:55 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_AM\_2w5-Rin-out-Hillisdale-OSR-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 2w & 5\_R-in-out AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Hillisdale Rd / Old State  
 Major Street Direction: North-South  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: Old State Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound				SouthBound			
Approach	1U	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R
Volume			110	10		9	60	
Peak Hour Factor, PHF					0.92			
Hourly Flow Rate, HFR			120	11		10	65	
Percent Heavy Vehicles						3		
Number of Lanes	0	0	1	0	0	0	1	0
Lane Configuration				TR		LT		
Median Type					Undivided			
Median Storage								
RT channelized?								
Left-Turn Lane Storage								
Upstream Signal?					Not Present			

Minor Street:	WestBound			EastBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume	31		4			
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR	34		4			
Percent Heavy Vehicles	3		3			
Number of Lanes	0	1	0	0	0	0
Lane Configuration		LR				
RT channelized?						
Flared Approach   Storage	No					
Percent Grade		0				

Pedestrian Volumes and Adjustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	NB	SB	WestBound	EastBound
Movement	1U	4U	7	8
Lane Configuration		LT	LR	
Flow Rate		10	38	
Lane Capacity		1449	786	
v/c		0.01	0.05	
95% Queue Length		0.0	0.2	
Control Delay		7.5	9.8	
LOS		A	A	
Approach Delay		1.0	9.8	
Approach LOS			A	
Intersection Delay	1.8			

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound				SouthBound			
Approach	1U	2	3	4U	4	5	6	
Priority	U	L	T	R	U	L	T	R

Minor Street:

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:								
Approach Movement	1U	NorthBound			4U	SouthBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Volume, V_x		110				9		60
Flow Rate, v_x		120				10		65

Minor Street:								
Approach Movement	WestBound				EastBound			
		7	8	9		10	11	12
		L	T	R		L	T	R
Volume, V_x		31				4		
Flow Rate, v_x		34				4		

Step 3: CONFLICTING FLOW RATES

Major Street:								
Approach Movement	1U	NorthBound			4U	SouthBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Flow Rate, v_x		120				10		65
Conflicting Flow, v_c, x						130		

Minor Street:								
Approach Movement	WestBound				EastBound			
		7	8	9		10	11	12
		L	T	R		L	T	R
Flow Rate, v_x		34				4		
Conflicting Flow, v_c, x		210				125		

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_c, base										
Single Stage				4.1	7.1		6.2			
Stage I										
Stage II										
t_c, HV				1.0	1.0		1.0			
P_HV				0.03	0.03		0.03			
t_c, G					0.2		0.1			
G					0		0			
t_3, LT				0.0	0.7		0.0			
t_c										
Single Stage				4.13	6.43		6.23			
Stage I										
Stage II										

FOLLOW-UP HEADWAYS										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_f, base				2.2	3.5		3.3			
t_f, HV				0.9	0.9		0.9			
P_HV				0.03	0.03		0.03			
t_f				2.23	3.53		3.33			

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
v_c, x				130	210		125			
t_c, x				4.13	6.43		6.23			
t_f, x				2.23	3.53		3.33			
c_p, x				1449	776		923			

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$ Lane Width, $w$ Walking Speed, $S_p$ Pedestrian Blockage Factor, $f_{pb}$	0	0	0	
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$ Major L-Shared Probability Queue-free State, $p^*_{0,j}$			130 1449 1.000 1449 0.993 0.993	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$		125 923 1.000 923 0.995		
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Shared L/U Capacity, $c_{SH}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Major L, Minor T Adjusted Impedance Factor, $p''$ Major L, Minor T Impedance Factor, $p'$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$		210 776 1.000   0.993 771		

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES											
Approach	WestBound				EastBound						
Movement	7	8	9	10	11	12					
Lane Configuration		LR									
Shared Flow Rate, $v_y$		38									
Movement Capacity, $c_{m,x}$	771		923								
Shared Capacity, $c_{SH}$		786									

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS												
Approach	NB				SB							
	1U	1	4U	4	7	8	9	10	11	12		
Movement						LR						
Flow Rate				10	34		4					
Movement Capacity				1449	771		923					
Lane Configuration				LT		LR						
Shared Capacity						786						
Control Delay				7.5		9.8						

CONTROL DELAY TO RANK 1 MOVEMENTS												
Approach					NB				SB			
Movement					2				5			
Number of Major Street Through Lanes, $N$					1				1			
Proportion of Rank 1 vehicles not blocked, $p^*_{0,j}$									0.993			
Delay to Major Left-turning Vehicles, $d_{MLT}$									7.5			
Major Street Through Vehicles in Shared Lane, $v_{i1}$									65			
Major Street Turning Vehicles in Shared Lane, $v_{i2}$									10			
Saturation Flow Rate for Major Street Through, $s_{i1}$					1800				1800			

Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

1500

0.1

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement	1U	NB 1	4U	SB 4	7	WestBound 8	9	10	EastBound 11	12
Lane Configuration				LT		LR				
Flow Rate				10		38				
Lane Capacity				1449		786				
v/c				0.01		0.05				
95% Queue Length				0.0		0.2				
Control Delay				7.5		9.8				
LOS				A		A				
Approach Delay				1.0		9.8				
Approach LOS						A				
Intersection Delay		1.8								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 3:13:24 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_PM\_2w5-Rin-out-Hillisdale-OSR-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 2w & 5 Rin-out PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Hillisdale Rd / Old State  
 Major Street Direction: North-South  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: Old State Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound						SouthBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Volume			120	4		0	123		
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR			130	4		0	134		
Percent Heavy Vehicles						3			
Number of Lanes	0	0	1	0	0	0	1	0	
Lane Configuration				TR		LT			
Median Type					Undivided				
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street:	WestBound			EastBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume	8		0			
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR	9		0			
Percent Heavy Vehicles	3		3			
Number of Lanes	0	1	0	0	0	0
Lane Configuration		LR				
RT channelized?						
Flared Approach   Storage	No					
Percent Grade		0				

Pedestrian Volumes and Adjustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f <sub>pb</sub>				

Delay, Queue Length, and Level of Service

Approach	NB	SB	WestBound	EastBound
Movement	1	4U	7	10
Lane Configuration		LT	LR	
Flow Rate		0	9	
Lane Capacity		1443	721	
v/c		0.00	0.01	
95% Queue Length		0.0	0.0	
Control Delay		7.5	10.1	
LOS		A	B	
Approach Delay		0.0	10.1	
Approach LOS			B	
Intersection Delay	0.3			

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound						SouthBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Priority									

Minor Street:

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement	1U	NorthBound				4U	SouthBound		
	U	1	2	3		U	4	5	6
		L	T	R			L	T	R
Volume, V_x			120	4			0	123	
Flow Rate, v_x			130	4			0	134	

Minor Street:									
Approach Movement	WestBound				EastBound				
		7	8	9			10	11	12
		L	T	R			L	T	R
Volume, V_x		8		0					
Flow Rate, v_x		9		0					

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement	1U	NorthBound				4U	SouthBound		
	U	1	2	3		U	4	5	6
		L	T	R			L	T	R
Flow Rate, v_x			130	4			0	134	
Conflicting Flow, v_c, x							135		

Minor Street:									
Approach Movement	WestBound				EastBound				
		7	8	9			10	11	12
		L	T	R			L	T	R
Flow Rate, v_x		9		0					
Conflicting Flow, v_c, x		266		133					

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_c, base										
Single Stage				4.1	7.1		6.2			
Stage I										
Stage II										
t_c, HV				1.0	1.0		1.0			
P_HV				0.03	0.03		0.03			
t_c, G					0.2		0.1			
G					0		0			
t_3, LT				0.0	0.7		0.0			
t_c										
Single Stage				4.13	6.43		6.23			
Stage I										
Stage II										

FOLLOW-UP HEADWAYS										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_f, base				2.2	3.5		3.3			
t_f, HV				0.9	0.9		0.9			
P_HV				0.03	0.03		0.03			
t_f				2.23	3.53		3.33			

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
v_c, x				135	266		133			
t_c, x				4.13	6.43		6.23			
t_f, x				2.23	3.53		3.33			
c_p, x				1443	721		914			

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$			135	
Potential Capacity, $c_{p,x}$			1443	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Movement Capacity, $c_{m,x}$			1443	
Probability of Queue-free State, $p_{0,j}$			1.000	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$			1.000	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		133		
Potential Capacity, $c_{p,x}$		914		
Pedestrian Impedance Factor, $p_{p,x}$		1.000		
Movement Capacity, $c_{m,x}$		914		
Probability of Queue-free State, $p_{0,j}$		1.000		
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$		266		
Potential Capacity, $c_{p,x}$		721		
Pedestrian Impedance Factor, $p_{p,x}$		1.000		
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$		1.000		
Movement Capacity, $c_{m,x}$		721		

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES											
Approach	WestBound				EastBound						
Movement	7	8	9	10	11	12					
Lane Configuration		LR									
Shared Flow Rate, $v_y$		9									
Movement Capacity, $c_{m,x}$	721		914								
Shared Capacity, $c_{SH}$		721									

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS												
Approach	NB				SB				WestBound			
Movement	1U	1	4U	4	7	8	9	10	11	12		
Flow Rate	0				9		0					
Movement Capacity	1443				721		914					
Lane Configuration	LT					LR						
Shared Capacity						721						
Control Delay	7.5					10.1						

CONTROL DELAY TO RANK 1 MOVEMENTS												
Approach					NB				SB			
Movement					2				5			
Number of Major Street Through Lanes, $N$					1				1			
Proportion of Rank 1 vehicles not blocked, $p^*_{0,j}$									1.000			
Delay to Major Left-turning Vehicles, $d_{MLT}$									7.5			
Major Street Through Vehicles in Shared Lane, $v_{i1}$									134			
Major Street Turning Vehicles in Shared Lane, $v_{i2}$									0			
Saturation Flow Rate for Major Street Through, $s_{i1}$					1800				1800			



Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

1500

0.0

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement	1U	NB 1	4U	SB 4	7	WestBound 8	9	10	EastBound 11	12
Lane Configuration				LT		LR				
Flow Rate				0		9				
Lane Capacity				1443		721				
v/c				0.00		0.01				
95% Queue Length				0.0		0.0				
Control Delay				7.5		10.1				
LOS				A		B				
Approach Delay				0.0		10.1				
Approach LOS						B				
Intersection Delay		0.3								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/19/2019 3:16:05 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_AM\_J-Turn\_5Rin-out\_Hill-US 41x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: J-Turn\_5Rin-out AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Hillisdale Rd / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound						SouthBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Volume	0	11	631	25	0	33	888	0	
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		12	686	27		36	965	0	
Percent Heavy Vehicles	3	3			3	3			
Number of Lanes	0	1	2	0	0	1	2	0	
Lane Configuration		L	T	TR		L	T	TR	
Median Type	Undivided								
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?	Not Present								

Minor Street:	WestBound			EastBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume			148			36
Peak Hour Factor, PHF			0.92			
Hourly Flow Rate, HFR			161			39
Percent Heavy Vehicles			3			3
Number of Lanes	0	0	1	0	0	1
Lane Configuration			R			R
RT channelized?			No			No
Flared Approach   Storage						
Percent Grade			0			0

Pedestrian Volumes and Adjustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	NB	SB	WestBound	EastBound
Movement	1	4U	7	10
Lane Configuration	L	L	R	R
Flow Rate	12	36	161	39
Lane Capacity	703	876	637	527
v/c	0.02	0.04	0.25	0.07
95% Queue Length	0.1	0.1	1.0	0.2
Control Delay	10.2	9.3	12.5	12.4
LOS	B	A	B	B
Approach Delay	0.2	0.3	12.5	12.4
Approach LOS			B	B
Intersection Delay	1.5			

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound						SouthBound		
Approach	1U	1	2	3	4U	4	5	6	
Priority	U	L	T	R	U	L	T	R	

Minor Street:

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement	1U	NorthBound				4U	SouthBound		
	U	1	2	3		U	4	5	6
		L	T	R			L	T	R
Volume, V_x	0	11	631	25		0	33	888	0
Flow Rate, v_x		12	686	27			36	965	0

Minor Street:								
Approach Movement	WestBound				EastBound			
	7	8	9		10	11	12	
	L	T	R		L	T	R	
Volume, V_x				148				36
Flow Rate, v_x				161				39

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement	1U	NorthBound				4U	SouthBound		
	U	1	2	3		U	4	5	6
		L	T	R			L	T	R
Flow Rate, v_x		12	686	27			36	965	0
Conflicting Flow, v_c, x		965					713		

Minor Street:								
Approach Movement	WestBound				EastBound			
	7	8	9		10	11	12	
	L	T	R		L	T	R	
Flow Rate, v_x				161				39
Conflicting Flow, v_c, x				357				483

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS										
Approach Movement	NB	SB			WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_c, base		4.1		4.1			6.9			6.9
Single Stage										
Stage I										
Stage II										
t_c, HV		2.0		2.0			2.0			2.0
P_HV		0.03		0.03			0.03			0.03
t_c, G							0.1			0.1
G							0			0
t_3, LT		0.0		0.0			0.0			0.0
t_c										
Single Stage		4.16		4.16			6.96			6.96
Stage I										
Stage II										

FOLLOW-UP HEADWAYS										
Approach Movement	NB	SB			WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_f, base		2.2		2.2			3.3			3.3
t_f, HV		1.0		1.0			1.0			1.0
P_HV		0.03		0.03			0.03			0.03
t_f		2.23		2.23			3.33			3.33

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach Movement	NB	SB			WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
v_c, x		965		713			357			483
t_c, x		4.16		4.16			6.96			6.96
t_f, x		2.23		2.23			3.33			3.33
c_p, x		703		876			637			527

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		965	713	
Potential Capacity, $c_{p,x}$		703	876	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		703	876	
Probability of Queue-free State, $p_{0,j}$		0.983	0.959	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		357	483	
Potential Capacity, $c_{p,x}$		637	527	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		637	527	
Probability of Queue-free State, $p_{0,j}$		0.747	0.926	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES											
Approach		WestBound				EastBound					
Movement	7	8	9		10	11	12				
Lane Configuration			R				R				
Shared Flow Rate, $v_y$			161				39				
Movement Capacity, $c_{m,x}$			637				527				
Shared Capacity, $c_{SH}$											

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS											
Approach		NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12	
Flow Rate		12		36			161			39	
Movement Capacity		703		876			637			527	
Lane Configuration		L		L			R			R	
Shared Capacity											
Control Delay		10.2		9.3			12.5			12.4	

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach		NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12	
Lane Configuration		L		L			R			R	
Flow Rate		12		36			161			39	
Lane Capacity		703		876			637			527	
$v/c$		0.02		0.04			0.25			0.07	
95% Queue Length		0.1		0.1			1.0			0.2	
Control Delay		10.2		9.3			12.5			12.4	

LOS	B	A		B		
Approach Delay	0.2	0.3	12.5		12.4	
Approach LOS			B		B	
Intersection Delay	1.5					

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/20/2019 10:25:04 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_AM\_J-Turn\_5Rin-out\_Hill-US 41x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: J-Turn\_5Rin-out\_Jadd AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Hillisdale Rd / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:									
Approach	NorthBound				SouthBound				
Movement	1U	2	3		4U	5	6		
	U	L	T	R	U	L	T	R	
Volume	0	11	631	47	0	33	987	8	
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		12	686	51		36	1073	9	
Percent Heavy Vehicles	3	3			3	3			
Number of Lanes	0	1	2	0	0	1	2	0	
Lane Configuration		L	T	TR		L	T	TR	
Median Type	Undivided								
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?	Not Present								

Minor Street:									
Approach	WestBound				EastBound				
Movement	7	8	9		10	11	12		
	L	T	R		L	T	R		
Volume				148					36
Peak Hour Factor, PHF				0.92					
Hourly Flow Rate, HFR				161					39
Percent Heavy Vehicles				3					3
Number of Lanes	0	0		1	0	0			1
Lane Configuration				R					R
RT channelized?				No					No
Flared Approach   Storage									
Percent Grade			0				0		

Pedestrian Volumes and Adjustments

Approach	NB				SB				WB				EB			
Movement	13				14				15				16			
Flow (ped/hr)	0				0				0				0			
Lane Width (ft)																
Walking Speed (ft/sec)																
Pedestrian Blockage Factor, f_pb																

Delay, Queue Length, and Level of Service

Approach	NB			SB			WestBound			EastBound		
Movement	1U	4U	7	8	9	10	11	12				
Lane Configuration	L	L	R	R	R	L	T	R				
Flow Rate	12	36	161	39								
Lane Capacity	635	858	626	483								
v/c	0.02	0.04	0.26	0.08								
95% Queue Length	0.1	0.1	1.0	0.3								
Control Delay	10.8	9.4	12.7	13.1								
LOS	B	A	B	B								
Approach Delay	0.2	0.3	12.7	13.1								
Approach LOS			B	B								
Intersection Delay	1.5											

Step 1: MOVEMENT PRIORITIES

Major Street:									
Approach	NorthBound				SouthBound				
Priority	1U	2	3		4U	5	6		
Movement	U	L	T	R	U	L	T	R	
Minor Street:									

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement		NorthBound				SouthBound			
	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Volume, V_x	0	11	631	47		0	33	987	8
Flow Rate, v_x		12	686	51			36	1073	9

Minor Street:									
Approach Movement		WestBound				EastBound			
		7	8	9			10	11	12
		L	T	R			L	T	R
Volume, V_x				148					36
Flow Rate, v_x				161					39

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement		NorthBound				SouthBound			
	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Flow Rate, v_x		12	686	51			36	1073	9
Conflicting Flow, v_c, x		1082					737		

Minor Street:									
Approach Movement		WestBound				EastBound			
		7	8	9			10	11	12
		L	T	R			L	T	R
Flow Rate, v_x				161					39
Conflicting Flow, v_c, x				368					541

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS										
Approach Movement		NB		SB		WestBound			EastBound	
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_c, base										
Single Stage		4.1		4.1			6.9			6.9
Stage I										
Stage II										
t_c, HV		2.0		2.0			2.0			2.0
P_HV		0.03		0.03			0.03			0.03
t_c, G							0.1			0.1
G							0			0
t_3, LT		0.0		0.0			0.0			0.0
t_c										
Single Stage		4.16		4.16			6.96			6.96
Stage I										
Stage II										

FOLLOW-UP HEADWAYS										
Approach Movement		NB		SB		WestBound			EastBound	
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_f, base										
t_f, HV		2.2		2.2			3.3			3.3
P_HV		1.0		1.0			1.0			1.0
t_f		0.03		0.03			0.03			0.03
c_p, x		2.23		2.23			3.33			3.33

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach Movement		NB		SB		WestBound			EastBound	
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
v_c, x		1082		737			368			541
t_c, x		4.16		4.16			6.96			6.96
t_f, x		2.23		2.23			3.33			3.33
c_p, x		635		858			626			483

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach				NR	SR	WR	EB
-------------------------------	--	--	--	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		1082	737	
Potential Capacity, $c_{p,x}$		635	858	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		635	858	
Probability of Queue-free State, $p_{0,j}$		0.981	0.958	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		368	541	
Potential Capacity, $c_{p,x}$		626	483	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		626	483	
Probability of Queue-free State, $p_{0,j}$		0.743	0.919	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	WestBound			EastBound		
Movement	7	8	9	10	11	12
Lane Configuration			R			R
Shared Flow Rate, $v_y$			161			39
Movement Capacity, $c_{m,x}$			626			483
Shared Capacity, $c_{SH}$						

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		12		36			161			39
Movement Capacity		635		858			626			483
Lane Configuration		L		L			R			R
Shared Capacity										
Control Delay		10.8		9.4			12.7			13.1

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration		L		L			R			R
Flow Rate		12		36			161			39
Lane Capacity		635		858			626			483
$v/c$		0.02		0.04			0.26			0.08
95% Queue Length		0.1		0.1			1.0			0.3
Control Delay		10.8		9.4			12.7			13.1



LOS	B	A		B	
Approach Delay	0.2	0.3	12.7		13.1
Approach LOS			B		B
Intersection Delay	1.5				

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/20/2019 10:39:41 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_AM\_J-Turn\_Hill-US 41x. xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: J-Turn\_AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Hillisdale Rd / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound						SouthBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Volume	0	11	631	47	0	33	984	8	
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		12	686	51		36	1070	9	
Percent Heavy Vehicles	3	3			3	3			
Number of Lanes	0	1	2	0	0	1	2	0	
Lane Configuration		L	T	TR		L	T	TR	
Median Type	Undivided								
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?	Not Present								

Minor Street:	WestBound			EastBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume			145			36
Peak Hour Factor, PHF			0.92			
Hourly Flow Rate, HFR			158			39
Percent Heavy Vehicles			3			3
Number of Lanes	0	0	1	0	0	1
Lane Configuration			R			R
RT channelized?			No			No
Flared Approach   Storage						
Percent Grade			0			0

Pedestrian Volumes and Adjustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	NB	SB	WestBound	EastBound
Movement	1U	4U	7	10
Lane Configuration	L	L	R	R
Flow Rate	12	36	158	39
Lane Capacity	637	858	626	484
v/c	0.02	0.04	0.25	0.08
95% Queue Length	0.1	0.1	1.0	0.3
Control Delay	10.8	9.4	12.7	13.1
LOS	B	A	B	B
Approach Delay	0.2	0.3	12.7	13.1
Approach LOS			B	B
Intersection Delay	1.4			

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound						SouthBound		
Approach	1U	1	2	3	4U	4	5	6	
Priority	U	L	T	R	U	L	T	R	

Minor Street:

Approach	WestBound					EastBound		
Priority	7	8	9			10	11	12
Movement	L	T	R			L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach	NorthBound					SouthBound			
Movement	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Volume, V_x	0	11	631	47		0	33	984	8
Flow Rate, v_x		12	686	51			36	1070	9

Minor Street:									
Approach	WestBound					EastBound			
Movement		7	8	9			10	11	12
		L	T	R			L	T	R
Volume, V_x				145					36
Flow Rate, v_x				158					39

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach	NorthBound					SouthBound			
Movement	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Flow Rate, v_x		12	686	51			36	1070	9
Conflicting Flow, v_c, x		1078					737		

Minor Street:									
Approach	WestBound					EastBound			
Movement		7	8	9			10	11	12
		L	T	R			L	T	R
Flow Rate, v_x				158					39
Conflicting Flow, v_c, x				368					539

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS										
Approach	NB		SB			WestBound		EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_c, base		4.1		4.1			6.9			6.9
Single Stage										
Stage I										
Stage II										
t_c, HV		2.0		2.0			2.0			2.0
P_HV		0.03		0.03			0.03			0.03
t_c, G							0.1			0.1
G							0			0
t_3, LT		0.0		0.0			0.0			0.0
t_c										
Single Stage		4.16		4.16			6.96			6.96
Stage I										
Stage II										

FOLLOW-UP HEADWAYS										
Approach	NB		SB			WestBound		EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_f, base		2.2		2.2			3.3			3.3
t_f, HV		1.0		1.0			1.0			1.0
P_HV		0.03		0.03			0.03			0.03
t_f		2.23		2.23			3.33			3.33

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach	NB		SB			WestBound		EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
v_c, x		1078		737			368			539
t_c, x		4.16		4.16			6.96			6.96
t_f, x		2.23		2.23			3.33			3.33
c_p, x		637		858			626			484

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		1078	737	
Potential Capacity, $c_{p,x}$		637	858	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		637	858	
Probability of Queue-free State, $p_{0,j}$		0.981	0.958	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		368	539	
Potential Capacity, $c_{p,x}$		626	484	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		626	484	
Probability of Queue-free State, $p_{0,j}$		0.748	0.919	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	WestBound			EastBound		
Movement	7	8	9	10	11	12
Lane Configuration			R			R
Shared Flow Rate, $v_y$			158			39
Movement Capacity, $c_{m,x}$			626			484
Shared Capacity, $c_{SH}$						

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		12		36			158			39
Movement Capacity		637		858			626			484
Lane Configuration		L		L			R			R
Shared Capacity										
Control Delay		10.8		9.4			12.7			13.1

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
Movement		L		L			R			R
Lane Configuration										
Flow Rate		12		36			158			39
Lane Capacity		637		858			626			484
$v/c$		0.02		0.04			0.25			0.08
95% Queue Length		0.1		0.1			1.0			0.3
Control Delay		10.8		9.4			12.7			13.1

LOS	B	A		B	
Approach Delay	0.2	0.3	12.7		13.1
Approach LOS			B		B
Intersection Delay	1.4				

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/21/2019 2:55:21 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_AM\_J-Turn\_5Rin-out\_Hill-US 41x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: J-Turn\_5Rin-out AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Hillisdale Rd / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound						SouthBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Volume	0	14	791	32	0	42	1112	0	
Peak Hour Factor, PHF					0.92				
Hourly Flow Rtae, HFR		15	860	35		46	1209	0	
Percent Heavy Vehicles	3	3			3	3			
Number of Lanes	0	1	2	0	0	1	2	0	
Lane Configuration		L	T	TR		L	T	TR	
Median Type	Undivided								
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?	Not Present								

Minor Street:	WestBound			EastBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume			210			46
Peak Hour Factor, PHF			0.92			
Hourly Flow Rtae, HFR			228			50
Percent Heavy Vehicles			3			3
Number of Lanes	0	0	1	0	0	1
Lane Configuration			R			R
RT channelized?			No			No
Flared Approach   Storage						
Percent Grade		0			0	

Pedestrian Volumes and Adjustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	NB	SB	WestBound	EastBound
Movement	1	4U	7	10
Lane Configuration	L	L	R	R
Flow Rate	15	46	228	50
Lane Capacity	567	748	556	439
v/c	0.03	0.06	0.41	0.11
95% Queue Length	0.1	0.2	2.0	0.4
Control Delay	11.5	10.1	15.9	14.3
LOS	B	B	C	B
Approach Delay	0.2	0.4	15.9	14.3
Approach LOS			C	B
Intersction Delay	2.0			

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound						SouthBound		
Approach	1U	1	2	3	4U	4	5	6	
Priority	U	L	T	R	U	L	T	R	

Minor Street:

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	NorthBound				SouthBound			
	1U	2	3		4U	5	6	
	U	L	T		U	L	R	
Volume, V_x	0	14	791	32	0	42	1112	0
Flow Rate, v_x		15	860	35		46	1209	0

Minor Street: Approach Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Volume, V_x			210				46
Flow Rate, v_x			228				50

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	NorthBound				SouthBound			
	1U	2	3		4U	5	6	
	U	L	T		U	L	R	
Flow Rate, v_x		15	860	35		46	1209	0
Conflicting Flow, v_c, x		1209				895		

Minor Street: Approach Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Flow Rate, v_x			228				50
Conflicting Flow, v_c, x			447				604

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	NB			SB			WestBound		EastBound		
	1U	1	4U	4	7	8	9	10	11	12	
	U	L	U	L	L	T	R	L	T	R	
t_c, base Single Stage		4.1		4.1			6.9			6.9	
t_c, HV		2.0		2.0			2.0			2.0	
P_HV		0.03		0.03			0.03			0.03	
t_c, G							0.1			0.1	
G							0			0	
t_3, LT		0.0		0.0			0.0			0.0	
t_c Single Stage		4.16		4.16			6.96			6.96	
Stage I											
Stage II											

FOLLOW-UP HEADWAYS Approach Movement	NB			SB			WestBound		EastBound		
	1U	1	4U	4	7	8	9	10	11	12	
	U	L	U	L	L	T	R	L	T	R	
t_f, base		2.2		2.2			3.3			3.3	
t_f, HV		1.0		1.0			1.0			1.0	
P_HV		0.03		0.03			0.03			0.03	
t_f		2.23		2.23			3.33			3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	NB			SB			WestBound		EastBound		
	1U	1	4U	4	7	8	9	10	11	12	
	U	L	U	L	L	T	R	L	T	R	
v_c, x		1209		895			447			604	
t_c, x		4.16		4.16			6.96			6.96	
t_f, x		2.23		2.23			3.33			3.33	
c_p, x		567		748			556			439	

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	NR	SR	WR	EB
-------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		1209	895	
Potential Capacity, $c_{p,x}$		567	748	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		567	748	
Probability of Queue-free State, $p_{0,j}$		0.973	0.939	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		447	604	
Potential Capacity, $c_{p,x}$		556	439	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		556	439	
Probability of Queue-free State, $p_{0,j}$		0.589	0.886	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	WestBound			EastBound		
Movement	7	8	9	10	11	12
Lane Configuration			R			R
Shared Flow Rate, $v_y$			228			50
Movement Capacity, $c_{m,x}$			556			439
Shared Capacity, $c_{SH}$						

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		15		46			228			50
Movement Capacity		567		748			556			439
Lane Configuration		L		L			R			R
Shared Capacity										
Control Delay		11.5		10.1			15.9			14.3

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration		L		L			R			R
Flow Rate		15		46			228			50
Lane Capacity		567		748			556			439
$v/c$		0.03		0.06			0.41			0.11
95% Queue Length		0.1		0.2			2.0			0.4
Control Delay		11.5		10.1			15.9			14.3



LOS	B	B	C	
Approach Delay	0.2	0.4	15.9	14.3
Approach LOS			C	B
Intersection Delay	2.0			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/20/2019 10:31:02 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_AM\_J-Turn\_5Ri -o\_Jadd\_Hill -US 41x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: J-Turn\_5Ri -o\_Jadd AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Hillisdale Rd / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:									
Approach	NorthBound					SouthBound			
Movement	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Volume	0	14	791	60		0	42	1267	13
Peak Hour Factor, PHF					0.92				
Hourly Flow Rtae, HFR		15	860	65			46	1377	14
Percent Heavy Vehicles	3	3				3	3		
Number of Lanes	0	1	2	0		0	1	2	0
Lane Configuration		L	T	TR			L	T	TR
Median Type	Undivided								
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?	Not Present								

Minor Street:									
Approach	WestBound					EastBound			
Movement	7	8	9		10	11	12		
	L	T	R		L	T	R		
Volume				210					46
Peak Hour Factor, PHF					0.92				
Hourly Flow Rtae, HFR				228					50
Percent Heavy Vehicles				3					3
Number of Lanes	0	0		1		0	0		1
Lane Configuration				R					R
RT channelized?				No					No
Flared Approach   Storage									
Percent Grade			0				0		

Pedestrian Volumes and Adjustments

Approach				
Movement	NB	SB	WB	EB
	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach														
Movement	1U	NB	1	4U	SB	4	7	WestBound	8	9	10	EastBound	11	12
Lane Configuration	U		L	L		L		WestBound	R			EastBound	R	
Flow Rate			15			46				228				50
Lane Capacity			483			728				543				382
v/c			0.03			0.06				0.42				0.13
95% Queue Length			0.1			0.2				2.1				0.4
Control Delay			12.7			10.3				16.3				15.8
LOS			B			B				C				C
Approach Delay			0.2			0.3			16.3				15.8	
Approach LOS									C				C	
Intersction Delay			2.0											

Step 1: MOVEMENT PRIORITIES

Major Street:									
Approach	NorthBound					SouthBound			
Priority	1U	1	2	3		4U	4	5	6
Movement	U	L	T	R		U	L	T	R
Minor Street:									

Approach Priority Movement	WestBound					EastBound				
	7	8	9			10	11	12		
	L	T	R			L	T	R		

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:											
Approach Movement	1U	NorthBound					4U	SouthBound			
	U	1	2	3			U	4	5	6	
		L	T	R				L	T	R	
Volume, V_x	0	14	791	60			0	42	1267	13	
Flow Rate, v_x		15	860	65				46	1377	14	

Minor Street:										
Approach Movement	WestBound					EastBound				
			9					12		
			R					R		
Volume, V_x			210					46		
Flow Rate, v_x			228					50		

Step 3: CONFLICTING FLOW RATES

Major Street:											
Approach Movement	1U	NorthBound					4U	SouthBound			
	U	1	2	3			U	4	5	6	
		L	T	R				L	T	R	
Flow Rate, v_x		15	860	65				46	1377	14	
Conflicting Flow, v_c, x		1391						925			

Minor Street:										
Approach Movement	WestBound					EastBound				
			9					12		
			R					R		
Flow Rate, v_x			228					50		
Conflicting Flow, v_c, x			463					696		

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS												
Approach Movement	NB	SB					WestBound		EastBound			
	1U	1	4U	4	7	8	9	10	11	12		
	U	L	U	L	L	T	R	L	T	R		
t_c, base		4.1		4.1			6.9			6.9		
Single Stage												
Stage I												
Stage II												
t_c, HV		2.0		2.0			2.0			2.0		
P_HV		0.03		0.03			0.03			0.03		
t_c, G							0.1			0.1		
G							0			0		
t_3, LT		0.0		0.0			0.0			0.0		
t_c												
Single Stage		4.16		4.16			6.96			6.96		
Stage I												
Stage II												

FOLLOW-UP HEADWAYS												
Approach Movement	NB	SB					WestBound		EastBound			
	1U	1	4U	4	7	8	9	10	11	12		
	U	L	U	L	L	T	R	L	T	R		
t_f, base		2.2		2.2			3.3			3.3		
t_f, HV		1.0		1.0			1.0			1.0		
P_HV		0.03		0.03			0.03			0.03		
t_f		2.23		2.23			3.33			3.33		

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT												
Approach Movement	NB	SB					WestBound		EastBound			
	1U	1	4U	4	7	8	9	10	11	12		
	U	L	U	L	L	T	R	L	T	R		
v_c, x		1391		925			463			696		
t_c, x		4.16		4.16			6.96			6.96		
t_f, x		2.23		2.23			3.33			3.33		
c_p, x		483		728			543			382		

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		1391	925	
Potential Capacity, $c_{p,x}$		483	728	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		483	728	
Probability of Queue-free State, $p_{0,j}$		0.968	0.937	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		463	696	
Potential Capacity, $c_{p,x}$		543	382	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		543	382	
Probability of Queue-free State, $p_{0,j}$		0.580	0.869	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	WestBound			EastBound		
Movement	7	8	9	10	11	12
Lane Configuration			R			R
Shared Flow Rate, $v_y$			228			50
Movement Capacity, $c_{m,x}$			543			382
Shared Capacity, $c_{SH}$						

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		15		46			228			50
Movement Capacity		483		728			543			382
Lane Configuration		L		L			R			R
Shared Capacity										
Control Delay		12.7		10.3			16.3			15.8

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
Movement		L		L			R			R
Lane Configuration										
Flow Rate		15		46			228			50
Lane Capacity		483		728			543			382
$v/c$		0.03		0.06			0.42			0.13
95% Queue Length		0.1		0.2			2.1			0.4
Control Delay		12.7		10.3			16.3			15.8

LOS	B	B	C	
Approach Delay	0.2	0.3	16.3	15.8
Approach LOS			C	C
Intersection Delay	2.0			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/20/2019 10:56:38 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_PM\_J-Turn\_5Rin-out\_Hill-US 41x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: J-Turn\_5Rin-out PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Hillisdale Rd / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound						SouthBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Volume	0	20	958	42	0	53	1025	1	
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		22	1041	46		58	1114	1	
Percent Heavy Vehicles	3	3			3	3			
Number of Lanes	0	1	2	0	0	1	2	0	
Lane Configuration		L	T	TR		L	T	TR	
Median Type	Undivided								
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?	Not Present								

Minor Street:	WestBound			EastBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume			107			30
Peak Hour Factor, PHF			0.92			
Hourly Flow Rate, HFR			116			33
Percent Heavy Vehicles			3			3
Number of Lanes	0	0	1	0	0	1
Lane Configuration			R			R
RT channelized?			No			No
Flared Approach   Storage						
Percent Grade			0			0

Pedestrian Volumes and Adjustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	NB	SB	WestBound	EastBound
Movement	1U	4U	7	10
Lane Configuration	L	L	R	R
Flow Rate	22	58	116	33
Lane Capacity	616	632	481	471
v/c	0.04	0.09	0.24	0.07
95% Queue Length	0.1	0.3	0.9	0.2
Control Delay	11.1	11.3	14.9	13.2
LOS	B	B	B	B
Approach Delay	0.2	0.6	14.9	13.2
Approach LOS			B	B
Intersection Delay	1.3			

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound						SouthBound		
Approach	1U	1	2	3	4U	4	5	6	
Priority	U	L	T	R	U	L	T	R	

Minor Street:

Approach Priority Movement	WestBound				EastBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x	0	20	958	42		0	53	1025	1
Flow Rate, v_x		22	1041	46			58	1114	1

Minor Street: Approach Movement	WestBound				EastBound					
	7 L	8 T	9 R		10 L	11 T	12 R			
Volume, V_x									107	30
Flow Rate, v_x									116	33

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		22	1041	46			58	1114	1
Conflicting Flow, v_c, x		1115					1087		

Minor Street: Approach Movement	WestBound				EastBound					
	7 L	8 T	9 R		10 L	11 T	12 R			
Flow Rate, v_x									116	33
Conflicting Flow, v_c, x									543	558

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	1U U	NB	SB			WestBound			EastBound		
		1 L	4U U	4 L	7 L	8 T	9 R	10 L	11 T	12 R	
t_c, base Single Stage Stage I Stage II		4.1		4.1			6.9			6.9	
t_c, HV P_HV		2.0 0.03		2.0 0.03			2.0 0.03			2.0 0.03	
t_c, G G							0.1 0			0.1 0	
t_3, LT t_c		0.0		0.0			0.0			0.0	
Single Stage Stage I Stage II		4.16		4.16			6.96			6.96	

FOLLOW-UP HEADWAYS Approach Movement	1U U	NB	SB			WestBound			EastBound		
		1 L	4U U	4 L	7 L	8 T	9 R	10 L	11 T	12 R	
t_f, base t_f, HV P_HV		2.2 1.0 0.03		2.2 1.0 0.03			3.3 1.0 0.03			3.3 1.0 0.03	
t_f		2.23		2.23			3.33			3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	1U U	NB	SB			WestBound			EastBound		
		1 L	4U U	4 L	7 L	8 T	9 R	10 L	11 T	12 R	
v_c, x		1115		1087			543			558	
t_c, x		4.16		4.16			6.96			6.96	
t_f, x		2.23		2.23			3.33			3.33	
c_p, x		616		632			481			471	

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	NR	SR	WR	EB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		1115	1087	
Potential Capacity, $c_{p,x}$		616	632	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		616	632	
Probability of Queue-free State, $p_{0,j}$		0.965	0.909	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		543	558	
Potential Capacity, $c_{p,x}$		481	471	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		481	471	
Probability of Queue-free State, $p_{0,j}$		0.758	0.931	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	WestBound			EastBound		
Movement	7	8	9	10	11	12
Lane Configuration			R			R
Shared Flow Rate, $v_y$			116			33
Movement Capacity, $c_{m,x}$			481			471
Shared Capacity, $c_{SH}$						

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		22		58			116			33
Movement Capacity		616		632			481			471
Lane Configuration		L		L			R			R
Shared Capacity										
Control Delay		11.1		11.3			14.9			13.2

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration		L		L			R			R
Flow Rate		22		58			116			33
Lane Capacity		616		632			481			471
$v/c$		0.04		0.09			0.24			0.07
95% Queue Length		0.1		0.3			0.9			0.2
Control Delay		11.1		11.3			14.9			13.2



LOS	B	B	B	B
Approach Delay	0.2	0.6	14.9	13.2
Approach LOS			B	B
Intersection Delay	1.3			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/20/2019 10:28:45 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_PM\_J-Turn\_5Ri -o\_Jadd\_Hill -US 41x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: J-Turn\_5Ri -o\_Jadd PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Hillisdale Rd / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:										
Approach	NorthBound			SouthBound						
Movement	1U	2	3	4U	5	6				
	U	L	T	R	L	T	R			
Volume	0	20	961	63	0	53	1067	22		
Peak Hour Factor, PHF					0.92					
Hourly Flow Rate, HFR						58	1160	24		
Percent Heavy Vehicles	3	3			3					
Number of Lanes	0	1	2	0	0	1	2	0		
Lane Configuration						L	T	TR		
Median Type	Undivided									
Median Storage										
RT channelized?										
Left-Turn Lane Storage										
Upstream Signal?	Not Present									

Minor Street:										
Approach	WestBound			EastBound						
Movement	7	8	9	10	11	12				
	L	T	R	L	T	R				
Volume				107				30		
Peak Hour Factor, PHF					0.92					
Hourly Flow Rate, HFR					116					
Percent Heavy Vehicles					3					
Number of Lanes	0	0	1	0	0	1				
Lane Configuration					R					
RT channelized?					No					
Flared Approach   Storage										
Percent Grade					0		0			

Pedestrian Volumes and Adjustments

Approach				
Movement	NB	SB	WB	EB
	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach												
Movement	1U	NB	4U	SB	7	WestBound	8	9	10	EastBound	11	12
Lane Configuration	L		L			R		R		L	T	R
Flow Rate	22		58			116		33				
Lane Capacity	580		617			472		447				
v/c	0.04		0.09			0.25		0.07				
95% Queue Length	0.1		0.3			1.0		0.2				
Control Delay	11.4		11.4			15.1		13.7				
LOS	B		B			C		B				
Approach Delay	0.2		0.5			15.1		13.7				
Approach LOS						C		B				
Intersection Delay	1.2											

Step 1: MOVEMENT PRIORITIES

Major Street:										
Approach	NorthBound			SouthBound						
Priority	1U	2	3	4U	5	6				
Movement	U	L	T	R	L	T	R			
Minor Street:										

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Maj or Street: Approach Movement		NorthBound				SouthBound			
	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Volume, V_x	0	20	961	63		0	53	1067	22
Flow Rate, v_x		22	1045	68			58	1160	24

Minor Street: Approach Movement		WestBound				EastBound			
		7	8	9			10	11	12
		L	T	R			L	T	R
Volume, V_x				107					30
Flow Rate, v_x				116					33

Step 3: CONFLICTING FLOW RATES

Maj or Street: Approach Movement		NorthBound				SouthBound			
	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Flow Rate, v_x		22	1045	68			58	1160	24
Conflicting Flow, v_c, x		1184					1113		

Minor Street: Approach Movement		WestBound				EastBound			
		7	8	9			10	11	12
		L	T	R			L	T	R
Flow Rate, v_x				116					33
Conflicting Flow, v_c, x				557					592

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS											
Approach Movement	NB	SB			WestBound			EastBound			
	1U	1	4U	4	7	8	9	10	11	12	
	U	L	U	L	L	T	R	L	T	R	
t_c, base											
Single Stage		4.1		4.1			6.9			6.9	
Stage I											
Stage II											
t_c, HV		2.0		2.0			2.0			2.0	
P_HV		0.03		0.03			0.03			0.03	
t_c, G							0.1			0.1	
G							0			0	
t_3, LT		0.0		0.0			0.0			0.0	
t_c											
Single Stage		4.16		4.16			6.96			6.96	
Stage I											
Stage II											

FOLLOW-UP HEADWAYS											
Approach Movement	NB	SB			WestBound			EastBound			
	1U	1	4U	4	7	8	9	10	11	12	
	U	L	U	L	L	T	R	L	T	R	
t_f, base											
t_f, HV		2.2		2.2			3.3			3.3	
P_HV		1.0		1.0			1.0			1.0	
t_f		0.03		0.03			0.03			0.03	
		2.23		2.23			3.33			3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT											
Approach Movement	NB	SB			WestBound			EastBound			
	1U	1	4U	4	7	8	9	10	11	12	
	U	L	U	L	L	T	R	L	T	R	
v_c, x		1184		1113			557			592	
t_c, x		4.16		4.16			6.96			6.96	
t_f, x		2.23		2.23			3.33			3.33	
c_p, x		580		617			472			447	

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	NR	SR	WR	EB
-------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		1184	1113	
Potential Capacity, $c_{p,x}$		580	617	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		580	617	
Probability of Queue-free State, $p_{0,j}$		0.963	0.907	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		557	592	
Potential Capacity, $c_{p,x}$		472	447	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		472	447	
Probability of Queue-free State, $p_{0,j}$		0.753	0.927	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES											
Approach		WestBound				EastBound					
Movement	7	8	9		10	11	12				
Lane Configuration			R				R				
Shared Flow Rate, $v_y$			116				33				
Movement Capacity, $c_{m,x}$			472				447				
Shared Capacity, $c_{SH}$											

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS											
Approach		NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12	
Flow Rate		22		58			116			33	
Movement Capacity		580		617			472			447	
Lane Configuration		L		L			R			R	
Shared Capacity											
Control Delay		11.4		11.4			15.1			13.7	

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach		NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12	
Lane Configuration		L		L			R			R	
Flow Rate		22		58			116			33	
Lane Capacity		580		617			472			447	
$v/c$		0.04		0.09			0.25			0.07	
95% Queue Length		0.1		0.3			1.0			0.2	
Control Delay		11.4		11.4			15.1			13.7	

LOS	B	B	C	
Approach Delay	0.2	0.5	15.1	13.7
Approach LOS			C	B
Intersection Delay	1.2			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/20/2019 10:50:45 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_PM\_J-Turn\_5Rin-out\_Hill-US 41x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: J-Turn\_5Rin-out PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Hillisdale Rd / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound						SouthBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Volume	0	25	1201	53	0	66	1284	1	
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		27	1305	58		72	1396	1	
Percent Heavy Vehicles	3	3			3	3			
Number of Lanes	0	1	2	0	0	1	2	0	
Lane Configuration		L	T	TR		L	T	TR	
Median Type	Undivided								
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?	Not Present								

Minor Street:	WestBound			EastBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume			186			39
Peak Hour Factor, PHF			0.92			
Hourly Flow Rate, HFR			202			42
Percent Heavy Vehicles			3			3
Number of Lanes	0	0	1	0	0	1
Lane Configuration			R			R
RT channelized?			No			No
Flared Approach   Storage						
Percent Grade		0			0	

Pedestrian Volumes and Adjustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	NB	SB	WestBound	EastBound
Movement	1	4U	7	10
Lane Configuration	L	L	R	R
Flow Rate	27	72	202	42
Lane Capacity	480	495	390	380
v/c	0.06	0.14	0.52	0.11
95% Queue Length	0.2	0.5	2.9	0.4
Control Delay	12.9	13.5	23.7	15.6
LOS	B	B	C	C
Approach Delay	0.3	0.7	23.7	15.6
Approach LOS			C	C
Intersction Delay	2.2			

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound						SouthBound		
Approach	1U	1	2	3	4U	4	5	6	
Priority	U	L	T	R	U	L	T	R	

Minor Street:

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U	NorthBound				4U	SouthBound		
	U	1	2	3		U	4	5	6
		L	T	R			L	T	R
Volume, V_x	0	25	1201	53		0	66	1284	1
Flow Rate, v_x		27	1305	58			72	1396	1

Minor Street: Approach Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Volume, V_x					186		
Flow Rate, v_x					202		

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U	NorthBound				4U	SouthBound		
	U	1	2	3		U	4	5	6
		L	T	R			L	T	R
Flow Rate, v_x		27	1305	58			72	1396	1
Conflicting Flow, v_c, x		1397					1363		

Minor Street: Approach Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Flow Rate, v_x					202		
Conflicting Flow, v_c, x					682		

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS										
Approach Movement	NB	SB			WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_c, base		4.1		4.1			6.9			6.9
Single Stage										
Stage I										
Stage II										
t_c, HV		2.0		2.0			2.0			2.0
P_HV		0.03		0.03			0.03			0.03
t_c, G							0.1			0.1
G							0			0
t_3, LT		0.0		0.0			0.0			0.0
t_c										
Single Stage		4.16		4.16			6.96			6.96
Stage I										
Stage II										

FOLLOW-UP HEADWAYS										
Approach Movement	NB	SB			WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_f, base		2.2		2.2			3.3			3.3
t_f, HV		1.0		1.0			1.0			1.0
P_HV		0.03		0.03			0.03			0.03
t_f		2.23		2.23			3.33			3.33

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach Movement	NB	SB			WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
v_c, x		1397		1363			682			698
t_c, x		4.16		4.16			6.96			6.96
t_f, x		2.23		2.23			3.33			3.33
c_p, x		480		495			390			380

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach		NR		SR		WR		EB
-------------------------------	--	----	--	----	--	----	--	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		1397	1363	
Potential Capacity, $c_{p,x}$		480	495	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		480	495	
Probability of Queue-free State, $p_{0,j}$		0.943	0.855	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		682	698	
Potential Capacity, $c_{p,x}$		390	380	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		390	380	
Probability of Queue-free State, $p_{0,j}$		0.482	0.889	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	WestBound			EastBound		
Movement	7	8	9	10	11	12
Lane Configuration			R			R
Shared Flow Rate, $v_y$			202			42
Movement Capacity, $c_{m,x}$			390			380
Shared Capacity, $c_{SH}$						

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		27		72			202			42
Movement Capacity		480		495			390			380
Lane Configuration		L		L			R			R
Shared Capacity										
Control Delay		12.9		13.5			23.7			15.6

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
Movement		L		L			R			R
Lane Configuration										
Flow Rate		27		72			202			42
Lane Capacity		480		495			390			380
$v/c$		0.06		0.14			0.52			0.11
95% Queue Length		0.2		0.5			2.9			0.4
Control Delay		12.9		13.5			23.7			15.6



LOS	B	B	C	
Approach Delay	0.3	0.7	23.7	15.6
Approach LOS			C	C
Intersection Delay	2.2			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/20/2019 10:33:57 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_PM\_J-Turn\_5Ri-o\_Jadd\_Hill-US 41x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: J-Turn\_5Ri -o\_Jadd PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Hillisdale Rd / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R
Volume	0	25	1205	80		0	66	1408	11
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		27	1310	87			72	1530	12
Percent Heavy Vehicles	3	3				3	3		
Number of Lanes	0	1	2	0		0	1	2	0
Lane Configuration		L	T	TR			L	T	TR
Median Type					Undivided				
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street: Approach Movement	WestBound 7 L	8 T	9 R		EastBound 10 L	11 T	12 R
Volume			186				39
Peak Hour Factor, PHF				0.92			
Hourly Flow Rate, HFR			202				42
Percent Heavy Vehicles			3				3
Number of Lanes	0	0	1		0	0	1
Lane Configuration			R				R
RT channelized?			No				No
Flared Approach   Storage							
Percent Grade		0				0	

Pedestrian Volumes and Adjustments

Approach Movement	NB 13	SB 14	WB 15	EB 16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach Movement Lane Configuration	1U U	NB 1 L	4U L	7 L	8 R	9 R	10 R	EastBound 11 T	12 R
Flow Rate		27		72		202			42
Lane Capacity		422		480		380			340
v/c		0.06		0.15		0.53			0.12
95% Queue Length		0.2		0.5		3.0			0.4
Control Delay		14.1		13.8		24.7			17.1
LOS		B		B		C			C
Approach Delay		0.3		0.6		24.7		17.1	
Approach LOS						C		C	
Intersction Delay		2.2							

Step 1: MOVEMENT PRIORITIES

Major Street: Approach Priority Movement	1U U	NorthBound 1 L	2 T	3 R		4U U	SouthBound 4 L	5 T	6 R
Minor Street:									

Approach	WestBound					EastBound		
Priority	7	8	9			10	11	12
Movement	L	T	R			L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach	NorthBound					SouthBound			
Movement	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Volume, V_x	0	25	1205	80		0	66	1408	11
Flow Rate, v_x		27	1310	87			72	1530	12

Minor Street:									
Approach	WestBound					EastBound			
Movement		7	8	9			10	11	12
		L	T	R			L	T	R
Volume, V_x				186					39
Flow Rate, v_x				202					42

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach	NorthBound					SouthBound			
Movement	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Flow Rate, v_x		27	1310	87			72	1530	12
Conflicting Flow, v_c, x		1542					1397		

Minor Street:									
Approach	WestBound					EastBound			
Movement		7	8	9			10	11	12
		L	T	R			L	T	R
Flow Rate, v_x				202					42
Conflicting Flow, v_c, x				698					771

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS										
Approach	NB		SB			WestBound		EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_c, base		4.1		4.1			6.9			6.9
Single Stage										
Stage I										
Stage II										
t_c, HV		2.0		2.0			2.0			2.0
P_HV		0.03		0.03			0.03			0.03
t_c, G							0.1			0.1
G							0			0
t_3, LT		0.0		0.0			0.0			0.0
t_c										
Single Stage		4.16		4.16			6.96			6.96
Stage I										
Stage II										

FOLLOW-UP HEADWAYS										
Approach	NB		SB			WestBound		EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_f, base		2.2		2.2			3.3			3.3
t_f, HV		1.0		1.0			1.0			1.0
P_HV		0.03		0.03			0.03			0.03
t_f		2.23		2.23			3.33			3.33

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach	NB		SB			WestBound		EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
v_c, x		1542		1397			698			771
t_c, x		4.16		4.16			6.96			6.96
t_f, x		2.23		2.23			3.33			3.33
c_p, x		422		480			380			340

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		1542	1397	
Potential Capacity, $c_{p,x}$		422	480	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		422	480	
Probability of Queue-free State, $p_{0,j}$		0.936	0.851	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		698	771	
Potential Capacity, $c_{p,x}$		380	340	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		380	340	
Probability of Queue-free State, $p_{0,j}$		0.469	0.875	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	WestBound			EastBound		
Movement	7	8	9	10	11	12
Lane Configuration			R			R
Shared Flow Rate, $v_y$			202			42
Movement Capacity, $c_{m,x}$			380			340
Shared Capacity, $c_{SH}$						

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		27		72			202			42
Movement Capacity		422		480			380			340
Lane Configuration		L		L			R			R
Shared Capacity										
Control Delay		14.1		13.8			24.7			17.1

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration		L		L			R			R
Flow Rate		27		72			202			42
Lane Capacity		422		480			380			340
$v/c$		0.06		0.15			0.53			0.12
95% Queue Length		0.2		0.5			3.0			0.4
Control Delay		14.1		13.8			24.7			17.1

LOS	B	B	C	
Approach Delay	0.3	0.6	24.7	17.1
Approach LOS			C	C
Intersection Delay	2.2			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/20/2019 10:59:51 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_AM\_JnoSMUT\_5Rin-out\_Hillisdale-US 41-x. xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5Rin-out/no sMUT AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Hillisdale Rd / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound				SouthBound			
Approach	1U	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R
Volume			631	25	0	33	987	8
Peak Hour Factor, PHF					0.92			
Hourly Flow Rate, HFR			686	27		36	1073	9
Percent Heavy Vehicles					3	3		
Number of Lanes	0	0	2	1	0	1	2	1
Lane Configuration			T	R		L	T	R
Median Type	Undivided							
Median Storage								
RT channelized?	No							
Left-Turn Lane Storage								
Upstream Signal?	Not Present							

Minor Street:	WestBound			EastBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume			148			14
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR			161			15
Percent Heavy Vehicles			3			3
Number of Lanes	0	0	1	0	0	1
Lane Configuration			R			R
RT channelized?	No					
Flared Approach   Storage						
Percent Grade		0			0	

Pedestrian Volumes and Adjustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	NB	SB	WestBound	EastBound
Movement	1	4U	7	10
Lane Configuration	1U	4	8	11
	U	L	R	R
Flow Rate		36	161	15
Lane Capacity		876	650	486
v/c		0.04	0.25	0.03
95% Queue Length		0.1	1.0	0.1
Control Delay		9.3	12.3	12.6
LOS		A	B	B
Approach Delay		0.3	12.3	12.6
Approach LOS			B	B
Intersection Delay	1.3			

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound				SouthBound			
Approach	1U	2	3	4U	4	5	6	
Priority	U	L	T	U	L	T	R	
Movement	U	L	T	U	L	T	R	

Minor Street:

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement	1U	NorthBound				SouthBound			
	U	1	2	3		4U	4	5	6
		L	T	R		U	L	T	R
Volume, V_x		631				0	33		
Flow Rate, v_x		686					36		
		25					987		
		27					1073		
							8		
							9		

Minor Street:								
Approach Movement	WestBound				EastBound			
	7	8	9		10	11	12	
	L	T	R		L	T	R	
Volume, V_x	148				14			
Flow Rate, v_x	161				15			

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement	1U	NorthBound				SouthBound			
	U	1	2	3		4U	4	5	6
		L	T	R		U	L	T	R
Flow Rate, v_x		686				27	36		
Conflicting Flow, v_c, x							1073		
							713		
							9		

Minor Street:								
Approach Movement	WestBound				EastBound			
	7	8	9		10	11	12	
	L	T	R		L	T	R	
Flow Rate, v_x	161				15			
Conflicting Flow, v_c, x	343				536			

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS											
Approach Movement	NB		SB		WestBound			EastBound			
	1U	1	4U	4	7	8	9	10	11	12	
	U	L	U	L	L	T	R	L	T	R	
t_c, base											
Single Stage				4.1			6.9			6.9	
Stage I											
Stage II											
t_c, HV				2.0			2.0			2.0	
P_HV				0.03			0.03			0.03	
t_c, G							0.1			0.1	
G							0			0	
t_3, LT				0.0			0.0			0.0	
t_c											
Single Stage				4.16			6.96			6.96	
Stage I											
Stage II											

FOLLOW-UP HEADWAYS											
Approach Movement	NB		SB		WestBound			EastBound			
	1U	1	4U	4	7	8	9	10	11	12	
	U	L	U	L	L	T	R	L	T	R	
t_f, base				2.2			3.3			3.3	
t_f, HV				1.0			1.0			1.0	
P_HV				0.03			0.03			0.03	
t_f				2.23			3.33			3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT											
Approach Movement	NB		SB		WestBound			EastBound			
	1U	1	4U	4	7	8	9	10	11	12	
	U	L	U	L	L	T	R	L	T	R	
v_c, x				713			343			536	
t_c, x				4.16			6.96			6.96	
t_f, x				2.23			3.33			3.33	
c_p, x				876			650			486	

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$			713	
Potential Capacity, $c_{p,x}$			876	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Movement Capacity, $c_{m,x}$			876	
Probability of Queue-free State, $p_{0,j}$			0.959	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		343	536	
Potential Capacity, $c_{p,x}$		650	486	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		650	486	
Probability of Queue-free State, $p_{0,j}$		0.753	0.969	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	WestBound			EastBound		
Movement	7	8	9	10	11	12
Lane Configuration			R			R
Shared Flow Rate, $v_y$			161			15
Movement Capacity, $c_{m,x}$			650			486
Shared Capacity, $c_{SH}$						

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
Movement										
Flow Rate				36			161			15
Movement Capacity				876			650			486
Lane Configuration				L			R			R
Shared Capacity										
Control Delay				9.3			12.3			12.6

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
Movement										
Lane Configuration				L			R			R
Flow Rate				36			161			15
Lane Capacity				876			650			486
$v/c$				0.04			0.25			0.03
95% Queue Length				0.1			1.0			0.1
Control Delay				9.3			12.3			12.6



LOS		A		B	
Approach Delay		0.3	12.3		12.6
Approach LOS			B		B
Intersection Delay	1.3				

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/21/2019 3:21:07 PM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_AM\_5-Rin-out\_NMUT-x.xtw  
 Analyst: Mike Koyak  
 Agency: INDOT  
 Date Performed: 8/22/2019  
 Time Analyzed: 2020 AM N-MUT w/5Rin-out  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: Hillisdale Road @ US 41  
 Units: U.S. Customary  
 Intersection Name: South MUT  
 Major Street Direction: North-South  
 East/West Street Name: South MUT  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound						SouthBound		
Approach	1U	1	2	3		4U	4	5	6
Movement	U	L	T	R		U	L	T	R
Volume									922
Peak Hour Factor, PHF						0.92			
Hourly Flow Rate, HFR									1002
Percent Heavy Vehicles									
Number of Lanes	0	0	0	0		0	0	2	0
Lane Configuration								T	
Median Type						Undivided			
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?						Not Present			

Minor Street:	WestBound			EastBound			
Approach	7	8	9		10	11	12
Movement	L	T	R		L	T	R
Volume							
Peak Hour Factor, PHF					0.92		
Hourly Flow Rate, HFR							
Percent Heavy Vehicles							
Number of Lanes	1	0	0		0	0	0
Lane Configuration	L						
RT channelized?							
Flared Approach   Storage							
Percent Grade			0				

Pedestrian Volumes and Adjustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)		0	0	
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Approach	Delay, Queue Length, and Level of Service		WestBound				EastBound			
Movement	1U	NB	4U	4	7	8	9	10	11	12
Lane Configuration					L					
Flow Rate					116					
Lane Capacity					497					
v/c					0.23					
95% Queue Length					0.9					
Control Delay					14.4					
LOS					B					
Approach Delay						14.4				
Approach LOS						B				
Intersection Delay		1.5								

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound						SouthBound		
Approach	1U	1	2	3		4U	4	5	6
Priority	U	L	T	R		U	L	T	R
Movement									

Minor Street:

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement	NorthBound				SouthBound				
	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Volume, V_x							922		
Flow Rate, v_x							1002		

Minor Street:									
Approach Movement	WestBound				EastBound				
		7	8	9			10	11	12
		L	T	R			L	T	R
Volume, V_x	107								
Flow Rate, v_x	116								

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement	NorthBound				SouthBound				
	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Flow Rate, v_x							1002		
Conflicting Flow, v_c, x									

Minor Street:									
Approach Movement	WestBound				EastBound				
		7	8	9			10	11	12
		L	T	R			L	T	R
Flow Rate, v_x	116								
Conflicting Flow, v_c, x	501								

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_c, base					7.5					
Single Stage										
Stage I										
Stage II										
t_c, HV					2.0					
P_HV					0.03					
t_c, G					0.2					
G					0					
t_3, LT					0.7					
t_c										
Single Stage					6.86					
Stage I										
Stage II										

FOLLOW-UP HEADWAYS										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_f, base					3.5					
t_f, HV					1.0					
P_HV					0.03					
t_f					3.53					

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
v_c, x					501					
t_c, x					6.86					
t_f, x					3.53					
c_p, x					497					

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$ Lane Width, $w$ Walking Speed, $S_p$ Pedestrian Blockage Factor, $f_{pb}$		0	0	
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$ Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Shared L/U Capacity, $c_{SH}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Major L, Minor T Adjusted Impedance Factor, $p''$ Major L, Minor T Impedance Factor, $p'$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$		501 497 1.000		

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES											
Approach	WestBound						EastBound				
Movement	7	8	9	10	11	12					
Lane Configuration	L										
Shared Flow Rate, $v_y$	116										
Movement Capacity, $c_{m,x}$	497										
Shared Capacity, $c_{SH}$											

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS											
Approach	NB		SB		WestBound			EastBound			
Movement	1U	1	4U	4	7	8	9	10	11	12	
Flow Rate					116						
Movement Capacity					497						
Lane Configuration					L						
Shared Capacity											
Control Delay					14.4						

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound			
Movement	1U	1	4U	4	7	8	9	10	11	12	
Lane Configuration					L						
Flow Rate					116						
Lane Capacity					497						
$v/c$					0.23						
95% Queue Length					0.9						
Control Delay					14.4						

LOS		B	
Approach Delay			14.4
Approach LOS			B
Intersection Delay	1.5		

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/22/2019 8:27:19 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_PM\_5-Rin-out\_NMUT-x.xtw  
 Analyst: Mike Koyak  
 Agency: INDOT  
 Date Performed: 8/22/2019  
 Time Analyzed: 2020 PM N-MUT w/5Rin-out  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: Hillisdale Road @ US 41  
 Units: U.S. Customary  
 Intersection Name: South MUT  
 Major Street Direction: North-South  
 East/West Street Name: South MUT  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound						SouthBound		
Approach	1U	1	2	3		4U	4	5	6
Movement	U	L	T	R		U	L	T	R
Volume									1097
Peak Hour Factor, PHF						0.92			
Hourly Flow Rate, HFR									1192
Percent Heavy Vehicles									
Number of Lanes	0	0	0	0		0	0	2	0
Lane Configuration								T	
Median Type						Undivided			
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?						Not Present			

Minor Street:	WestBound			EastBound			
Approach	7	8	9		10	11	12
Movement	L	T	R		L	T	R
Volume	63						
Peak Hour Factor, PHF					0.92		
Hourly Flow Rate, HFR	68						
Percent Heavy Vehicles	3						
Number of Lanes	1	0	0		0	0	0
Lane Configuration	L						
RT channelized?							
Flared Approach   Storage							
Percent Grade		0					

Pedestrian Volumes and Adjustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)		0	0	
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f <sub>pb</sub>				

Delay, Queue Length, and Level of Service

Approach	NB	SB	WestBound				EastBound		
Movement	1U	4U	7	8	9	10	11	12	
Lane Configuration			L						
Flow Rate			68						
Lane Capacity			432						
v/c			0.16						
95% Queue Length			0.6						
Control Delay			14.9						
LOS			B						
Approach Delay				14.9					
Approach LOS				B					
Intersection Delay	0.8								

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound						SouthBound		
Approach	1U	1	2	3		4U	4	5	6
Movement	U	L	T	R		U	L	T	R
Priority									

Minor Street:

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement	NorthBound				SouthBound				
	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Volume, V_x							1097		
Flow Rate, v_x							1192		

Minor Street:									
Approach Movement	WestBound				EastBound				
		7	8	9			10	11	12
		L	T	R			L	T	R
Volume, V_x	63								
Flow Rate, v_x	68								

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement	NorthBound				SouthBound				
	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Flow Rate, v_x							1192		
Conflicting Flow, v_c, x									

Minor Street:									
Approach Movement	WestBound				EastBound				
		7	8	9			10	11	12
		L	T	R			L	T	R
Flow Rate, v_x	68								
Conflicting Flow, v_c, x	596								

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_c, base					7.5					
Single Stage										
Stage I										
Stage II										
t_c, HV					2.0					
P_HV					0.03					
t_c, G					0.2					
G					0					
t_3, LT					0.7					
t_c										
Single Stage					6.86					
Stage I										
Stage II										

FOLLOW-UP HEADWAYS										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_f, base					3.5					
t_f, HV					1.0					
P_HV					0.03					
t_f					3.53					

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
v_c, x					596					
t_c, x					6.86					
t_f, x					3.53					
c_p, x					432					

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$ Lane Width, $w$ Walking Speed, $S_p$ Pedestrian Blockage Factor, $f_{pb}$		0	0	
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$ Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Shared L/U Capacity, $c_{SH}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Major L, Minor T Adjusted Impedance Factor, $p''$ Major L, Minor T Impedance Factor, $p'$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$		596 432 1.000		

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES											
Approach	WestBound						EastBound				
Movement	7	8	9	10	11	12	10	11	12	11	12
Lane Configuration	L										
Shared Flow Rate, $v_y$	68										
Movement Capacity, $c_{m,x}$	432										
Shared Capacity, $c_{SH}$											

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS																
Approach	NB				SB				WestBound				EastBound			
Movement	1U	1	4U	4	7	8	9	10	11	12	10	11	12	11	12	
Flow Rate					68											
Movement Capacity					432											
Lane Configuration					L											
Shared Capacity																
Control Delay					14.9											

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB				SB				WestBound				EastBound			
Movement	1U	1	4U	4	7	8	9	10	11	12	10	11	12	11	12	
Lane Configuration					L											
Flow Rate					68											
Lane Capacity					432											
$v/c$					0.16											
95% Queue Length					0.6											
Control Delay					14.9											



LOS		B	
Approach Delay			14.9
Approach LOS			B
Intersection Delay	0.8		

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/22/2019 8:28:57 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: TWSC1.xtw  
 Analyst: Mike Koyak  
 Agency: INDOT  
 Date Performed: 8/22/2019  
 Time Analyzed: 2020 AM S-MUT w/5Ri -o  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: Hillisdale Road @ US 41  
 Units: U. S. Customary  
 Intersection Name: South MUT  
 Major Street Direction: North-South  
 East/West Street Name: South MUT  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	1U U	NorthBound 1 L	2 T	3 R	4U U	SouthBound 4 L	5 T	6 R
Volume			667					
Peak Hour Factor, PHF					0.92			
Hourly Flow Rate, HFR			725					
Percent Heavy Vehicles								
Number of Lanes	0	0	2 T	0	0	0	0	0
Lane Configuration								
Median Type					Undivided			
Median Storage								
RT channelized?								
Left-Turn Lane Storage								
Upstream Signal?					Not Present			

Minor Street: Approach Movement	WestBound 7 L	8 T	9 R	EastBound 10 L	11 T	12 R
Volume					22	
Peak Hour Factor, PHF					0.92	
Hourly Flow Rate, HFR					24	
Percent Heavy Vehicles					3	
Number of Lanes	0	0	0	1 L	0	0
Lane Configuration						
RT channelized?						
Flared Approach   Storage					3	
Percent Grade						

Pedestrian Volumes and Adjustments

Approach Movement	NB 13	SB 14	WB 15	EB 16
Flow (ped/hr)		0		0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach Movement	1U U	NB 1	4U U	4 T	7 R	WestBound 8	9	10 L	EastBound 11	12
Lane Configuration										
Flow Rate								24		
Lane Capacity								572		
v/c								0.04		
95% Queue Length								0.1		
Control Delay								11.6		
LOS								B		
Approach Delay									11.6	
Approach LOS									B	
Intersection Delay										

Step 1: MOVEMENT PRIORITIES

Major Street: Approach Priority Movement	1U U	NorthBound 1 L	2 T	3 R	4U U	SouthBound 4 L	5 T	6 R
Minor Street:								

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement	NorthBound				SouthBound				
	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Volume, V_x	667				22				
Flow Rate, v_x	725				24				

Minor Street:							
Approach Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Volume, V_x	22				24		
Flow Rate, v_x	24				24		

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement	NorthBound				SouthBound				
	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Flow Rate, v_x	725				24				
Conflicting Flow, v_c, x	725				363				

Minor Street:							
Approach Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Flow Rate, v_x	24				363		
Conflicting Flow, v_c, x	24				363		

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_c, base							7.5			
Single Stage							7.5			
Stage I							7.5			
Stage II							7.5			
t_c, HV							2.0			
P_HV							0.03			
t_c, G							0.2			
G							3			
t_3, LT							0.7			
t_c							7.46			
Single Stage							7.46			
Stage I							7.46			
Stage II							7.46			

FOLLOW-UP HEADWAYS										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_f, base							3.5			
t_f, HV							1.0			
P_HV							0.03			
t_f							3.53			

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
v_c, x							363			
t_c, x							7.46			
t_f, x							3.53			
c_p, x							572			

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	NR	SR	WR	EB
-------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0			0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$				363
Potential Capacity, $c_{p,x}$				572
Pedestrian Impedance Factor, $p_{p,x}$				1.000
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$				1.000
Movement Capacity, $c_{m,x}$				572

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES											
Approach	WestBound						EastBound				
Movement	7	8	9	10	11	12	10	11	12	10	11
Lane Configuration				L			L			L	
Shared Flow Rate, $v_y$							24				
Movement Capacity, $c_{m,x}$							572				
Shared Capacity, $c_{SH}$											

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS												
Approach	NB			SB			WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12	12	
Flow Rate								24				
Movement Capacity								572				
Lane Configuration								L				
Shared Capacity												
Control Delay								11.6				

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB			SB			WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12	12	
Lane Configuration								L				
Flow Rate								24				
Lane Capacity								572				
$v/c$								0.04				
95% Queue Length								0.1				
Control Delay								11.6				

LOS  
Approach Delay  
Approach LOS  
Intersection Delay

B

11.6  
B

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/22/2019 8:05:57 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2020\_PM\_5-Rin-out\_SMUT-x.xtw  
 Analyst: Mike Koyak  
 Agency: INDOT  
 Date Performed: 8/22/2019  
 Time Analyzed: 2020 PM S-MUT w/5Ri -o  
 Jurisdiction: Vincennes  
 Analysis Year: 2020  
 Project Description: Hillisdale Road @ US 41  
 Units: U. S. Customary  
 Intersection Name: South MUT  
 Major Street Direction: North-South  
 East/West Street Name: South MUT  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound						SouthBound		
Approach	1U	1	2	3		4U	4	5	6
Movement	U	L	T	R		U	L	T	R
Volume			1020						
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR			1109						
Percent Heavy Vehicles									
Number of Lanes	0	0	2	0		0	0	0	0
Lane Configuration			T						
Median Type						Undivided			
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?						Not Present			

Minor Street:	WestBound			EastBound			
Approach	7	8	9		10	11	12
Movement	L	T	R		L	T	R
Volume						24	
Peak Hour Factor, PHF					0.92		
Hourly Flow Rate, HFR						26	
Percent Heavy Vehicles						3	
Number of Lanes	0	0	0		1	0	0
Lane Configuration					L		
RT channelized?							
Flared Approach   Storage							3
Percent Grade							

Pedestrian Volumes and Adjustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)	0			0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	NB	SB	WestBound	EastBound
Movement	1U	4U	7	8
Lane Configuration				L
Flow Rate				26
Lane Capacity				419
v/c				0.06
95% Queue Length				0.2
Control Delay				14.2
LOS				B
Approach Delay				14.2
Approach LOS				B
Intersction Delay				

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound						SouthBound		
Approach	1U	1	2	3		4U	4	5	6
Movement	U	L	T	R		U	L	T	R
Priority									

Minor Street:

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement	NorthBound				SouthBound				
	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R

Volume, V_x	1020
Flow Rate, v_x	1109

Minor Street:							
Approach Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Volume, V_x	24
Flow Rate, v_x	26

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement	NorthBound				SouthBound				
	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R

Flow Rate, v_x	1109
Conflicting Flow, v_c, x	

Minor Street:							
Approach Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Flow Rate, v_x	26
Conflicting Flow, v_c, x	554

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R

t_c, base	
Single Stage	7.5
Stage I	
Stage II	
t_c, HV	2.0
P_HV	0.03
t_c, G	0.2
G	3
t_3, LT	0.7
t_c	
Single Stage	7.46
Stage I	
Stage II	

FOLLOW-UP HEADWAYS										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R

t_f, base	
t_f, HV	1.0
P_HV	0.03
t_f	3.53

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R

v_c, x	554
t_c, x	7.46
t_f, x	3.53
c_p, x	419

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	NR	SR	WR	EB
-------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$ Lane Width, $w$ Walking Speed, $S_p$ Pedestrian Blockage Factor, $f_{pb}$	0			0
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$ Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Shared L/U Capacity, $c_{SH}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Major L, Minor T Adjusted Impedance Factor, $p''$ Major L, Minor T Impedance Factor, $p'$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$			554 419 1.000	
			1.000 419	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES												
Approach	WestBound						EastBound					
Movement	7	8	9	10	11	12						
Lane Configuration							L					
Shared Flow Rate, $v_y$							26					
Movement Capacity, $c_{m,x}$							419					
Shared Capacity, $c_{SH}$												

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS												
Approach	NB			SB			WestBound		EastBound			
Movement	1U	1	4U	4	7	8	9	10	11	12		
Flow Rate							26					
Movement Capacity							419					
Lane Configuration							L					
Shared Capacity												
Control Delay							14.2					

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB			SB			WestBound		EastBound			
Movement	1U	1	4U	4	7	8	9	10	11	12		
Lane Configuration							L					
Flow Rate							26					
Lane Capacity							419					
$v/c$							0.06					
95% Queue Length							0.2					
Control Delay							14.2					



LOS  
Approach Delay  
Approach LOS  
Intersection Delay

B

14.2  
B

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/22/2019 8:09:33 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_AM\_5-Rin-out\_NMUT-x.xtw  
 Analyst: Mike Koyak  
 Agency: INDOT  
 Date Performed: 8/22/2019  
 Time Analyzed: 2045 AM N-MUT w/5RI -o  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: Hillisdale Road @ US 41  
 Units: U. S. Customary  
 Intersection Name: South MUT  
 Major Street Direction: North-South  
 East/West Street Name: South MUT  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound						SouthBound		
Approach	1U	1	2	3		4U	4	5	6
Movement	U	L	T	R		U	L	T	R
Volume									1167
Peak Hour Factor, PHF						0.92			
Hourly Flow Rate, HFR									1268
Percent Heavy Vehicles									
Number of Lanes	0	0	0	0		0	0	2	0
Lane Configuration								T	
Median Type						Undivided			
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?						Not Present			

Minor Street:	WestBound			EastBound			
Approach	7	8	9		10	11	12
Movement	L	T	R		L	T	R
Volume							
Peak Hour Factor, PHF					0.92		
Hourly Flow Rate, HFR							
Percent Heavy Vehicles							
Number of Lanes	1	0	0		0	0	0
Lane Configuration	L						
RT channelized?							
Flared Approach   Storage							
Percent Grade			0				

Pedestrian Volumes and Adjustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)			0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Approach	Delay, Queue Length, and Level of Service											
	NB	WestBound			EastBound							
Movement	1U	4U	7	8	9	10	11	12				
Lane Configuration			L									
Flow Rate									146			
Lane Capacity									409			
v/c									0.36			
95% Queue Length									1.6			
Control Delay									18.6			
LOS									C			
Approach Delay									18.6			
Approach LOS									C			
Intersection Delay	1.9											

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound						SouthBound		
Approach	1U	1	2	3		4U	4	5	6
Movement	U	L	T	R		U	L	T	R
Priority									

Minor Street:

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement	NorthBound				SouthBound				
	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R

Volume, V_x							1167
Flow Rate, v_x							1268

Minor Street:							
Approach Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Volume, V_x	134
Flow Rate, v_x	146

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement	NorthBound				SouthBound				
	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R

Flow Rate, v_x							1268
Conflicting Flow, v_c, x							

Minor Street:							
Approach Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Flow Rate, v_x	146
Conflicting Flow, v_c, x	634

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R

t_c, base										
Single Stage										
Stage I										
Stage II										
t_c, HV										
P_HV										
t_c, G										
G										
t_3, LT										
t_c										
Single Stage										
Stage I										
Stage II										

FOLLOW-UP HEADWAYS										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R

t_f, base										
t_f, HV										
P_HV										
t_f										

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R

v_c, x										
t_c, x										
t_f, x										
c_p, x										

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$ Lane Width, $w$ Walking Speed, $S_p$ Pedestrian Blockage Factor, $f_{pb}$		0	0	
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$ Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Shared L/U Capacity, $c_{SH}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Major L, Minor T Adjusted Impedance Factor, $p''$ Major L, Minor T Impedance Factor, $p'$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$		634 409 1.000		

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES											
Approach	WestBound						EastBound				
Movement	7	8	9	10	11	12					
Lane Configuration	L										
Shared Flow Rate, $v_y$	146										
Movement Capacity, $c_{m,x}$	409										
Shared Capacity, $c_{SH}$											

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS												
Approach	NB			SB			WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12		
Flow Rate					146							
Movement Capacity					409							
Lane Configuration					L							
Shared Capacity												
Control Delay					18.6							

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB			SB			WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12		
Lane Configuration					L							
Flow Rate					146							
Lane Capacity					409							
$v/c$					0.36							
95% Queue Length					1.6							
Control Delay					18.6							

LOS		C	
Approach Delay			18.6
Approach LOS			C
Intersection Delay	1.9		

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/22/2019 8:25:34 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_PM\_5-Rin-out\_NMUT-x.xtw  
 Analyst: Mike Koyak  
 Agency: INDOT  
 Date Performed: 8/22/2019  
 Time Analyzed: 2045 AM N-MUT w/5Ri -o  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: Hillisdale Road @ US 41  
 Units: U. S. Customary  
 Intersection Name: South MUT  
 Major Street Direction: North-South  
 East/West Street Name: South MUT  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound						SouthBound		
Approach	1U	1	2	3		4U	4	5	6
Movement	U	L	T	R		U	L	T	R
Volume									1388
Peak Hour Factor, PHF						0.92			
Hourly Flow Rate, HFR									1509
Percent Heavy Vehicles									
Number of Lanes	0	0	0	0		0	0	2	0
Lane Configuration								T	
Median Type						Undivided			
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?						Not Present			

Minor Street:	WestBound			EastBound			
Approach	7	8	9		10	11	12
Movement	L	T	R		L	T	R
Volume	80						
Peak Hour Factor, PHF					0.92		
Hourly Flow Rate, HFR	87						
Percent Heavy Vehicles	3						
Number of Lanes	1	0	0		0	0	0
Lane Configuration	L						
RT channelized?							
Flared Approach   Storage							
Percent Grade		0					

Pedestrian Volumes and Adjustments

Approach	NB		SB		WB		EB	
Movement		13		14		15		16
Flow (ped/hr)					0		0	
Lane Width (ft)								
Walking Speed (ft/sec)								
Pedestrian Blockage Factor, f_pb								

Delay, Queue Length, and Level of Service

Approach	NB		SB		WestBound		EastBound			
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration					L					
Flow Rate					87					
Lane Capacity					343					
v/c					0.25					
95% Queue Length					1.0					
Control Delay					19.0					
LOS					C					
Approach Delay						19.0				
Approach LOS						C				
Intersection Delay		1.0								

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound						SouthBound		
Approach	1U	1	2	3		4U	4	5	6
Priority	U	L	T	R		U	L	T	R
Minor Street:									

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement	NorthBound				SouthBound				
	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Volume, V_x							1388		
Flow Rate, v_x							1509		

Minor Street:									
Approach Movement	WestBound				EastBound				
		7	8	9			10	11	12
		L	T	R			L	T	R
Volume, V_x	80								
Flow Rate, v_x	87								

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement	NorthBound				SouthBound				
	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Flow Rate, v_x							1509		
Conflicting Flow, v_c, x									

Minor Street:									
Approach Movement	WestBound				EastBound				
		7	8	9			10	11	12
		L	T	R			L	T	R
Flow Rate, v_x	87								
Conflicting Flow, v_c, x	754								

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_c, base					7.5					
Single Stage										
Stage I										
Stage II										
t_c, HV					2.0					
P_HV					0.03					
t_c, G					0.2					
G					0					
t_3, LT					0.7					
t_c										
Single Stage					6.86					
Stage I										
Stage II										

FOLLOW-UP HEADWAYS										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_f, base					3.5					
t_f, HV					1.0					
P_HV					0.03					
t_f					3.53					

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
v_c, x					754					
t_c, x					6.86					
t_f, x					3.53					
c_p, x					343					

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$ Lane Width, $w$ Walking Speed, $S_p$ Pedestrian Blockage Factor, $f_{pb}$		0	0	
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$ Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Shared L/U Capacity, $c_{SH}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Major L, Minor T Adjusted Impedance Factor, $p''$ Major L, Minor T Impedance Factor, $p'$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$		754 343 1.000  1.000 343		

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES											
Approach	WestBound						EastBound				
Movement	7	8	9	10	11	12	10	11	12	11	12
Lane Configuration	L										
Shared Flow Rate, $v_y$	87										
Movement Capacity, $c_{m,x}$	343										
Shared Capacity, $c_{SH}$											

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS												
Approach	NB				SB				EastBound			
Movement	1U	1	4U	4	7	8	9	10	11	12	12	
Flow Rate					87							
Movement Capacity					343							
Lane Configuration					L							
Shared Capacity												
Control Delay					19.0							

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB				SB				EastBound			
Movement	1U	1	4U	4	7	8	9	10	11	12	12	
Lane Configuration					L							
Flow Rate					87							
Lane Capacity					343							
$v/c$					0.25							
95% Queue Length					1.0							
Control Delay					19.0							



LOS		C	
Approach Delay			19.0
Approach LOS			C
Intersection Delay	1.0		

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/22/2019 8:23:11 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_AM\_5-Rin-out\_SMUT-x.xtw  
 Analyst: Mike Koyak  
 Agency: INDOT  
 Date Performed: 8/22/2019  
 Time Analyzed: 2045 AM S-MUT w/5Rin-out  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: Hillisdale Road @ US 41  
 Units: U.S. Customary  
 Intersection Name: South MUT  
 Major Street Direction: North-South  
 East/West Street Name: South MUT  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound			SouthBound			
Approach	1U	2	3	4U	5	6	
Movement	U	L	T	R	L	T	R
Volume			837				
Peak Hour Factor, PHF				0.92			
Hourly Flow Rate, HFR			910				
Percent Heavy Vehicles							
Number of Lanes	0	0	2	0	0	0	0
Lane Configuration			T				
Median Type							Undivided
Median Storage							
RT channelized?							
Left-Turn Lane Storage							
Upstream Signal?							Not Present

Minor Street:	WestBound			EastBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume						28
Peak Hour Factor, PHF						0.92
Hourly Flow Rate, HFR						30
Percent Heavy Vehicles						3
Number of Lanes	0	0	0	1	0	0
Lane Configuration				L		
RT channelized?						
Flared Approach   Storage						3
Percent Grade						

Pedestrian Volumes and Adjustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)	0			0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f <sub>pb</sub>				

Delay, Queue Length, and Level of Service

Approach	NB	SB	WestBound	EastBound
Movement	1U	4U	7	8
Lane Configuration				L
Flow Rate				30
Lane Capacity				493
v/c				0.06
95% Queue Length				0.2
Control Delay				12.8
LOS				B
Approach Delay				
Approach LOS				12.8
Intersection Delay				B

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound			SouthBound			
Approach	1U	2	3	4U	5	6	
Priority	U	L	T	R	L	T	R
Movement							

Minor Street:

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement		NorthBound				SouthBound			
	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Volume, V_x	837				28				
Flow Rate, v_x	910				30				

Minor Street:								
Approach Movement		WestBound				EastBound		
		7	8	9		10	11	12
		L	T	R		L	T	R
Volume, V_x					28			
Flow Rate, v_x					30			

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement		NorthBound				SouthBound			
	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Flow Rate, v_x	910								
Conflicting Flow, v_c, x									

Minor Street:								
Approach Movement		WestBound				EastBound		
		7	8	9		10	11	12
		L	T	R		L	T	R
Flow Rate, v_x					30			
Conflicting Flow, v_c, x					455			

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS										
Approach Movement		NB		SB		WestBound		EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_c, base							7.5			
Single Stage										
Stage I										
Stage II										
t_c, HV							2.0			
P_HV							0.03			
t_c, G							0.2			
G							3			
t_3, LT							0.7			
t_c										
Single Stage							7.46			
Stage I										
Stage II										

FOLLOW-UP HEADWAYS										
Approach Movement		NB		SB		WestBound		EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_f, base							3.5			
t_f, HV							1.0			
P_HV							0.03			
t_f							3.53			

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach Movement		NB		SB		WestBound		EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
v_c, x							455			
t_c, x							7.46			
t_f, x							3.53			
c_p, x							493			

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$ Lane Width, $w$ Walking Speed, $S_p$ Pedestrian Blockage Factor, $f_{pb}$	0			0
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$ Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Shared L/U Capacity, $c_{SH}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Major L, Minor T Adjusted Impedance Factor, $p''$ Major L, Minor T Impedance Factor, $p'$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$			455 493 1.000	
			1.000 493	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES										
Approach	WestBound							EastBound		
Movement	7	8	9				10	11	12	
Lane Configuration							L			
Shared Flow Rate, $v_y$							30			
Movement Capacity, $c_{m,x}$							493			
Shared Capacity, $c_{SH}$										

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate								30		
Movement Capacity								493		
Lane Configuration								L		
Shared Capacity										
Control Delay								12.8		

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration								L		
Flow Rate								30		
Lane Capacity								493		
$v/c$								0.06		
95% Queue Length								0.2		
Control Delay								12.8		

LOS  
Approach Delay  
Approach LOS  
Intersection Delay

B

12.8  
B

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/22/2019 8:12:02 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_PM\_5-Rin-out\_SMUT-x.xtw  
 Analyst: Mike Koyak  
 Agency: INDOT  
 Date Performed: 8/22/2019  
 Time Analyzed: 2045 PM S-MUT w/5Ri -o  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: Hillisdale Road @ US 41  
 Units: U. S. Customary  
 Intersection Name: South MUT  
 Major Street Direction: North-South  
 East/West Street Name: South MUT  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound			SouthBound			
Approach	1U	2	3	4U	5	6	
Movement	U	L	T	R	L	T	R
Volume			1279				
Peak Hour Factor, PHF				0.92			
Hourly Flow Rate, HFR			1390				
Percent Heavy Vehicles							
Number of Lanes	0	0	2	0	0	0	0
Lane Configuration			T				
Median Type					Undivided		
Median Storage							
RT channelized?							
Left-Turn Lane Storage							
Upstream Signal?					Not Present		

Minor Street:	WestBound			EastBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume				31		
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR				34		
Percent Heavy Vehicles				3		
Number of Lanes	0	0	0	1	0	0
Lane Configuration				L		
RT channelized?						
Flared Approach   Storage						3
Percent Grade						

Pedestrian Volumes and Adjustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)	0			0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	NB	SB	WestBound	EastBound
Movement	1U	4U	7	8
Lane Configuration				L
Flow Rate				34
Lane Capacity				333
v/c				0.10
95% Queue Length				0.3
Control Delay				17.0
LOS				C
Approach Delay				17.0
Approach LOS				C
Intersction Delay	0.4			

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound			SouthBound			
Approach	1U	2	3	4U	5	6	
Priorty	U	L	T	R	L	T	R
Movement							

Minor Street:

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement		NorthBound				SouthBound			
	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Volume, V_x				1279					
Flow Rate, v_x				1390					

Minor Street:									
Approach Movement		WestBound				EastBound			
		7	8	9			10	11	12
		L	T	R			L	T	R
Volume, V_x								31	
Flow Rate, v_x								34	

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement		NorthBound				SouthBound			
	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Flow Rate, v_x				1390					
Conflicting Flow, v_c, x									

Minor Street:									
Approach Movement		WestBound				EastBound			
		7	8	9			10	11	12
		L	T	R			L	T	R
Flow Rate, v_x								34	
Conflicting Flow, v_c, x								695	

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS										
Approach Movement		NB		SB		WestBound			EastBound	
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_c, base										
Single Stage										7.5
Stage I										
Stage II										
t_c, HV										2.0
P_HV										0.03
t_c, G										0.2
G										3
t_3, LT										0.7
t_c										
Single Stage										7.46
Stage I										
Stage II										

FOLLOW-UP HEADWAYS										
Approach Movement		NB		SB		WestBound			EastBound	
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_f, base										3.5
t_f, HV										1.0
P_HV										0.03
t_f										3.53

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach Movement		NB		SB		WestBound			EastBound	
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
v_c, x										695
t_c, x										7.46
t_f, x										3.53
c_p, x										333

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0			0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$				695
Potential Capacity, $c_{p,x}$				333
Pedestrian Impedance Factor, $p_{p,x}$				1.000
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$				1.000
Movement Capacity, $c_{m,x}$				333

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES											
Approach	WestBound						EastBound				
Movement	7	8	9	10	11	12	10	11	12		
Lane Configuration				L			L				
Shared Flow Rate, $v_y$											34
Movement Capacity, $c_{m,x}$											333
Shared Capacity, $c_{SH}$											

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS												
Approach	NB			SB			WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12		
Flow Rate											34	
Movement Capacity											333	
Lane Configuration											L	
Shared Capacity												
Control Delay											17.0	

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB			SB			WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12		
Lane Configuration								L				
Flow Rate											34	
Lane Capacity											333	
$v/c$											0.10	
95% Queue Length											0.3	
Control Delay											17.0	



LOS  
Approach Delay  
Approach LOS  
Intersection Delay

0.4

C

17.0  
C

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/22/2019 8:13:44 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_AM\_5-Rin-out-Hilldale-Walnut-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5\_Rin-out\_AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hilldale  
 Units: U.S. Customary  
 Intersection Name: Hilldale Rd / Walnut Rd  
 Major Street Direction: East-West  
 East/West Street Name: Hilldale Road  
 North/South Street Name: Walnut Avenue  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Volume		1	91	28		15	130	5	
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		1	99	30		16	141	5	
Percent Heavy Vehicles		3				3			
Number of Lanes	0	0	1	0	0	0	1	0	
Lane Configuration			LTR				LTR		
Median Type					Undivided				
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street:	NorthBound			SouthBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume	28	6	25	5	6	7
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR	30	7	27	5	7	8
Percent Heavy Vehicles	3	3	3	3	3	3
Number of Lanes	0	1	0	0	1	0
Lane Configuration		LTR			LTR	
RT channelized?						
Flared Approach   Storage	No			No		
Percent Grade		0			0	

Pedestrian Volumes and Adjustments

Approach	EB	WB	NB	SB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	1U	EB	4U	WB	4	7	NorthBound	8	9	10	SouthBound	11	12
Movement							LTR				LTR		
Lane Configuration													
Flow Rate		1		16		64				20			
Lane Capacity		1429		1450		730				692			
v/c		0.00		0.01		0.09				0.03			
95% Queue Length		0.0		0.0		0.3				0.1			
Control Delay		7.5		7.5		10.4				10.4			
LOS		A		A		B				B			
Approach Delay		0.1		0.8		10.4				10.4			
Approach LOS						B				B			
Intersection Delay		2.7											

Step 1: MOVEMENT PRIORITIES

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Priority									

Minor Street:

Approach Priority Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x		1	91	28			15	130	5
Flow Rate, v_x		1	99	30			16	141	5

Minor Street: Approach Movement	NorthBound				SouthBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Volume, V_x		28	6	25		5	6	7
Flow Rate, v_x		30	7	27		5	7	8

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		1	99	30			16	141	5
Conflicting Flow, v_c, x		147					129		

Minor Street: Approach Movement	NorthBound				SouthBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Flow Rate, v_x		30	7	27		5	7	8
Conflicting Flow, v_c, x		300	296	114		310	308	144

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	1U U	EB 1 L	4U U	WB 4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_c, base											
Single Stage		4.1		4.1	7.1	6.5	6.2	7.1	6.5	6.2	
Stage I											
Stage II											
t_c, HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	
P_HV		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1	
G					0	0	0	0	0	0	
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
t_c											
Single Stage		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23	
Stage I											
Stage II											

FOLLOW-UP HEADWAYS Approach Movement	1U U	EB 1 L	4U U	WB 4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_f, base											
t_f, HV		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3	
P_HV		0.9		0.9	0.9	0.9	0.9	0.9	0.9	0.9	
t_f		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
c_p, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	1U U	EB 1 L	4U U	WB 4 L	NorthBound			SouthBound		
					7 L	8 T	9 R	10 L	11 T	12 R
v_c, x		147		129	300	296	114	310	308	144
t_c, x		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23
t_f, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33
c_p, x		1429		1450	650	614	936	641	604	901

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	FR	WR	NR	SB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		147	129	
Potential Capacity, $c_{p,x}$		1429	1450	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		1429	1450	
Probability of Queue-free State, $p_{0,j}$		0.999	0.989	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$		0.999	0.988	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		114	144	
Potential Capacity, $c_{p,x}$		936	901	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		936	901	
Probability of Queue-free State, $p_{0,j}$		0.971	0.992	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$		296	308	
Potential Capacity, $c_{p,x}$		614	604	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Capacity Adjustment Factor, $f_x$		0.987	0.987	
Movement Capacity, $c_{m,x}$		606	596	
Probability of Queue-free State, $p_{0,j}$		0.989	0.989	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$		300	310	
Potential Capacity, $c_{p,x}$		650	641	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$		0.976	0.976	
Major L, Minor T Impedance Factor, $p'$		0.982	0.982	
Capacity Adjustment Factor, $f_x$		0.974	0.953	
Movement Capacity, $c_{m,x}$		633	611	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	NorthBound			SouthBound		
Movement	7	8	9	10	11	12
Lane Configuration		LTR			LTR	
Shared Flow Rate, $v_y$		64			20	
Movement Capacity, $c_{m,x}$	633	606	936	611	596	901
Shared Capacity, $c_{SH}$		730			692	

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB		WB		NorthBound			SouthBound		
	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		1		16	30	7	27	5	7	8
Movement Capacity		1429		1450	633	606	936	611	596	901
Lane Configuration						LTR			LTR	
Shared Capacity						730			692	
Control Delay		7.5		7.5		10.4			10.4	

CONTROL DELAY TO RANK 1 MOVEMENTS		
Approach	EB	WB
Movement	2	5
Number of Major Street Through Lanes, $N$	1	1
Proportion of Rank 1 vehicles not blocked, $p^*_{0,j}$	0.999	0.988
Delay to Major Left-turning Vehicles, $d_{MLT}$	7.5	7.5
Major Street Through Vehicles in Shared Lane, $v_{i1}$	99	141
Major Street Turning Vehicles in Shared Lane, $v_{i2}$	32	22
Saturation Flow Rate for Major Street Through, $s_{i1}$	1800	1800

Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

1500  
 0.0

1500  
 0.1

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement Lane Configuration	1U	EB 1	4U	WB 4	7	NorthBound 8	9	10	SouthBound 11	12
						LTR			LTR	
Flow Rate		1		16		64			20	
Lane Capacity		1429		1450		730			692	
v/c		0.00		0.01		0.09			0.03	
95% Queue Length		0.0		0.0		0.3			0.1	
Control Delay		7.5		7.5		10.4			10.4	
LOS		A		A		B			B	
Approach Delay		0.1		0.8		10.4			10.4	
Approach LOS						B			B	
Intersection Delay		2.7								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/20/2019 9:22:05 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_PM\_5-Rin-out-Hillisdale-Walnut-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5\_Rin-out\_PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Hillisdale Rd / Walnut Rd  
 Major Street Direction: East-West  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: Walnut Avenue  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	1U U	EastBound 1 L	2 T	3 R		4U U	WestBound 4 L	5 T	6 R
Volume		15	181	28			11	108	25
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		16	197	30			12	117	27
Percent Heavy Vehicles		3					3		
Number of Lanes	0	0	1	0		0	0	1	0
Lane Configuration			LTR					LTR	
Median Type					Undivided				
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street: Approach Movement		NorthBound 7 L	8 T	9 R			SouthBound 10 L	11 T	12 R
Volume		35	11	38			8	9	4
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		38	12	41			9	10	4
Percent Heavy Vehicles		3	3	3			3	3	3
Number of Lanes		0	1	0			0	1	0
Lane Configuration			LTR					LTR	
RT channelized?									
Flared Approach   Storage		No					No		
Percent Grade			0					0	

Pedestrian Volumes and Adjustments

Approach Movement		EB 13		WB 14		NB 15		SB 16
Flow (ped/hr)		0		0		0		0
Lane Width (ft)								
Walking Speed (ft/sec)								
Pedestrian Blockage Factor, f_pb								

Delay, Queue Length, and Level of Service

Approach Movement	1U	EB 1	4U 4	7 7	NorthBound 8 LTR	9	10	SouthBound 11 LTR	12
Lane Configuration					LTR			LTR	
Flow Rate		16			12			91	23
Lane Capacity		1432			1335			632	551
v/c		0.01			0.01			0.14	0.04
95% Queue Length		0.0			0.0			0.5	0.1
Control Delay		7.5			7.7			11.7	11.8
LOS		A			A			B	B
Approach Delay		0.6			0.7			11.7	11.8
Approach LOS								B	B
Intersection Delay		3.1							

Step 1: MOVEMENT PRIORITIES

Major Street: Approach Priority Movement	1U U	EastBound 1 L	2 T	3 R		4U U	WestBound 4 L	5 T	6 R
Minor Street:									

Approach Priority Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x		15	181	28			11	108	25
Flow Rate, v_x		16	197	30			12	117	27

Minor Street: Approach Movement	NorthBound				SouthBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Volume, V_x		35	11	38		8	9	4
Flow Rate, v_x		38	12	41		9	10	4

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		16	197	30			12	117	27
Conflicting Flow, v_c, x		145					227		

Minor Street: Approach Movement	NorthBound				SouthBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Flow Rate, v_x		38	12	41		9	10	4
Conflicting Flow, v_c, x		407	413	212		426	415	131

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_c, base											
Single Stage		4.1		4.1	7.1	6.5	6.2	7.1	6.5	6.2	
Stage I											
Stage II											
t_c, HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	
P_HV		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1	
G					0	0	0	0	0	0	
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
t_c											
Single Stage		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23	
Stage I											
Stage II											

FOLLOW-UP HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_f, base											
t_f, HV		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3	
P_HV		0.9		0.9	0.9	0.9	0.9	0.9	0.9	0.9	
t_f		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
c_p, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound		
					7 L	8 T	9 R	10 L	11 T	12 R
v_c, x		145		227	407	413	212	426	415	131
t_c, x		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23
t_f, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33
c_p, x		1432		1335	553	528	826	537	527	916

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	FR	WR	NR	SB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate v_x	0	0	0	0
Lane Width, w				
Walking Speed, S_p				
Pedestrian Blockage Factor, f_pb				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, v_c, x		145	227	
Potential Capacity, c_p, x		1432	1335	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Movement Capacity, c_m, x		1432	1335	
Probability of Queue-free State, p_0, j		0.989	0.991	
Major L-Shared Probability Queue-free State, p*_0, j		0.987	0.990	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, v_c, x		212	131	
Potential Capacity, c_p, x		826	916	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Movement Capacity, c_m, x		826	916	
Probability of Queue-free State, p_0, j		0.950	0.995	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, v_c, x				
Potential Capacity, c_p, x				
Capacity Adjustment Factor, f_x				
Movement Capacity, c_m, x				
Shared L/U Capacity, c_SH				
Probability of Queue-free State, p_0, j				
Minor-Street Through Movements		8	11	
Conflicting Flow, v_c, x		413	415	
Potential Capacity, c_p, x		528	527	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Capacity Adjustment Factor, f_x		0.977	0.977	
Movement Capacity, c_m, x		516	515	
Probability of Queue-free State, p_0, j		0.977	0.981	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, v_c, x		407	426	
Potential Capacity, c_p, x		553	537	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, p"		0.959	0.955	
Major L, Minor T Impedance Factor, p'		0.968	0.965	
Capacity Adjustment Factor, f_x		0.964	0.917	
Movement Capacity, c_m, x		533	492	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	NorthBound			SouthBound		
Movement	7	8	9	10	11	12
Lane Configuration		LTR			LTR	
Shared Flow Rate, v_y		91			23	
Movement Capacity, c_m, x	533	516	826	492	515	916
Shared Capacity, c_SH		632			551	

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		16		12	38	12	41	9	10	4
Movement Capacity		1432		1335	533	516	826	492	515	916
Lane Configuration						LTR			LTR	
Shared Capacity						632			551	
Control Delay		7.5		7.7		11.7			11.8	

CONTROL DELAY TO RANK 1 MOVEMENTS		
Approach	EB	WB
Movement	2	5
Number of Major Street Through Lanes, N	1	1
Proportion of Rank 1 vehicles not blocked, p*_0, j	0.987	0.990
Delay to Major Left-turning Vehicles, d_MLT	7.5	7.7
Major Street Through Vehicles in Shared Lane, v_i1	197	117
Major Street Turning Vehicles in Shared Lane, v_i2	47	39
Saturation Flow Rate for Major Street Through, s_i1	1800	1800



Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

1500  
 0.1

1500  
 0.1

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement Lane Configuration	1U	EB 1	4U	WB 4	7	NorthBound 8	9	10	SouthBound 11	12
						LTR			LTR	
Flow Rate		16		12		91			23	
Lane Capacity		1432		1335		632			551	
v/c		0.01		0.01		0.14			0.04	
95% Queue Length		0.0		0.0		0.5			0.1	
Control Delay		7.5		7.7		11.7			11.8	
LOS		A		A		B			B	
Approach Delay		0.6		0.7		11.7			11.8	
Approach LOS						B			B	
Intersection Delay		3.1								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/20/2019 9:20:55 AM

HCS7 Two-Way Stop-Control Text Report

File Name: TWO-WAY STOP CONTROL (TWSC) Analysis  
 2045\_AM\_5-Rin-out\_Radio-Walnut-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5-Rin-out AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Radio Ave / Walnut Rd  
 Major Street Direction: East-West  
 East/West Street Name: Radio Ave  
 North/South Street Name: Walnut Road  
 Analysis Time Period (hrs): 0.25

Vehi cl e Vol umes and Adj ustments

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Volume		3	3	4		5	0	1	
Peak Hour Factor, PHF					0.92				
Hourly Flow Rtae, HFR		3	3	4		5	0	1	
Percent Heavy Vehicles		3				3			
Number of Lanes	0	0	1	0	0	0	1	0	
Lane Configuration			LTR				LTR		
Median Type					Undivided				
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street:	NorthBound			SouthBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume	9	15	1	1	4	5
Peak Hour Factor, PHF				0.92		
Hourly Flow Rtae, HFR	10	16	1	1	4	5
Percent Heavy Vehicles	3	3	3	3	3	3
Number of Lanes	0	1	0	0	1	0
Lane Configuration		LTR			LTR	
RT channelized?						
Flared Approach   Storage	No			No		
Percent Grade		0			0	

Pedestri an Vol umes and Adj ustments

Approach	EB	WB	NB	SB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Del ay, Queue Length, and Level of Service

Approach	EB	WB	NorthBound			SouthBound			
Movement	1	4U	4	7	8	9	10	11	12
Lane Configuration	U				LTR			LTR	
Flow Rate	3		5		27			11	
Lane Capacity	1615		1606		905			969	
v/c	0.00		0.00		0.03			0.01	
95% Queue Length	0.0		0.0		0.1			0.0	
Control Delay	7.2		7.2		9.1			8.8	
LOS	A		A		A			A	
Approach Delay	2.2		6.0		9.1			8.8	
Approach LOS					A			A	
Intersction Delay	7.3								

Step 1: MOVEMENT PRI OR I TI ES

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Priority									

Minor Street:

Approach Priority Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x		3	3	4			5	0	1
Flow Rate, v_x		3	3	4			5	0	1

Minor Street: Approach Movement	NorthBound				SouthBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Volume, V_x		9	15	1		1	4	5
Flow Rate, v_x		10	16	1		1	4	5

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		3	3	4			5	0	1
Conflicting Flow, v_c, x		1					8		

Minor Street: Approach Movement	NorthBound				SouthBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Flow Rate, v_x		10	16	1		1	4	5
Conflicting Flow, v_c, x		28	24	5		32	26	1

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_c, base											
Single Stage		4.1		4.1	7.1	6.5	6.2	7.1	6.5	6.2	
Stage I											
Stage II											
t_c, HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	
P_HV		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1	
G					0	0	0	0	0	0	
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
t_c											
Single Stage		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23	
Stage I											
Stage II											

FOLLOW-UP HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_f, base											
t_f, HV		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3	
P_HV		0.9		0.9	0.9	0.9	0.9	0.9	0.9	0.9	
t_f		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
c_p, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound		
					7 L	8 T	9 R	10 L	11 T	12 R
v_c, x		1		8	28	24	5	32	26	1
t_c, x		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23
t_f, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33
c_p, x		1615		1606	979	868	1075	973	866	1081

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	FR	WR	NR	SB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate v_x	0	0	0	0
Lane Width, w				
Walking Speed, S_p				
Pedestrian Blockage Factor, f_pb				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, v_c, x		1	8	
Potential Capacity, c_p, x		1615	1606	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Movement Capacity, c_m, x		1615	1606	
Probability of Queue-free State, p_0, j		0.998	0.997	
Major L-Shared Probability Queue-free State, p*_0, j		0.998	0.997	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, v_c, x		5	1	
Potential Capacity, c_p, x		1075	1081	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Movement Capacity, c_m, x		1075	1081	
Probability of Queue-free State, p_0, j		0.999	0.995	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, v_c, x				
Potential Capacity, c_p, x				
Capacity Adjustment Factor, f_x				
Movement Capacity, c_m, x				
Shared L/U Capacity, c_SH				
Probability of Queue-free State, p_0, j				
Minor-Street Through Movements		8	11	
Conflicting Flow, v_c, x		24	26	
Potential Capacity, c_p, x		868	866	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Capacity Adjustment Factor, f_x		0.995	0.995	
Movement Capacity, c_m, x		863	861	
Probability of Queue-free State, p_0, j		0.981	0.995	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, v_c, x		28	32	
Potential Capacity, c_p, x		979	973	
Pedestrian Impedance Factor, p_p, x		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, p"		0.990	0.976	
Major L, Minor T Impedance Factor, p'		0.992	0.982	
Capacity Adjustment Factor, f_x		0.987	0.981	
Movement Capacity, c_m, x		966	954	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	NorthBound			SouthBound		
Movement	7	8	9	10	11	12
Lane Configuration		LTR			LTR	
Shared Flow Rate, v_y		27			11	
Movement Capacity, c_m, x	966	863	1075	954	861	1081
Shared Capacity, c_SH		905			969	

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		3		5	10	16	1	1	4	5
Movement Capacity		1615		1606	966	863	1075	954	861	1081
Lane Configuration						LTR			LTR	
Shared Capacity						905			969	
Control Delay		7.2		7.2		9.1			8.8	

CONTROL DELAY TO RANK 1 MOVEMENTS		
Approach	EB	WB
Movement	2	5
Number of Major Street Through Lanes, N	1	1
Proportion of Rank 1 vehicles not blocked, p*_0, j	0.998	0.997
Delay to Major Left-turning Vehicles, d_MLT	7.2	7.2
Major Street Through Vehicles in Shared Lane, v_i1	3	0
Major Street Turning Vehicles in Shared Lane, v_i2	8	7
Saturation Flow Rate for Major Street Through, s_i1	1800	1800

Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

1500  
 0.0

1500  
 0.0

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement Lane Configuration	1U	EB 1	4U	WB 4	7	NorthBound 8	9	10	SouthBound 11	12
						LTR			LTR	
Flow Rate		3		5		27			11	
Lane Capacity		1615		1606		905			969	
v/c		0.00		0.00		0.03			0.01	
95% Queue Length		0.0		0.0		0.1			0.0	
Control Delay		7.2		7.2		9.1			8.8	
LOS		A		A		A			A	
Approach Delay		2.2		6.0		9.1			8.8	
Approach LOS						A			A	
Intersection Delay		7.3								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/20/2019 9:46:29 AM

HCS7 Two-Way Stop-Control Text Report

File Name: 2045\_PM\_5-Rin-out\_Radio-Walnut-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5-Rin-out PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Radio Ave / Walnut Rd  
 Major Street Direction: East-West  
 East/West Street Name: Radio Ave  
 North/South Street Name: Walnut Road  
 Analysis Time Period (hrs): 0.25

Vehi cle Vol umes and Adj ustments

Major Street: Approach Movement	1U U	EastBound 1 L 2 T	3 R	4U U	WestBound 4 L 5 T	6 R
Volume		1 4	4		1 4	0
Peak Hour Factor, PHF				0.92		
Hourly Flow Rtae, HFR		1 4	4		1 4	0
Percent Heavy Vehicles		3			3	
Number of Lanes	0	0 1	0	0	0 1	0
Lane Configuration			LTR			LTR
Median Type				Undivided		
Median Storage						
RT channelized?						
Left-Turn Lane Storage						
Upstream Signal?				Not Present		

Minor Street: Approach Movement	NorthBound 7 L 8 T	9 R	SouthBound 10 L 11 T	12 R
Volume	8 13	1	2 13	2
Peak Hour Factor, PHF			0.92	
Hourly Flow Rtae, HFR	9 14	1	2 14	2
Percent Heavy Vehicles	3 3	3	3 3	3
Number of Lanes	0 1	0	0 1	0
Lane Configuration		LTR		LTR
RT channelized?				
Flared Approach   Storage	No	0	No	0
Percent Grade				

Pedestri an Vol umes and Adj ustments

Approach Movement	EB 13	WB 14	NB 15	SB 16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Del ay, Queue Length, and Level of Service

Approach Movement	1U U	EB 1	4U 4	WB 4	7 7	NorthBound 8 LTR	9	10	SouthBound 11 LTR	12
Lane Configuration										
Flow Rate		1		1		24			18	
Lane Capacity		1611		1605		916			904	
v/c		0.00		0.00		0.03			0.02	
95% Queue Length		0.0		0.0		0.1			0.1	
Control Delay		7.2		7.2		9.0			9.1	
LOS		A		A		A			A	
Approach Delay		0.8		1.5		9.0			9.1	
Approach LOS						A			A	
Intersction Delay		6.9								

Step 1: MOVEMENT PRI OR TI ES

Major Street: Approach Priority Movement	1U U	EastBound 1 L 2 T	3 R	4U U	WestBound 4 L 5 T	6 R
Minor Street:						

Approach Priority Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x		1	4	4			1	4	0
Flow Rate, v_x		1	4	4			1	4	0

Minor Street: Approach Movement	NorthBound				SouthBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Volume, V_x		8	13	1		2	13	2
Flow Rate, v_x		9	14	1		2	14	2

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	EastBound				4U U	WestBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		1	4	4			1	4	0
Conflicting Flow, v_c, x		4					9		

Minor Street: Approach Movement	NorthBound				SouthBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Flow Rate, v_x		9	14	1		2	14	2
Conflicting Flow, v_c, x		23	15	7		23	17	4

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_c, base											
Single Stage		4.1		4.1	7.1	6.5	6.2	7.1	6.5	6.2	
Stage I											
Stage II											
t_c, HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	
P_HV		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1	
G					0	0	0	0	0	0	
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
t_c											
Single Stage		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23	
Stage I											
Stage II											

FOLLOW-UP HEADWAYS Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound			
					7 L	8 T	9 R	10 L	11 T	12 R	
t_f, base											
t_f, HV		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3	
P_HV		0.9		0.9	0.9	0.9	0.9	0.9	0.9	0.9	
t_f		0.03		0.03	0.03	0.03	0.03	0.03	0.03	0.03	
c_p, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	EB 1U U	1 L	WB 4U U	4 L	NorthBound			SouthBound		
					7 L	8 T	9 R	10 L	11 T	12 R
v_c, x		4		9	23	15	7	23	17	4
t_c, x		4.13		4.13	7.13	6.53	6.23	7.13	6.53	6.23
t_f, x		2.23		2.23	3.53	4.03	3.33	3.53	4.03	3.33
c_p, x		1611		1605	986	877	1073	987	875	1076

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	FR	WR	NR	SB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		4	9	
Potential Capacity, $c_{p,x}$		1611	1605	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		1611	1605	
Probability of Queue-free State, $p_{0,j}$		0.999	0.999	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$		0.999	0.999	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		7	4	
Potential Capacity, $c_{p,x}$		1073	1076	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		1073	1076	
Probability of Queue-free State, $p_{0,j}$		0.999	0.998	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$		15	17	
Potential Capacity, $c_{p,x}$		877	875	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Capacity Adjustment Factor, $f_x$		0.999	0.999	
Movement Capacity, $c_{m,x}$		876	873	
Probability of Queue-free State, $p_{0,j}$		0.984	0.984	
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$		23	23	
Potential Capacity, $c_{p,x}$		986	987	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$		0.982	0.983	
Major L, Minor T Impedance Factor, $p'$		0.987	0.987	
Capacity Adjustment Factor, $f_x$		0.985	0.986	
Movement Capacity, $c_{m,x}$		971	972	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	NorthBound			SouthBound		
Movement	7	8	9	10	11	12
Lane Configuration		LTR			LTR	
Shared Flow Rate, $v_y$		24			18	
Movement Capacity, $c_{m,x}$	971	876	1073	972	873	1076
Shared Capacity, $c_{SH}$		916			904	

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB			WB						
	1U	1	4U	4	7	8	9	10	11	12
Movement										
Flow Rate		1	1	9	14	1	2	14	2	
Movement Capacity		1611	1605	971	876	1073	972	873	1076	
Lane Configuration					LTR			LTR		
Shared Capacity					916			904		
Control Delay		7.2	7.2		9.0			9.1		

CONTROL DELAY TO RANK 1 MOVEMENTS		
Approach	EB	WB
Movement	2	5
Number of Major Street Through Lanes, $N$	1	1
Proportion of Rank 1 vehicles not blocked, $p^*_{0,j}$	0.999	0.999
Delay to Major Left-turning Vehicles, $d_{MLT}$	7.2	7.2
Major Street Through Vehicles in Shared Lane, $v_{i1}$	4	4
Major Street Turning Vehicles in Shared Lane, $v_{i2}$	5	1
Saturation Flow Rate for Major Street Through, $s_{i1}$	1800	1800



Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

1500  
 0.0

1500  
 0.0

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement Lane Configuration	1U	EB 1	4U	WB 4	7	NorthBound 8	9	10	SouthBound 11	12
						LTR			LTR	
Flow Rate		1		1		24			18	
Lane Capacity		1611		1605		916			904	
v/c		0.00		0.00		0.03			0.02	
95% Queue Length		0.0		0.0		0.1			0.1	
Control Delay		7.2		7.2		9.0			9.1	
LOS		A		A		A			A	
Approach Delay		0.8		1.5		9.0			9.1	
Approach LOS						A			A	
Intersection Delay		6.9								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/20/2019 9:45:11 AM

HCS7 Two-Way Stop-Control Text Report

File Name: TWO-WAY STOP CONTROL (TWSC) Analysis  
 2045\_AM\_5-Rin-out\_Radio-US 41-x. xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5-Rin-out AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Radio Ave / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Radio Ave  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehi cle Vol umes and Adj ustments

Major Street:	NorthBound			SouthBound			
Approach	1U	2	3	4U	5	6	
Movement	U	L	T	R	L	T	R
Volume			1077	5			1167
Peak Hour Factor, PHF				0.92			
Hourly Flow Rtae, HFR			1171	5			1268
Percent Heavy Vehicles							
Number of Lanes	0	0	2	0	0	2	0
Lane Configuration			T	TR		T	
Median Type							
Median Storage							
RT channelized?							
Left-Turn Lane Storage							
Upstream Signal ?							Not Present

Minor Street:	WestBound			EastBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume			18			
Peak Hour Factor, PHF			0.92			
Hourly Flow Rtae, HFR			20			
Percent Heavy Vehicles			3			
Number of Lanes	0	0	1	0	0	0
Lane Configuration			R			
RT channelized?			No			
Flared Approach   Storage						
Percent Grade			0			

Pedestri an Vol umes and Adj ustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Del ay, Queue Length, and Level of Service

Approach	NB	SB	WestBound	EastBound
Movement	1U	4U	7	8
Lane Configuration				R
Flow Rate				20
Lane Capacity				450
v/c				0.04
95% Queue Length				0.1
Control Delay				13.4
LOS				B
Approach Delay				13.4
Approach LOS				B
Interscti on Del ay	0.1			

Step 1: MOVEMENT PRI ORI TI ES

Major Street:	NorthBound			SouthBound		
Approach	1U	2	3	4U	5	6
Pri ority	U	L	T	R	L	T
Movement						

Minor Street:

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement		NorthBound				SouthBound			
	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Volume, V_x		1077				1167			
Flow Rate, v_x		1171				1268			

Minor Street:									
Approach Movement		WestBound				EastBound			
		7	8	9			10	11	12
		L	T	R			L	T	R
Volume, V_x		18							
Flow Rate, v_x		20							

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement		NorthBound				SouthBound			
	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Flow Rate, v_x		1171				1268			
Conflicting Flow, v_c, x		5							

Minor Street:									
Approach Movement		WestBound				EastBound			
		7	8	9			10	11	12
		L	T	R			L	T	R
Flow Rate, v_x		20							
Conflicting Flow, v_c, x		588							

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS											
Approach Movement		NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12	
	U	L	U	L	L	T	R	L	T	R	
t_c, base											
Single Stage							6.9				
Stage I											
Stage II											
t_c, HV							2.0				
P_HV							0.03				
t_c, G							0.1				
G							0				
t_3, LT							0.0				
t_c											
Single Stage							6.96				
Stage I											
Stage II											

FOLLOW-UP HEADWAYS											
Approach Movement		NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12	
	U	L	U	L	L	T	R	L	T	R	
t_f, base							3.3				
t_f, HV							1.0				
P_HV							0.03				
t_f							3.33				

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT											
Approach Movement		NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12	
	U	L	U	L	L	T	R	L	T	R	
v_c, x							588				
t_c, x							6.96				
t_f, x							3.33				
c_p, x							450				

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$ Lane Width, $w$ Walking Speed, $S_p$ Pedestrian Blockage Factor, $f_{pb}$	0	0	0	
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$ Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$		588 450 1.000 450 0.956		
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Shared L/U Capacity, $c_{SH}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Major L, Minor T Adjusted Impedance Factor, $p''$ Major L, Minor T Impedance Factor, $p'$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$				

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES											
Approach	WestBound						EastBound				
Movement	7	8	9	10	11	12					
Lane Configuration			R								
Shared Flow Rate, $v_y$			20								
Movement Capacity, $c_{m,x}$			450								
Shared Capacity, $c_{SH}$											

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS												
Approach	NB			SB			WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12		
Flow Rate							20					
Movement Capacity							450					
Lane Configuration							R					
Shared Capacity												
Control Delay							13.4					

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB			SB			WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12		
Lane Configuration							R					
Flow Rate							20					
Lane Capacity							450					
$v/c$							0.04					
95% Queue Length							0.1					
Control Delay							13.4					

LOS			B
Approach Delay		13.4	
Approach LOS		B	
Intersection Delay	0.1		

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/20/2019 9:48:19 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis  
 File Name: 2045\_PM\_5-Rin-out\_Radio-US 41-x. xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5-Rin-out PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Radio Ave / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Radio Ave  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehi cle Vol umes and Adj ustments

Major Street:	NorthBound				SouthBound			
Approach	1U	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R
Volume			1040	4			1388	
Peak Hour Factor, PHF					0.92			
Hourly Flow Rtae, HFR			1130	4			1509	
Percent Heavy Vehicles								
Number of Lanes	0	0	2	0	0	0	2	0
Lane Configuration			T	TR			T	
Median Type	Undivided							
Median Storage								
RT channelized?								
Left-Turn Lane Storage								
Upstream Signal ?	Not Present							

Minor Street:	WestBound			EastBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume			5			
Peak Hour Factor, PHF				0.92		
Hourly Flow Rtae, HFR			5			
Percent Heavy Vehicles			3			
Number of Lanes	0	0	1	0	0	0
Lane Configuration			R			
RT channelized?	No					
Flared Approach   Storage						
Percent Grade		0				

Pedestri an Vol umes and Adj ustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Del ay, Queue Length, and Level of Service

Approach	NB	SB	WestBound	EastBound
Movement	1U	4U	7	8
Lane Configuration			R	
Flow Rate				5
Lane Capacity				464
v/c				0.01
95% Queue Length				0.0
Control Delay				12.9
LOS				B
Approach Delay				12.9
Approach LOS				B
Intersction Delay	0.0			

Step 1: MOVEMENT PRI OR I TI ES

Major Street:	NorthBound				SouthBound			
Approach	1U	2	3	4U	4	5	6	
Pri ority Movement	U	L	T	R	U	L	T	R
Minor Street:								

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement		NorthBound				SouthBound			
	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Volume, V_x		1040				1388			
Flow Rate, v_x		1130				1509			

Minor Street:								
Approach Movement		WestBound				EastBound		
		7	8	9		10	11	12
		L	T	R		L	T	R
Volume, V_x		5						
Flow Rate, v_x		5						

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement		NorthBound				SouthBound			
	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Flow Rate, v_x		1130				1509			
Conflicting Flow, v_c, x		4							

Minor Street:								
Approach Movement		WestBound				EastBound		
		7	8	9		10	11	12
		L	T	R		L	T	R
Flow Rate, v_x		5						
Conflicting Flow, v_c, x		567						

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS											
Approach Movement		NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12	
	U	L	U	L	L	T	R	L	T	R	
t_c, base											
Single Stage							6.9				
Stage I											
Stage II											
t_c, HV							2.0				
P_HV							0.03				
t_c, G							0.1				
G							0				
t_3, LT							0.0				
t_c											
Single Stage							6.96				
Stage I											
Stage II											

FOLLOW-UP HEADWAYS											
Approach Movement		NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12	
	U	L	U	L	L	T	R	L	T	R	
t_f, base							3.3				
t_f, HV							1.0				
P_HV							0.03				
t_f							3.33				

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT											
Approach Movement		NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12	
	U	L	U	L	L	T	R	L	T	R	
v_c, x							567				
t_c, x							6.96				
t_f, x							3.33				
c_p, x							464				

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$ Lane Width, $w$ Walking Speed, $S_p$ Pedestrian Blockage Factor, $f_{pb}$	0	0	0	
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$ Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$		567 464 1.000 464 0.988		
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Shared L/U Capacity, $c_{SH}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Major L, Minor T Adjusted Impedance Factor, $p''$ Major L, Minor T Impedance Factor, $p'$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$				

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES										
Approach		WestBound							EastBound	
Movement	7	8	9					10	11	12
Lane Configuration			R							
Shared Flow Rate, $v_y$ Movement Capacity, $c_{m,x}$ Shared Capacity, $c_{SH}$			5 464							

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach		NB		SB		WestBound			EastBound	
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate Movement Capacity Lane Configuration Shared Capacity Control Delay							5 464 R 12.9			

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach		NB		SB		WestBound			EastBound	
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration							R			
Flow Rate Lane Capacity $v/c$ 95% Queue Length Control Delay							5 464 0.01 0.0 12.9			





HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_AM\_5-Rin-out\_Campbell-Walnut-x.txt  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5-Rin-out AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Campbell Rd / Walnut Rd  
 Major Street Direction: East-West  
 East/West Street Name: Campbell Road  
 North/South Street Name: Walnut Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street: Approach Movement	1U U	EastBound 1 L	2 T	3 R	4U U	WestBound 4 L	5 T	6 R
Volume			5	3		1	16	
Peak Hour Factor, PHF					0.92			
Hourly Flow Rate, HFR			5	3		1	17	
Percent Heavy Vehicles						3		
Number of Lanes	0	0	1	1	0	1	1	0
Lane Configuration			T	R		L	T	
Median Type	Undivided							
Median Storage								
RT channelized?	No							
Left-Turn Lane Storage								
Upstream Signal?	Not Present							

Minor Street: Approach Movement	NorthBound 7 L	8 T	9 R	SouthBound 10 L	11 T	12 R
Volume	18		3			
Peak Hour Factor, PHF						0.92
Hourly Flow Rate, HFR	20		3			
Percent Heavy Vehicles	3		3			
Number of Lanes	1	0	1	0	0	0
Lane Configuration	L		R			
RT channelized?			No			
Flared Approach   Storage						
Percent Grade		0				

Pedestrian Volumes and Adjustments

Approach Movement	EB 13	WB 14	NB 15	SB 16
Flow (ped/hr)	0	0	0	
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach Movement	1U U	EB 1	4U 4	WB 4	NorthBound 7 L	8 L	9 R	SouthBound 10 L	11 T	12 R
Lane Configuration				L	L		R			
Flow Rate				1	20		3			
Lane Capacity				1605	987		1075			
v/c				0.00	0.02		0.00			
95% Queue Length				0.0	0.1		0.0			
Control Delay				7.2	8.7		8.4			
LOS				A	A		A			
Approach Delay				0.4		8.7				
Approach LOS						A				
Intersection Delay		4.1								

Step 1: MOVEMENT PRIORITIES

Major Street: Approach Priority Movement	1U U	EastBound 1 L	2 T	3 R	4U U	WestBound 4 L	5 T	6 R
Minor Street:								

Approach Priority Movement	NorthBound				SouthBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:								
Approach Movement	1U	EastBound			4U	WestBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Volume, V_x		5	3			1	16	
Flow Rate, v_x		5	3			1	17	

Minor Street:							
Approach Movement	NorthBound				SouthBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Volume, V_x	18		3				
Flow Rate, v_x	20		3				

Step 3: CONFLICTING FLOW RATES

Major Street:								
Approach Movement	1U	EastBound			4U	WestBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Flow Rate, v_x		5	3			1	17	
Conflicting Flow, v_c, x						9		

Minor Street:							
Approach Movement	NorthBound				SouthBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Flow Rate, v_x	20		3				
Conflicting Flow, v_c, x	25		5				

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS												
Approach Movement	EB	1	WB	4	7	NorthBound	8	9	SouthBound	10	11	12
	1U	L	4U	L	L		T	R		L	T	R
	U		U									
t_c, base												
Single Stage				4.1	7.1			6.2				
Stage I												
Stage II												
t_c, HV				1.0	1.0			1.0				
P_HV				0.03	0.03			0.03				
t_c, G					0.2			0.1				
G					0			0				
t_3, LT				0.0	0.7			0.0				
t_c												
Single Stage				4.13	6.43			6.23				
Stage I												
Stage II												

FOLLOW-UP HEADWAYS												
Approach Movement	EB	1	WB	4	7	NorthBound	8	9	SouthBound	10	11	12
	1U	L	4U	L	L		T	R		L	T	R
	U		U									
t_f, base				2.2	3.5			3.3				
t_f, HV				0.9	0.9			0.9				
P_HV				0.03	0.03			0.03				
t_f				2.23	3.53			3.33				

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT												
Approach Movement	EB	1	WB	4	7	NorthBound	8	9	SouthBound	10	11	12
	1U	L	4U	L	L		T	R		L	T	R
	U		U									
v_c, x				9	25			5				
t_c, x				4.13	6.43			6.23				
t_f, x				2.23	3.53			3.33				
c_p, x				1605	988			1075				

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	FR	WR	NR	SB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$ Lane Width, $w$ Walking Speed, $S_p$ Pedestrian Blockage Factor, $f_{pb}$	0	0	0	
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$ Major L-Shared Probability Queue-free State, $p^*_{0,j}$			9 1605 1.000 1605 0.999	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$		5 1075 1.000 1075 0.997		
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Shared L/U Capacity, $c_{SH}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Major L, Minor T Adjusted Impedance Factor, $p''$ Major L, Minor T Impedance Factor, $p'$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$		25 988 1.000   0.999 987		

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES										
Approach	NorthBound					SouthBound				
Movement	7	8	9		10	11	12			
Lane Configuration	L		R							
Shared Flow Rate, $v_y$	20		3							
Movement Capacity, $c_{m,x}$	987		1075							
Shared Capacity, $c_{SH}$										

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate	1				20		3			
Movement Capacity	1605				987		1075			
Lane Configuration	L				L		R			
Shared Capacity										
Control Delay	7.2				8.7		8.4			

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration				L	L		R			
Flow Rate	1				20		3			
Lane Capacity	1605				987		1075			
$v/c$	0.00				0.02		0.00			
95% Queue Length	0.0				0.1		0.0			
Control Delay	7.2				8.7		8.4			

LOS		A	A		A
Approach Delay		0.4		8.7	
Approach LOS				A	
Intersection Delay	4.1				

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/20/2019 9:54:06 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_PM\_5-Rin-out\_Campbell-Walnut-x.txt  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5-Rin-out PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Campbell Rd / Walnut Rd  
 Major Street Direction: East-West  
 East/West Street Name: Campbell Road  
 North/South Street Name: Walnut Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Volume			19	22		1	6		
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR			21	24		1	7		
Percent Heavy Vehicles						3			
Number of Lanes	0	0	1	1	0	1	1	0	
Lane Configuration			T	R		L	T		
Median Type	Undivided								
Median Storage									
RT channelized?	No								
Left-Turn Lane Storage									
Upstream Signal?	Not Present								

Minor Street:	NorthBound			SouthBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume	10		0			
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR	11		0			
Percent Heavy Vehicles	3		3			
Number of Lanes	1	0	1	0	0	0
Lane Configuration	L		R			
RT channelized?			No			
Flared Approach   Storage						
Percent Grade		0				

Pedestrian Volumes and Adjustments

Approach	EB	WB	NB	SB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f <sub>pb</sub>				

Delay, Queue Length, and Level of Service

Approach	EB	WB	NorthBound			SouthBound		
Movement	1U	4U	7	8	9	10	11	12
Lane Configuration		L	L		R			
Flow Rate		1	11		0			
Lane Capacity		1557	982		1054			
v/c		0.00	0.01		0.00			
95% Queue Length		0.0	0.0		0.0			
Control Delay		7.3	8.7		8.4			
LOS		A	A		A			
Approach Delay		1.0		8.7				
Approach LOS				A				
Intersection Delay	1.6							

Step 1: MOVEMENT PRIORITIES

Major Street:	EastBound						WestBound		
Approach	1U	1	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R	
Priority									

Minor Street:

Approach Priority Movement	NorthBound				SouthBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:								
Approach Movement	1U	EastBound			4U	WestBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Volume, V_x		19	22			1	6	
Flow Rate, v_x		21	24			1	7	

Minor Street:							
Approach Movement	NorthBound				SouthBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Volume, V_x	10		0				
Flow Rate, v_x	11		0				

Step 3: CONFLICTING FLOW RATES

Major Street:								
Approach Movement	1U	EastBound			4U	WestBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Flow Rate, v_x		21	24			1	7	
Conflicting Flow, v_c, x						45		

Minor Street:							
Approach Movement	NorthBound				SouthBound		
	7	8	9		10	11	12
	L	T	R		L	T	R
Flow Rate, v_x	11		0				
Conflicting Flow, v_c, x	29		21				

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS														
Approach Movement	1U	EB	1	4U	WB	4	7	NorthBound	8	9	10	SouthBound	11	12
	U		L	U		L	L		T	R	L		T	R
t_c, base							4.1		7.1		6.2			
Single Stage														
Stage I														
Stage II														
t_c, HV							1.0		1.0		1.0			
P_HV							0.03		0.03		0.03			
t_c, G									0.2		0.1			
G									0		0			
t_3, LT							0.0		0.7		0.0			
t_c														
Single Stage							4.13		6.43		6.23			
Stage I														
Stage II														

FOLLOW-UP HEADWAYS														
Approach Movement	1U	EB	1	4U	WB	4	7	NorthBound	8	9	10	SouthBound	11	12
	U		L	U		L	L		T	R	L		T	R
t_f, base							2.2		3.5		3.3			
t_f, HV							0.9		0.9		0.9			
P_HV							0.03		0.03		0.03			
t_f							2.23		3.53		3.33			

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT														
Approach Movement	1U	EB	1	4U	WB	4	7	NorthBound	8	9	10	SouthBound	11	12
	U		L	U		L	L		T	R	L		T	R
v_c, x							45		29		21			
t_c, x							4.13		6.43		6.23			
t_f, x							2.23		3.53		3.33			
c_p, x							1557		983		1054			

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	FR	WR	NR	SB

Movement	13	14	15	16
Pedestrian Flow Rate v_x Lane Width, w Walking Speed, S_p Pedestrian Blockage Factor, f_pb	0	0	0	
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j Major L-Shared Probability Queue-free State, p*_0, j			45 1557 1.000 1557 0.999	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j		21 1054 1.000 1054 1.000		
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Capacity Adjustment Factor, f_x Movement Capacity, c_m, x Shared L/U Capacity, c_SH Probability of Queue-free State, p_0, j				
Minor-Street Through Movements		8	11	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Capacity Adjustment Factor, f_x Movement Capacity, c_m, x Probability of Queue-free State, p_0, j				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, v_c, x Potential Capacity, c_p, x Pedestrian Impedance Factor, p_p, x Major L, Minor T Adjusted Impedance Factor, p" Major L, Minor T Impedance Factor, p' Capacity Adjustment Factor, f_x Movement Capacity, c_m, x		29 983 1.000   0.999 982		

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES										
Approach	NorthBound				SouthBound					
Movement	7	8	9		10	11	12			
Lane Configuration	L		R							
Shared Flow Rate, v_y	11		0							
Movement Capacity, c_m, x	982		1054							
Shared Capacity, c_SH										

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate										
Movement Capacity										
Lane Configuration										
Shared Capacity										
Control Delay										
Flow Rate	1	11					0			
Movement Capacity	1557	982					1054			
Lane Configuration	L	L					R			
Shared Capacity										
Control Delay	7.3	8.7					8.4			

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration										
Flow Rate										
Lane Capacity										
v/c										
95% Queue Length										
Control Delay										
Flow Rate	1	11					0			
Lane Capacity	1557	982					1054			
v/c	0.00	0.01					0.00			
95% Queue Length	0.0	0.0					0.0			
Control Delay	7.3	8.7					8.4			



LOS		A	A		A
Approach Delay		1.0		8.7	
Approach LOS				A	
Intersection Delay	1.6				

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/20/2019 9:56:22 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis  
 File Name: 2045\_AM\_5-Rin-out\_Campbell-OSR-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5-Rin-out AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Campbell Rd / OSR  
 Major Street Direction: East-West  
 East/West Street Name: Campbell Road  
 North/South Street Name: Old State Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	EastBound						WestBound					
Approach	1U		2		3		4U		5		6	
Movement	U	L	T	R		U	L	T	R	U	L	R
Volume		47	32					14	22			
Peak Hour Factor, PHF					0.92							
Hourly Flow Rate, HFR		51	35					15	24			
Percent Heavy Vehicles		3										
Number of Lanes	0	1	1	0		0	0	1	1			
Lane Configuration		L	T					T	R			
Median Type	Undivided											
Median Storage												
RT channelized?												
Left-Turn Lane Storage												
Upstream Signal?	Not Present											

Minor Street:	NorthBound						SouthBound					
Approach	7		8		9		10		11		12	
Movement	L	T	R		L	T	R	L	T	R	L	R
Volume								18			81	
Peak Hour Factor, PHF					0.92							
Hourly Flow Rate, HFR								20			88	
Percent Heavy Vehicles								3			3	
Number of Lanes	0	0	0					1	0		1	
Lane Configuration								L			R	
RT channelized?												
Flared Approach   Storage												
Percent Grade												

Pedestrian Volumes and Adjustments

Approach	EB			WB			NB			SB		
Movement	13			14			15			16		
Flow (ped/hr)	0			0						0		
Lane Width (ft)												
Walking Speed (ft/sec)												
Pedestrian Blockage Factor, f_pb												

Delay, Queue Length, and Level of Service

Approach	EB			WB			NorthBound			SouthBound		
Movement	1U	4U	7	8	9	10	11	12	L	T	R	R
Lane Configuration	L	T	R	L	T	R	L	T	L	T	R	R
Flow Rate	51						20					88
Lane Capacity	1564						810					1061
v/c	0.03						0.02					0.08
95% Queue Length	0.1						0.1					0.3
Control Delay	7.4						9.6					8.7
LOS	A						A					A
Approach Delay	4.4									8.9		
Approach LOS										A		
Intersection Delay	5.7											

Step 1: MOVEMENT PRIORITIES

Major Street:	EastBound						WestBound					
Approach	1U		2		3		4U		5		6	
Priority	U	L	T	R		U	L	T	R	U	L	R

Minor Street:

Approach Priority Movement	NorthBound				SouthBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement	1U	EastBound		3		4U	WestBound		
	U	1	2	R		U	4	5	6
		L	T				L	T	R
Volume, V_x		47	32				14	22	
Flow Rate, v_x		51	35				15	24	

Minor Street:									
Approach Movement		NorthBound				SouthBound			
		7	8	9		10	11	12	
		L	T	R		L	T	R	
Volume, V_x						18		81	
Flow Rate, v_x						20		88	

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement	1U	EastBound		3		4U	WestBound		
	U	1	2	R		U	4	5	6
		L	T				L	T	R
Flow Rate, v_x		51	35				15	24	
Conflicting Flow, v_c, x		39							

Minor Street:									
Approach Movement		NorthBound				SouthBound			
		7	8	9		10	11	12	
		L	T	R		L	T	R	
Flow Rate, v_x						20		88	
Conflicting Flow, v_c, x						152		15	

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS														
Approach Movement	1U	EB	1	4U	WB	4	7	NorthBound	8	9	SouthBound	10	11	12
	U		L	U		L	L		T	R	L	T	R	
t_c, base														
Single Stage			4.1								7.1		6.2	
Stage I														
Stage II														
t_c, HV			1.0								1.0		1.0	
P_HV			0.03								0.03		0.03	
t_c, G											0.2		0.1	
G											0		0	
t_3, LT			0.0								0.7		0.0	
t_c														
Single Stage			4.13								6.43		6.23	
Stage I														
Stage II														

FOLLOW-UP HEADWAYS														
Approach Movement	1U	EB	1	4U	WB	4	7	NorthBound	8	9	SouthBound	10	11	12
	U		L	U		L	L		T	R	L	T	R	
t_f, base			2.2								3.5		3.3	
t_f, HV			0.9								0.9		0.9	
P_HV			0.03								0.03		0.03	
t_f			2.23								3.53		3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT														
Approach Movement	1U	EB	1	4U	WB	4	7	NorthBound	8	9	SouthBound	10	11	12
	U		L	U		L	L		T	R	L	T	R	
v_c, x			39								152		15	
t_c, x			4.13								6.43		6.23	
t_f, x			2.23								3.53		3.33	
c_p, x			1564								837		1061	

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	FR	WR	NR	SB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0		0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		39		
Potential Capacity, $c_{p,x}$		1564		
Pedestrian Impedance Factor, $p_{p,x}$		1.000		
Movement Capacity, $c_{m,x}$		1564		
Probability of Queue-free State, $p_{0,j}$		0.967		
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$			15	
Potential Capacity, $c_{p,x}$			1061	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Movement Capacity, $c_{m,x}$			1061	
Probability of Queue-free State, $p_{0,j}$			0.917	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$			152	
Potential Capacity, $c_{p,x}$			837	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$			0.967	
Movement Capacity, $c_{m,x}$			810	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES											
Approach	NorthBound						SouthBound				
Movement	7	8	9				10	11	12		
Lane Configuration							L		R		
Shared Flow Rate, $v_y$							20		88		
Movement Capacity, $c_{m,x}$							810		1061		
Shared Capacity, $c_{SH}$											

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS												
Approach	EB			WB			NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12		
Flow Rate		51						20		88		
Movement Capacity		1564						810		1061		
Lane Configuration		L						L		R		
Shared Capacity												
Control Delay		7.4						9.6		8.7		

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	EB			WB			NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12		
Lane Configuration		L						L		R		
Flow Rate		51						20		88		
Lane Capacity		1564						810		1061		
$v/c$		0.03						0.02		0.08		
95% Queue Length		0.1						0.1		0.3		
Control Delay		7.4						9.6		8.7		

LOS	A	A		A
Approach Delay	4.4		8.9	
Approach LOS			A	
Intersection Delay	5.7			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/20/2019 9:58:26 AM

HCS7 Two-Way Stop-Control Text Report

File Name: TWO-WAY STOP CONTROL (TWSC) Analysis  
 2045\_PM\_5-Rin-out\_Campbell-OSR-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5-Rin-out PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Campbell Rd / OSR  
 Major Street Direction: East-West  
 East/West Street Name: Campbell Road  
 North/South Street Name: Old State Road  
 Analysis Time Period (hrs): 0.25

Vehi cle Vol umes and Adj ustments

Major Street:							
Approach	EastBound			WestBound			
Movement	1U	2	3	4U	5	6	
	U	L	T	R	L	T	R
Volume		56	22			11	9
Peak Hour Factor, PHF				0.92			
Hourly Flow Rtae, HFR		61	24			12	10
Percent Heavy Vehicles		3					
Number of Lanes	0	1	1	0	0	1	1
Lane Configuration		L	T			T	R
Median Type	Undivided						
Median Storage							
RT channelized?							
Left-Turn Lane Storage							
Upstream Signal?	Not Present						

Minor Street:						
Approach	NorthBound			SouthBound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume					33	42
Peak Hour Factor, PHF				0.92		
Hourly Flow Rtae, HFR					36	46
Percent Heavy Vehicles					3	3
Number of Lanes	0	0	0	1	0	1
Lane Configuration				L		R
RT channelized?						
Flared Approach   Storage						
Percent Grade						

Pedestri an Vol umes and Adj ustments

Approach	EB	WB	NB	SB
Movement	13	14	15	16
Flow (ped/hr)	0	0		0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Del ay, Queue Length, and Level of Service

Approach	NorthBound			SouthBound				
Movement	1U	4U	7	8	9	10	11	12
	U	L	T	R		L	T	R
Lane Configuration								
Flow Rate	61					36		46
Lane Capacity	1587					799		1066
v/c	0.04					0.04		0.04
95% Queue Length	0.1					0.1		0.1
Control Delay	7.4					9.7		8.5
LOS	A					A		A
Approach Delay	5.3						9.1	
Approach LOS							A	
Intersction Delay	6.3							

Step 1: MOVEMENT PRI OR TI ES

Major Street:							
Approach	EastBound			WestBound			
Pri ority	1U	2	3	4U	5	6	
Movement	U	L	T	R	L	T	R
Minor Street:							

Approach Priority Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	EastBound		3 R		4U U	WestBound		6 R
		1 L	2 T				4 L	5 T	
Volume, V_x		56	22				11	9	
Flow Rate, v_x		61	24				12	10	

Minor Street: Approach Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R
Volume, V_x					33		42
Flow Rate, v_x					36		46

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	EastBound		3 R		4U U	WestBound		6 R
		1 L	2 T				4 L	5 T	
Flow Rate, v_x		61	24				12	10	
Conflicting Flow, v_c, x		22							

Minor Street: Approach Movement	NorthBound				SouthBound		
	7 L	8 T	9 R		10 L	11 T	12 R
Flow Rate, v_x					36		46
Conflicting Flow, v_c, x					158		12

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS										
Approach Movement	EB	WB			NorthBound			SouthBound		
	1U U	1 L	4U U	4 L	7 L	8 T	9 R	10 L	11 T	12 R
t_c, base										
Single Stage		4.1						7.1		6.2
Stage I										
Stage II										
t_c, HV		1.0						1.0		1.0
P_HV		0.03						0.03		0.03
t_c, G								0.2		0.1
G								0		0
t_3, LT		0.0						0.7		0.0
t_c										
Single Stage		4.13						6.43		6.23
Stage I										
Stage II										

FOLLOW-UP HEADWAYS										
Approach Movement	EB	WB			NorthBound			SouthBound		
	1U U	1 L	4U U	4 L	7 L	8 T	9 R	10 L	11 T	12 R
t_f, base		2.2						3.5		3.3
t_f, HV		0.9						0.9		0.9
P_HV		0.03						0.03		0.03
t_f		2.23						3.53		3.33

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach Movement	EB	WB			NorthBound			SouthBound		
	1U U	1 L	4U U	4 L	7 L	8 T	9 R	10 L	11 T	12 R
v_c, x		22						158		12
t_c, x		4.13						6.43		6.23
t_f, x		2.23						3.53		3.33
c_p, x		1587						831		1066

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	FR	WR	NR	SB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0		0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		22		
Potential Capacity, $c_{p,x}$		1587		
Pedestrian Impedance Factor, $p_{p,x}$		1.000		
Movement Capacity, $c_{m,x}$		1587		
Probability of Queue-free State, $p_{0,j}$		0.962		
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$			12	
Potential Capacity, $c_{p,x}$			1066	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Movement Capacity, $c_{m,x}$			1066	
Probability of Queue-free State, $p_{0,j}$			0.957	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$			158	
Potential Capacity, $c_{p,x}$			831	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$			0.962	
Movement Capacity, $c_{m,x}$			799	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES										
Approach	NorthBound				SouthBound					
Movement	7	8	9		10	11	12			
Lane Configuration					L		R			
Shared Flow Rate, $v_y$					36		46			
Movement Capacity, $c_{m,x}$					799		1066			
Shared Capacity, $c_{SH}$										

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		61						36		46
Movement Capacity		1587						799		1066
Lane Configuration		L						L		R
Shared Capacity										
Control Delay		7.4						9.7		8.5

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	EB		WB		NorthBound			SouthBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration		L						L		R
Flow Rate		61						36		46
Lane Capacity		1587						799		1066
$v/c$		0.04						0.04		0.04
95% Queue Length		0.1						0.1		0.1
Control Delay		7.4						9.7		8.5



LOS	A	A		A
Approach Delay	5.3		9.1	
Approach LOS			A	
Intersection Delay	6.3			

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/20/2019 10:00:26 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_AM\_5-Rin-out\_Old State-US 41.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 8/2/2019  
 Time Analyzed: 5\_Rin-out AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Old State Rd / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Old State Road  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound				SouthBound				
Approach Movement	1U	2	3	4U	4	5	6		
	U	L	T	R	U	L	T	R	
Volume	0	0	1258	14	0	4	1180	49	
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR						4	1283	53	
Percent Heavy Vehicles	3	0			3	3			
Number of Lanes	0	1	2	0	0	1	2	1	
Lane Configuration						L	T	R	
Median Type	Undivided								
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?	Not Present								

Minor Street:	WestBound			EastBound			
Approach Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume	26	19	6	89	32	8	
Peak Hour Factor, PHF				0.92			
Hourly Flow Rate, HFR	28	21	7	97	35	9	
Percent Heavy Vehicles	0	3	3	0	3	3	
Number of Lanes	0	1	0	0	1	0	
Lane Configuration					LTR		
RT channelized?							
Flared Approach   Storage	No			No			
Percent Grade					0		

Pedestrian Volumes and Adjustments

Approach Movement	NB	SB	WB	EB
	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach Movement	NB	4U	7	8	9	10	11	12
	1	L	L	LTR			LTR	
Lane Configuration								
Flow Rate	0	4	55	140				
Lane Capacity	523	486						
v/c	0.00	0.01						
95% Queue Length	0.0	0.0						
Control Delay	11.9	12.5						
LOS	B	B						
Approach Delay	0.0	0.0						
Approach LOS								
Intersction Delay								

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound				SouthBound			
Approach Priority Movement	1U	2	3	4U	4	5	6	
	U	L	T	R	U	L	T	R
Minor Street:								

Approach Priority Movement	WestBound				EastBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x	0	0	1258	14		0	4	1180	49
Flow Rate, v_x		0	1367	15			4	1283	53

Minor Street: Approach Movement	WestBound				EastBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Volume, V_x		26	19	6		89	32	8
Flow Rate, v_x		28	21	7		97	35	9

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		0	1367	15		4	1283	53	
Conflicting Flow, v_c, x		1336				1383			

Minor Street: Approach Movement	WestBound				EastBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Flow Rate, v_x		28	21	7		97	35	9
Conflicting Flow, v_c, x		2042	2720	691		1985	2674	641

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	1U U	NB	4U U	SB	4	7	WestBound		10	EastBound	
		1 L					8 T	9 R		11 T	12 R
t_c, base											
Single Stage		4.1		4.1	7.5	6.5	6.9	7.5	6.5	6.9	
Stage I											
Stage II											
t_c, HV		2.0		2.0	2.0	2.0	2.0	2.0	2.0	2.0	
P_HV		0.00		0.03	0.00	0.03	0.03	0.00	0.03	0.03	
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1	
G					0	0	0	0	0	0	
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
t_c											
Single Stage		4.10		4.16	7.50	6.56	6.96	7.50	6.56	6.96	
Stage I											
Stage II											

FOLLOW-UP HEADWAYS Approach Movement	1U U	NB	4U U	SB	4	7	WestBound		10	EastBound	
		1 L					8 T	9 R		11 T	12 R
t_f, base											
t_f, HV		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3	
P_HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	
t_f		0.00		0.03	0.00	0.03	0.03	0.00	0.03	0.03	
		2.20		2.23	3.50	4.03	3.33	3.50	4.03	3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	1U U	NB	4U U	SB	4	7	WestBound		10	EastBound	
		1 L					8 T	9 R		11 T	12 R
v_c, x		1336		1383	2042	2720	691	1985	2674	641	
t_c, x		4.10		4.16	7.50	6.56	6.96	7.50	6.56	6.96	
t_f, x		2.20		2.23	3.50	4.03	3.33	3.50	4.03	3.33	
c_p, x		523		486	34	20	384	37	22	415	

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	NR	SR	WR	EB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		1336	1383	
Potential Capacity, $c_{p,x}$		523	486	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		523	486	
Probability of Queue-free State, $p_{0,j}$		1.000	0.991	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		691	641	
Potential Capacity, $c_{p,x}$		384	415	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		384	415	
Probability of Queue-free State, $p_{0,j}$		0.983	0.979	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$		2720	2674	
Potential Capacity, $c_{p,x}$		20	22	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Capacity Adjustment Factor, $f_x$		0.991	0.991	
Movement Capacity, $c_{m,x}$		20	21	
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$		2042	1985	
Potential Capacity, $c_{p,x}$		34	37	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES										
Approach	WestBound				EastBound					
Movement	7	8	9	10	11	12				
Lane Configuration		LTR			LTR					
Shared Flow Rate, $v_y$		55			140					
Movement Capacity, $c_{m,x}$		20	384		21	415				
Shared Capacity, $c_{SH}$										

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		0	4	28	21	7	97	35	9	
Movement Capacity		523	486	20	384	21	415			
Lane Configuration		L	L	LTR				LTR		
Shared Capacity										
Control Delay		11.9	12.5							

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration		L	L	LTR					LTR	
Flow Rate		0	4	55				140		
Lane Capacity		523	486							
$v/c$		0.00	0.01							
95% Queue Length		0.0	0.0							
Control Delay		11.9	12.5							

LOS  
Approach Delay  
Approach LOS  
Intersection Delay

B  
0.0

B  
0.0

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/20/2019 10:07:14 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_PM\_5-Rin-out\_Old State-US 41.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 8/2/2019  
 Time Analyzed: 5\_Rin-out PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Old State Rd / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Old State Road  
 North/South Street Name: US 41  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound						SouthBound		
Approach Movement	1U	1	2	3	4U	4	5	6	
	U	L	T	R	U	L	T	R	
Volume	0	8	1166	21	0	9	1414	161	
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		9	1267	23		10	1537	175	
Percent Heavy Vehicles	3	0			3	3			
Number of Lanes	0	1	2	0	0	1	2	1	
Lane Configuration		L	T	TR		L	T	R	
Median Type	Undivided								
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?	Not Present								

Minor Street:	WestBound			EastBound		
Approach Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume	24	25	5	48	19	5
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR	26	27	5	52	21	5
Percent Heavy Vehicles	0	3	3	0	3	3
Number of Lanes	0	1	0	0	1	0
Lane Configuration		LTR			LTR	
RT channelized?						
Flared Approach   Storage	No			No		
Percent Grade		0			0	

Pedestrian Volumes and Adjustments

Approach Movement	NB	SB	WB	EB
	13	14	15	16
Flow (ped/hr)	0	0	0	0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach Movement	1U	NB	4U	SB	7	8	9	10	EastBound	11	12
	U	L	L	L	L	LTR			LTR		
Flow Rate	9		10		59			78			
Lane Capacity	376		528								
v/c	0.02		0.02								
95% Queue Length	0.1		0.1								
Control Delay	14.8		11.9								
LOS	B		B								
Approach Delay	0.1		0.1								
Approach LOS											
Intersction Delay											

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound						SouthBound		
Approach Priority Movement	1U	1	2	3	4U	4	5	6	
	U	L	T	R	U	L	T	R	
Minor Street:									

Approach Priority Movement	WestBound				EastBound		
	7 L	8 T	9 R		10 L	11 T	12 R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Volume, V_x	0	8	1166	21		0	9	1414	161
Flow Rate, v_x		9	1267	23			10	1537	175

Minor Street: Approach Movement	WestBound				EastBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Volume, V_x		24	25	5		48	19	5
Flow Rate, v_x		26	27	5		52	21	5

Step 3: CONFLICTING FLOW RATES

Major Street: Approach Movement	1U U	NorthBound				4U U	SouthBound		
		1 L	2 T	3 R			4 L	5 T	6 R
Flow Rate, v_x		9	1267	23		10	1537	175	
Conflicting Flow, v_c, x		1712				1290			

Minor Street: Approach Movement	WestBound				EastBound			
	7 L	8 T	9 R		10 L	11 T	12 R	
Flow Rate, v_x		26	27	5		52	21	5
Conflicting Flow, v_c, x		2095	3028	645		2221	2864	768

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS Approach Movement	1U U	NB	4U U	SB	4	7	WestBound		10	EastBound	
		1 L					8 T	9 R		11 T	12 R
t_c, base											
Single Stage		4.1		4.1	7.5	6.5	6.9	7.5	6.5	6.9	
Stage I											
Stage II											
t_c, HV		2.0		2.0	2.0	2.0	2.0	2.0	2.0	2.0	
P_HV		0.00		0.03	0.00	0.03	0.03	0.00	0.03	0.03	
t_c, G					0.2	0.2	0.1	0.2	0.2	0.1	
G					0	0	0	0	0	0	
t_3, LT		0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
t_c											
Single Stage		4.10		4.16	7.50	6.56	6.96	7.50	6.56	6.96	
Stage I											
Stage II											

FOLLOW-UP HEADWAYS Approach Movement	1U U	NB	4U U	SB	4	7	WestBound		10	EastBound	
		1 L					8 T	9 R		11 T	12 R
t_f, base											
t_f, HV		2.2		2.2	3.5	4.0	3.3	3.5	4.0	3.3	
P_HV		1.0		1.0	1.0	1.0	1.0	1.0	1.0	1.0	
t_f		0.00		0.03	0.00	0.03	0.03	0.00	0.03	0.03	
c_p, x		2.20		2.23	3.50	4.03	3.33	3.50	4.03	3.33	

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT Approach Movement	1U U	NB	4U U	SB	4	7	WestBound		10	EastBound	
		1 L					8 T	9 R		11 T	12 R
v_c, x		1712		1290	2095	3028	645	2221	2864	768	
t_c, x		4.10		4.16	7.50	6.56	6.96	7.50	6.56	6.96	
t_f, x		2.20		2.23	3.50	4.03	3.33	3.50	4.03	3.33	
c_p, x		376		528	31	13	412	25	16	342	

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance Approach	NR	SR	WR	EB
----------------------------------	----	----	----	----

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		1712	1290	
Potential Capacity, $c_{p,x}$		376	528	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		376	528	
Probability of Queue-free State, $p_{0,j}$		0.977	0.981	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$				
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		645	768	
Potential Capacity, $c_{p,x}$		412	342	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Movement Capacity, $c_{m,x}$		412	342	
Probability of Queue-free State, $p_{0,j}$		0.987	0.984	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$		3028	2864	
Potential Capacity, $c_{p,x}$		13	16	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Capacity Adjustment Factor, $f_x$		0.959	0.959	
Movement Capacity, $c_{m,x}$		12	15	
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$		2095	2221	
Potential Capacity, $c_{p,x}$		31	25	
Pedestrian Impedance Factor, $p_{p,x}$		1.000	1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES										
Approach	WestBound				EastBound					
Movement	7	8	9	10	11	12				
Lane Configuration		LTR			LTR					
Shared Flow Rate, $v_y$		59			78					
Movement Capacity, $c_{m,x}$		12	412		15	342				
Shared Capacity, $c_{SH}$										

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		9		10	26	27	5	52	21	5
Movement Capacity		376		528		12	412		15	342
Lane Configuration		L		L		LTR			LTR	
Shared Capacity										
Control Delay		14.8		11.9						

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach	NB		SB		WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12
Lane Configuration		L		L		LTR			LTR	
Flow Rate		9		10		59			78	
Lane Capacity		376		528						
$v/c$		0.02		0.02						
95% Queue Length		0.1		0.1						
Control Delay		14.8		11.9						



LOS  
Approach Delay  
Approach LOS  
Intersection Delay

B  
0.1

B  
0.1

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/20/2019 10:10:12 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_AM\_R-in-out\_Old State-Wortman-x. xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5-Rin-out AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Old State Rd / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Wortman Road  
 North/South Street Name: Old State Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound						SouthBound		
Approach	1U	2	3	4U	4	5	6		
Movement	U	L	T	R	U	L	T	R	
Volume		16	110				67	9	
Peak Hour Factor, PHF					0.92				
Hourly Flow Rtae, HFR		17	120				73	10	
Percent Heavy Vehicles		0							
Number of Lanes	0	0	1	0	0	0	1	0	
Lane Configuration		LT						TR	
Median Type					Undivided				
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street:	WestBound			EastBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume				23		31
Peak Hour Factor, PHF				0.92		
Hourly Flow Rtae, HFR				25		34
Percent Heavy Vehicles				0		3
Number of Lanes	0	0	0	0	1	0
Lane Configuration					LR	
RT channelized?						
Flared Approach   Storage				No		
Percent Grade					0	

Pedestrian Volumes and Adjustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)	0	0		0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Approach	Delay, Queue Length, and Level of Service											
	NB	WestBound			EastBound							
Movement	1U	4U	7	8	9	10	11	12				
Lane Configuration	LT						LR					
Flow Rate	17						59					
Lane Capacity	1527						868					
v/c	0.01						0.07					
95% Queue Length	0.0						0.2					
Control Delay	7.4						9.5					
LOS	A						A					
Approach Delay	1.0						9.5					
Approach LOS							A					
Intersction Delay	2.5											

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound						SouthBound		
Approach	1U	2	3	4U	4	5	6		
Priorty	U	L	T	R	U	L	T	R	
Movement									

Minor Street:

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement	1U	NorthBound				4U	SouthBound		
	U	1	2	3		U	4	5	6
		L	T	R			L	T	R
Volume, V_x		16	110				67	9	
Flow Rate, v_x		17	120				73	10	

Minor Street:									
Approach Movement		WestBound				EastBound			
		7	8	9			10	11	12
		L	T	R			L	T	R
Volume, V_x							23	31	
Flow Rate, v_x							25	34	

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement	1U	NorthBound				4U	SouthBound		
	U	1	2	3		U	4	5	6
		L	T	R			L	T	R
Flow Rate, v_x		17	120				73	10	
Conflicting Flow, v_c, x		83							

Minor Street:									
Approach Movement		WestBound				EastBound			
		7	8	9			10	11	12
		L	T	R			L	T	R
Flow Rate, v_x							25	34	
Conflicting Flow, v_c, x							232	78	

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS										
Approach Movement	NB	SB			WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_c, base		4.1						7.1		6.2
Single Stage										
Stage I										
Stage II										
t_c, HV		1.0						1.0		1.0
P_HV		0.00						0.00		0.03
t_c, G								0.2		0.1
G								0		0
t_3, LT		0.0						0.7		0.0
t_c										
Single Stage		4.10						6.40		6.23
Stage I										
Stage II										

FOLLOW-UP HEADWAYS										
Approach Movement	NB	SB			WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_f, base		2.2						3.5		3.3
t_f, HV		0.9						0.9		0.9
P_HV		0.00						0.00		0.03
t_f		2.20						3.50		3.33

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach Movement	NB	SB			WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
v_c, x		83						232		78
t_c, x		4.10						6.40		6.23
t_f, x		2.20						3.50		3.33
c_p, x		1527						761		980

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0		0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		83		
Potential Capacity, $c_{p,x}$		1527		
Pedestrian Impedance Factor, $p_{p,x}$		1.000		
Movement Capacity, $c_{m,x}$		1527		
Probability of Queue-free State, $p_{0,j}$		0.989		
Major L-Shared Probability Queue-free State, $p^*_{0,j}$		0.988		
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$			78	
Potential Capacity, $c_{p,x}$			980	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Movement Capacity, $c_{m,x}$			980	
Probability of Queue-free State, $p_{0,j}$			0.966	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$			232	
Potential Capacity, $c_{p,x}$			761	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$			0.988	
Movement Capacity, $c_{m,x}$			751	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES						
Approach	7	WestBound	9	10	EastBound	12
Movement		8			11	
Lane Configuration					LR	
Shared Flow Rate, $v_y$					59	
Movement Capacity, $c_{m,x}$				751		980
Shared Capacity, $c_{SH}$					868	

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS										
Approach	NB			SB			WestBound		EastBound	
Movement	1U	1	4U	4	7	8	9	10	11	12
Flow Rate		17						25		34
Movement Capacity		1527						751		980
Lane Configuration		LT							LR	
Shared Capacity									868	
Control Delay		7.4							9.5	

CONTROL DELAY TO RANK 1 MOVEMENTS			
Approach	NB		SB
Movement		2	5
Number of Major Street Through Lanes, $N$		1	1
Proportion of Rank 1 vehicles not blocked, $p^*_{0,j}$		0.988	
Delay to Major Left-turning Vehicles, $d_{MLT}$		7.4	
Major Street Through Vehicles in Shared Lane, $v_{i1}$		120	
Major Street Turning Vehicles in Shared Lane, $v_{i2}$		17	
Saturation Flow Rate for Major Street Through, $s_{i1}$		1800	1800

Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

0.1

1500

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement Lane Configuration	NB 1U	1 LT	SB 4U	4	7	WestBound 8	9	10	EastBound 11	12
Flow Rate		17							59	
Lane Capacity		1527							868	
v/c		0.01							0.07	
95% Queue Length		0.0							0.2	
Control Delay		7.4							9.5	
LOS		A							A	
Approach Delay		1.0							9.5	
Approach LOS									A	
Intersction Delay		2.5								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/20/2019 10:13:03 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis

File Name: 2045\_PM\_5-Rin-out\_OSR-Wortman-x. xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5-Rin-out PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U. S. Customary  
 Intersection Name: Old State Rd / US 41  
 Major Street Direction: North-South  
 East/West Street Name: Wortman Road  
 North/South Street Name: Old State Road  
 Analysis Time Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street:	NorthBound						SouthBound		
Approach	1U	2	3	4U	4	5	6		
Movement	U	L	T	R	U	L	T	R	
Volume		28	94				146	35	
Peak Hour Factor, PHF					0.92				
Hourly Flow Rate, HFR		30	102				159	38	
Percent Heavy Vehicles		0							
Number of Lanes	0	0	1	0	0	0	1	0	
Lane Configuration		LT						TR	
Median Type					Undivided				
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?					Not Present				

Minor Street:	WestBound			EastBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume				19		23
Peak Hour Factor, PHF				0.92		
Hourly Flow Rate, HFR				21		25
Percent Heavy Vehicles				0		3
Number of Lanes	0	0	0	0	1	0
Lane Configuration					LR	
RT channelized?						
Flared Approach   Storage				No		
Percent Grade					0	

Pedestrian Volumes and Adjustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)	0	0		0
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Delay, Queue Length, and Level of Service

Approach	NB	SB	WestBound	EastBound
Movement	1U	4U	7	8
Lane Configuration	LT	4	9	10
			11	12
			LR	
Flow Rate	30			46
Lane Capacity	1388			748
v/c	0.02			0.06
95% Queue Length	0.1			0.2
Control Delay	7.7			10.1
LOS	A			B
Approach Delay	1.9			10.1
Approach LOS				B
Intersection Delay	1.9			

Step 1: MOVEMENT PRIORITIES

Major Street:	NorthBound						SouthBound		
Approach	1U	2	3	4U	4	5	6		
Priority	U	L	T	R	U	L	T	R	

Minor Street:

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:								
Approach Movement	1U	NorthBound			4U	SouthBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Volume, V_x		28	94			146	35	
Flow Rate, v_x		30	102			159	38	

Minor Street:								
Approach Movement		WestBound				EastBound		
		7	8	9		10	11	12
		L	T	R		L	T	R
Volume, V_x						19	23	
Flow Rate, v_x						21	25	

Step 3: CONFLICTING FLOW RATES

Major Street:								
Approach Movement	1U	NorthBound			4U	SouthBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Flow Rate, v_x		30	102			159	38	
Conflicting Flow, v_c, x		197						

Minor Street:								
Approach Movement		WestBound				EastBound		
		7	8	9		10	11	12
		L	T	R		L	T	R
Flow Rate, v_x						21	25	
Conflicting Flow, v_c, x						341	178	

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS												
Approach Movement	NB	1	4U	4	7	WestBound	8	9	10	EastBound	11	12
	1U	L	U	L	L	T	R	L	L	T	R	R
	U											
t_c, base												
Single Stage		4.1							7.1			6.2
Stage I												
Stage II												
t_c, HV		1.0							1.0			1.0
P_HV		0.00							0.00			0.03
t_c, G									0.2			0.1
G									0			0
t_3, LT		0.0							0.7			0.0
t_c												
Single Stage		4.10							6.40			6.23
Stage I												
Stage II												

FOLLOW-UP HEADWAYS												
Approach Movement	NB	1	4U	4	7	WestBound	8	9	10	EastBound	11	12
	1U	L	U	L	L	T	R	L	L	T	R	R
	U											
t_f, base		2.2							3.5			3.3
t_f, HV		0.9							0.9			0.9
P_HV		0.00							0.00			0.03
t_f		2.20							3.50			3.33

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT												
Approach Movement	NB	1	4U	4	7	WestBound	8	9	10	EastBound	11	12
	1U	L	U	L	L	T	R	L	L	T	R	R
	U											
v_c, x		197							341			178
t_c, x		4.10							6.40			6.23
t_f, x		2.20							3.50			3.33
c_p, x		1388							659			863

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0		0
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$		197		
Potential Capacity, $c_{p,x}$		1388		
Pedestrian Impedance Factor, $p_{p,x}$		1.000		
Movement Capacity, $c_{m,x}$		1388		
Probability of Queue-free State, $p_{0,j}$		0.978		
Major L-Shared Probability Queue-free State, $p^*_{0,j}$		0.977		
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$			178	
Potential Capacity, $c_{p,x}$			863	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Movement Capacity, $c_{m,x}$			863	
Probability of Queue-free State, $p_{0,j}$			0.971	
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$			341	
Potential Capacity, $c_{p,x}$			659	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$			0.977	
Movement Capacity, $c_{m,x}$			644	

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES											
Approach	WestBound				EastBound						
Movement	7	8	9	10	11	12					
Lane Configuration					LR						
Shared Flow Rate, $v_y$					46						
Movement Capacity, $c_{m,x}$					644					863	
Shared Capacity, $c_{SH}$					748						

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS												
Approach	NB			SB			WestBound			EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12		
Flow Rate		30						21			25	
Movement Capacity		1388						644			863	
Lane Configuration		LT							LR			
Shared Capacity									748			
Control Delay		7.7							10.1			

CONTROL DELAY TO RANK 1 MOVEMENTS												
Approach	NB			SB			WestBound			EastBound		
Movement	2	5										
Number of Major Street Through Lanes, $N$						1			1			
Proportion of Rank 1 vehicles not blocked, $p^*_{0,j}$						0.977						
Delay to Major Left-turning Vehicles, $d_{MLT}$						7.7						
Major Street Through Vehicles in Shared Lane, $v_{i1}$						102						
Major Street Turning Vehicles in Shared Lane, $v_{i2}$						30						
Saturation Flow Rate for Major Street Through, $s_{i1}$						1800			1800			



Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

0.2

1500

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement Lane Configuration	NB 1U	1 LT	SB 4U	4	7	WestBound 8	9	10	EastBound 11	12
Flow Rate		30							46	
Lane Capacity		1388							748	
v/c		0.02							0.06	
95% Queue Length		0.1							0.2	
Control Delay		7.7							10.1	
LOS		A							B	
Approach Delay		1.9							10.1	
Approach LOS									B	
Intersction Delay		1.9								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/20/2019 10:16:32 AM

HCS7 Two-Way Stop-Control Text Report

File Name: TWO-WAY STOP CONTROL (TWSC) Analysis  
 2045\_AM\_5-Rin-out\_Hilldale-OSR-x.xtw  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5\_R-in-out AM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hilldale  
 Units: U.S. Customary  
 Intersection Name: Hilldale Rd / Old State  
 Major Street Direction: North-South  
 East/West Street Name: Hilldale Road  
 North/South Street Name: Old State Road  
 Analysis Time Period (hrs): 0.25

Vehi cl e Vol umes and Adj ustments

Major Street:	NorthBound						SouthBound		
Approach Movement	1U	1	2	3	4U	4	5	6	
	U	L	T	R	U	L	T	R	
Volume			123	29		22	76		
Peak Hour Factor, PHF					0.92				
Hourly Flow Rtae, HFR			134	32		24	83		
Percent Heavy Vehicles						3			
Number of Lanes	0	0	1	0	0	0	1	0	
Lane Configuration				TR		LT			
Median Type	Undivided								
Median Storage									
RT channelized?									
Left-Turn Lane Storage									
Upstream Signal?	Not Present								

Minor Street:	WestBound			EastBound		
Approach Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume			39			8
Peak Hour Factor, PHF						0.92
Hourly Flow Rtae, HFR			42			9
Percent Heavy Vehicles			3			3
Number of Lanes			0		1	0
Lane Configuration					LR	
RT channelized?						
Flared Approach   Storage	No					
Percent Grade			0			

Pedestri an Vol umes and Adj ustments

Approach Movement	NB	SB	WB	EB
	13	14	15	16
Flow (ped/hr)	0	0	0	
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Del ay, Queue Length, and Level of Service

Approach Movement	1U	NB	4U	SB	7	WestBound	8	9	10	EastBound	11	12
Lane Configuration							LR					
Flow Rate							24					51
Lane Capacity							1407					723
v/c							0.02					0.07
95% Queue Length							0.1					0.2
Control Delay							7.6					10.4
LOS							A					B
Approach Delay							1.8					10.4
Approach LOS												B
Intersction Delay			2.2									

Step 1: MOVEMENT PRI OR TI ES

Major Street:	NorthBound						SouthBound		
Approach Priority Movement	1U	1	2	3	4U	4	5	6	
	U	L	T	R	U	L	T	R	
Minor Street:									

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:									
Approach Movement		NorthBound				SouthBound			
	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Volume, V_x			123	29			22	76	
Flow Rate, v_x			134	32			24	83	

Minor Street:									
Approach Movement		WestBound				EastBound			
		7	8	9			10	11	12
		L	T	R			L	T	R
Volume, V_x		39		8					
Flow Rate, v_x		42		9					

Step 3: CONFLICTING FLOW RATES

Major Street:									
Approach Movement		NorthBound				SouthBound			
	1U	1	2	3		4U	4	5	6
	U	L	T	R		U	L	T	R
Flow Rate, v_x			134	32			24	83	
Conflicting Flow, v_c, x							165		

Minor Street:									
Approach Movement		WestBound				EastBound			
		7	8	9			10	11	12
		L	T	R			L	T	R
Flow Rate, v_x		42		9					
Conflicting Flow, v_c, x		280		149					

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS											
Approach Movement		NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12	
	U	L	U	L	L	T	R	L	T	R	
t_c, base											
Single Stage				4.1	7.1			6.2			
Stage I											
Stage II											
t_c, HV				1.0	1.0		1.0				
P_HV				0.03	0.03		0.03				
t_c, G					0.2		0.1				
G					0		0				
t_3, LT				0.0	0.7		0.0				
t_c											
Single Stage				4.13	6.43		6.23				
Stage I											
Stage II											

FOLLOW-UP HEADWAYS											
Approach Movement		NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12	
	U	L	U	L	L	T	R	L	T	R	
t_f, base				2.2	3.5		3.3				
t_f, HV				0.9	0.9		0.9				
P_HV				0.03	0.03		0.03				
t_f				2.23	3.53		3.33				

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT											
Approach Movement		NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12	
	U	L	U	L	L	T	R	L	T	R	
v_c, x				165	280		149				
t_c, x				4.13	6.43		6.23				
t_f, x				2.23	3.53		3.33				
c_p, x				1407	708		894				

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$ Lane Width, $w$ Walking Speed, $S_p$ Pedestrian Blockage Factor, $f_{pb}$	0	0	0	
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$ Major L-Shared Probability Queue-free State, $p^*_{0,j}$			165 1407 1.000 1407 0.983 0.982	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$		149 894 1.000 894 0.990		
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Shared L/U Capacity, $c_{SH}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$ Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$ Potential Capacity, $c_{p,x}$ Pedestrian Impedance Factor, $p_{p,x}$ Major L, Minor T Adjusted Impedance Factor, $p''$ Major L, Minor T Impedance Factor, $p'$ Capacity Adjustment Factor, $f_x$ Movement Capacity, $c_{m,x}$		280 708 1.000   0.982 695		

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES											
Approach	WestBound				EastBound						
Movement	7	8	9	10	11	12					
Lane Configuration		LR									
Shared Flow Rate, $v_y$		51									
Movement Capacity, $c_{m,x}$	695		894								
Shared Capacity, $c_{SH}$		723									

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS												
Approach	NB				SB							
	1U	1	4U	4	7	8	9	10	11	12		
Movement						LR						
Flow Rate							9					
Movement Capacity						695	894					
Lane Configuration						LR						
Shared Capacity						723						
Control Delay				7.6			10.4					

CONTROL DELAY TO RANK 1 MOVEMENTS												
Approach					NB				SB			
Movement					2				5			
Number of Major Street Through Lanes, $N$					1				1			
Proportion of Rank 1 vehicles not blocked, $p^*_{0,j}$									0.982			
Delay to Major Left-turning Vehicles, $d_{MLT}$									7.6			
Major Street Through Vehicles in Shared Lane, $v_{i1}$									83			
Major Street Turning Vehicles in Shared Lane, $v_{i2}$									24			
Saturation Flow Rate for Major Street Through, $s_{i1}$					1800				1800			

Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

1500

0.1

Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement	1U	NB 1	4U	SB 4	7	WestBound 8	9	10	EastBound 11	12
Lane Configuration				LT		LR				
Flow Rate				24		51				
Lane Capacity				1407		723				
v/c				0.02		0.07				
95% Queue Length				0.1		0.2				
Control Delay				7.6		10.4				
LOS				A		B				
Approach Delay				1.8		10.4				
Approach LOS						B				
Intersection Delay		2.2								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/20/2019 9:33:41 AM

HCS7 Two-Way Stop-Control Text Report

TWO-WAY STOP CONTROL (TWSC) Analysis  
 File Name: 2045\_PM\_5-Rin-out-Hillisdale-OSR-x.txt  
 Analyst: MJK  
 Agency: INDOT  
 Date Performed: 5/29/2019  
 Time Analyzed: 5\_R-in-out PM Peak  
 Jurisdiction: Vincennes  
 Analysis Year: 2045  
 Project Description: US 41 / Hillisdale  
 Units: U.S. Customary  
 Intersection Name: Hillisdale Rd / Old State  
 Major Street Direction: North-South  
 East/West Street Name: Hillisdale Road  
 North/South Street Name: Old State Road  
 Analysis Time Period (hrs): 0.25

Vehi cl e Vol umes and Adj ustments

Major Street:	NorthBound				SouthBound			
Approach	1U	2	3	4U	4	5	6	
Movement	U	L	T	R	U	L	T	R
Volume		108	34			11	156	
Peak Hour Factor, PHF				0.92				
Hourly Flow Rtae, HFR		117	37			12	170	
Percent Heavy Vehicles						3		
Number of Lanes	0	0	1	0	0	0	1	0
Lane Configuration				TR		LT		
Median Type					Undivided			
Median Storage								
RT channelized?								
Left-Turn Lane Storage								
Upstream Signal?					Not Present			

Minor Street:	WestBound			EastBound		
Approach	7	8	9	10	11	12
Movement	L	T	R	L	T	R
Volume	20		14			
Peak Hour Factor, PHF				0.92		
Hourly Flow Rtae, HFR	22		15			
Percent Heavy Vehicles	3		3			
Number of Lanes	0	1	0	0	0	0
Lane Configuration			LR			
RT channelized?						
Flared Approach   Storage	No					
Percent Grade		0				

Pedestri an Vol umes and Adj ustments

Approach	NB	SB	WB	EB
Movement	13	14	15	16
Flow (ped/hr)	0	0	0	
Lane Width (ft)				
Walking Speed (ft/sec)				
Pedestrian Blockage Factor, f_pb				

Del ay, Queue Length, and Level of Service

Approach	NB	SB	WestBound	EastBound
Movement	1U	4U	7	8
Lane Configuration		LT	LR	
Flow Rate		12	37	
Lane Capacity		1420	742	
v/c		0.01	0.05	
95% Queue Length		0.0	0.2	
Control Delay		7.6	10.1	
LOS		A	B	
Approach Delay		0.6	10.1	
Approach LOS			B	
Intersction Delay	1.3			

Step 1: MOVEMENT PRI ORITI ES

Major Street:	NorthBound				SouthBound			
Approach	1U	2	3	4U	4	5	6	
Pri ority	U	L	T	R	U	L	T	R
Movement								

Minor Street:

Approach Priority Movement	WestBound				EastBound		
	7	8	9		10	11	12
	L	T	R		L	T	R

Step 2: MOVEMENT DEMAND VOLUMES AND FLOW RATES

Major Street:								
Approach Movement	1U	NorthBound			4U	SouthBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Volume, V_x		108				11		156
Flow Rate, v_x		117				12		170

Minor Street:								
Approach Movement	WestBound				EastBound			
		7	8	9		10	11	12
		L	T	R		L	T	R
Volume, V_x		20				14		
Flow Rate, v_x		22				15		

Step 3: CONFLICTING FLOW RATES

Major Street:								
Approach Movement	1U	NorthBound			4U	SouthBound		
	U	1	2	3		4	5	6
		L	T	R		L	T	R
Flow Rate, v_x		117				12		170
Conflicting Flow, v_c, x		37				154		

Minor Street:								
Approach Movement	WestBound				EastBound			
		7	8	9		10	11	12
		L	T	R		L	T	R
Flow Rate, v_x		22				15		
Conflicting Flow, v_c, x		329				136		

Step 4: CRITICAL HEADWAYS and FOLLOW-UP HEADWAYS

CRITICAL HEADWAYS										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_c, base										
Single Stage				4.1	7.1		6.2			
Stage I										
Stage II										
t_c, HV				1.0	1.0		1.0			
P_HV				0.03	0.03		0.03			
t_c, G					0.2		0.1			
G					0		0			
t_3, LT				0.0	0.7		0.0			
t_c										
Single Stage				4.13	6.43		6.23			
Stage I										
Stage II										

FOLLOW-UP HEADWAYS										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
t_f, base				2.2	3.5		3.3			
t_f, HV				0.9	0.9		0.9			
P_HV				0.03	0.03		0.03			
t_f				2.23	3.53		3.33			

Step 5: POTENTIAL CAPACITIES

NO UPSTREAM SIGNAL EFFECTS PRESENT										
Approach Movement	NB		SB		WestBound			EastBound		
	1U	1	4U	4	7	8	9	10	11	12
	U	L	U	L	L	T	R	L	T	R
v_c, x				154	329		136			
t_c, x				4.13	6.43		6.23			
t_f, x				2.23	3.53		3.33			
c_p, x				1420	663		910			

Steps 6 - 9: MOVEMENT CAPACITIES

Pedestrian Impedance				
Approach	NR	SR	WR	EB

Movement	13	14	15	16
Pedestrian Flow Rate $v_x$	0	0	0	
Lane Width, $w$				
Walking Speed, $S_p$				
Pedestrian Blockage Factor, $f_{pb}$				
Major-Street Left-Turn Movements		1	4	
Conflicting Flow, $v_{c,x}$			154	
Potential Capacity, $c_{p,x}$			1420	
Pedestrian Impedance Factor, $p_{p,x}$			1.000	
Movement Capacity, $c_{m,x}$			1420	
Probability of Queue-free State, $p_{0,j}$			0.992	
Major L-Shared Probability Queue-free State, $p^*_{0,j}$			0.991	
Minor-Street Right-Turn Movements		9	12	
Conflicting Flow, $v_{c,x}$		136		
Potential Capacity, $c_{p,x}$		910		
Pedestrian Impedance Factor, $p_{p,x}$		1.000		
Movement Capacity, $c_{m,x}$		910		
Probability of Queue-free State, $p_{0,j}$		0.983		
Major-Street U-Turn Movements		1U	4U	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Shared L/U Capacity, $c_{SH}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Through Movements		8	11	
Conflicting Flow, $v_{c,x}$				
Potential Capacity, $c_{p,x}$				
Pedestrian Impedance Factor, $p_{p,x}$				
Capacity Adjustment Factor, $f_x$				
Movement Capacity, $c_{m,x}$				
Probability of Queue-free State, $p_{0,j}$				
Minor-Street Left-Turn Movements		7	10	
Conflicting Flow, $v_{c,x}$		329		
Potential Capacity, $c_{p,x}$		663		
Pedestrian Impedance Factor, $p_{p,x}$		1.000		
Major L, Minor T Adjusted Impedance Factor, $p''$				
Major L, Minor T Impedance Factor, $p'$				
Capacity Adjustment Factor, $f_x$		0.991		
Movement Capacity, $c_{m,x}$		657		

Step 10: FINAL CAPACITY ADJUSTMENTS

SHARED-LANE CAPACITY OF MINOR STREET APPROACHES											
Approach	WestBound				EastBound						
Movement	7	8	9	10	11	12					
Lane Configuration		LR									
Shared Flow Rate, $v_y$		37									
Movement Capacity, $c_{m,x}$	657		910								
Shared Capacity, $c_{SH}$		742									

Step 11: CONTROL DELAY

CONTROL DELAY TO RANK 2 THROUGH 4 MOVEMENTS											
Approach	NB		SB		WestBound				EastBound		
Movement	1U	1	4U	4	7	8	9	10	11	12	
Flow Rate							15				
Movement Capacity							910				
Lane Configuration						LR					
Shared Capacity						742					
Control Delay				7.6		10.1					

CONTROL DELAY TO RANK 1 MOVEMENTS												
Approach					NB				SB			
Movement					2				5			
Number of Major Street Through Lanes, $N$					1				1			
Proportion of Rank 1 vehicles not blocked, $p^*_{0,j}$									0.991			
Delay to Major Left-turning Vehicles, $d_{MLT}$									7.6			
Major Street Through Vehicles in Shared Lane, $v_{i1}$									170			
Major Street Turning Vehicles in Shared Lane, $v_{i2}$									12			
Saturation Flow Rate for Major Street Through, $s_{i1}$					1800				1800			



Saturation Flow Rate for Major Street Right-Turn, s\_i2  
 Delay to Rank 1 Vehicles, d\_Rank1

1500

0.1

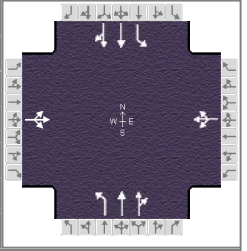
Steps 12 - 13: APPROACH/INTERSECTION CONTROL DELAY and 95% QUEUE LENGTHS

Approach Movement Lane Configuration	NB 1U	1	4U SB	4 LT	7	WestBound 8 LR	9	10	EastBound 11	12
Flow Rate				12		37				
Lane Capacity				1420		742				
v/c				0.01		0.05				
95% Queue Length				0.0		0.2				
Control Delay				7.6		10.1				
LOS				A		B				
Approach Delay				0.6		10.1				
Approach LOS						B				
Intersection Delay		1.3								

This TWSC text report was created in HCS™ TWSC Version 7.8 on 8/20/2019 9:38:00 AM

# HCS7 Signalized Intersection Results Summary

General Information				Intersection Information			
Agency		Duration, h	0.25				
Analyst		Analysis Date	7/15/2019		Area Type	Other	
Jurisdiction		Time Period				PHF	1.00
Urban Street	US 41	Analysis Year	2019		Analysis Period	1 > 7:00	
Intersection	Hillsdale Road	File Name	2020 No Build_SIGNAL_AM Peakx.xus				
Project Description	2020 AM Peak						



Demand Information	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Demand ( $v$ ), veh/h	0	22	14	96	8	41	11	631	25	33	888	0

Signal Information														
Cycle, s	82.5	Reference Phase	2											
Offset, s	0	Reference Point	End											
Uncoordinated	Yes	Simult. Gap E/W	On	Green	40.0	11.2	19.3	0.0	0.0	0.0				
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	4.0	4.0	4.0	0.0	0.0	0.0				
				Red	0.0	0.0	0.0	0.0	0.0	0.0				

Timer Results	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase		4		8		2		6
Case Number		12.0		12.0		6.0		6.0
Phase Duration, s		15.2		23.3		44.0		44.0
Change Period, ( $Y+R_c$ ), s		4.0		4.0		4.0		4.0
Max Allow Headway ( $MAH$ ), s		3.2		3.2		3.1		3.1
Queue Clearance Time ( $g_s$ ), s		3.5		7.7		16.0		15.0
Green Extension Time ( $g_e$ ), s		0.0		0.2		4.0		4.0
Phase Call Probability		0.56		0.96		1.00		1.00
Max Out Probability		0.00		0.00		0.00		0.00

Movement Group Results	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6	16
Adjusted Flow Rate ( $v$ ), veh/h	0			145			11	330	326	33	888	0
Adjusted Saturation Flow Rate ( $s$ ), veh/h/ln	0			1753			636	1900	1874	790	1900	1885
Queue Service Time ( $g_s$ ), s	0.0			5.7			1.0	8.9	9.0	2.2	13.0	0.0
Cycle Queue Clearance Time ( $g_c$ ), s	0.0			5.7			14.0	8.9	9.0	11.2	13.0	0.0
Green Ratio ( $g/C$ )				0.23			0.48	0.48	0.48	0.48	0.48	0.48
Capacity ( $c$ ), veh/h				410			295	921	908	384	1842	
Volume-to-Capacity Ratio ( $X$ )	0.000			0.354			0.037	0.358	0.359	0.086	0.482	0.000
Back of Queue ( $Q$ ), ft/ln ( 50 th percentile)	0			57.6			3.5	87.6	86.6	9.7	126.6	0
Back of Queue ( $Q$ ), veh/ln ( 50 th percentile)	0.0			2.3			0.1	3.5	3.5	0.4	5.1	0.0
Queue Storage Ratio ( $RQ$ ) ( 50 th percentile)	0.00			0.00			0.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay ( $d_1$ ), s/veh				26.4			19.0	13.3	13.3	16.8	14.3	
Incremental Delay ( $d_2$ ), s/veh	0.0			0.2			0.0	0.1	0.1	0.0	0.1	0.0
Initial Queue Delay ( $d_3$ ), s/veh	0.0			0.0			0.0	0.0	0.0	0.0	0.0	0.0
Control Delay ( $d$ ), s/veh				26.6			19.0	13.3	13.4	16.8	14.4	
Level of Service (LOS)				C			B	B	B	B	B	
Approach Delay, s/veh / LOS	31.5		C	26.6		C	13.4		B	14.5		B
Intersection Delay, s/veh / LOS	15.4						B					

Multimodal Results	EB		WB		NB		SB	
Pedestrian LOS Score / LOS	2.28	B	2.31	B	1.67	B	1.67	B
Bicycle LOS Score / LOS	0.55	A	0.73	A	1.04	A	1.25	A

## HCS7 Signalized Intersection Results Summary

General Information				Intersection Information			
Agency		Duration, h	0.25				
Analyst		Analysis Date	7/15/2019				
Jurisdiction		Time Period					
Urban Street	US 41	Analysis Year	2019				
Intersection	Hillsdale Road	File Name	2020 No Build_SIGNAL_PM Peakx.xus				
Project Description	2020 PM Peak						

Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand ( v ), veh/h	3	21	6	41	21	44	20	958	42	52	1025	1

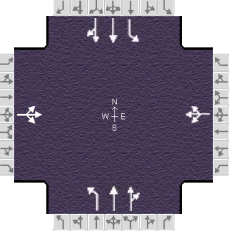
Signal Information												
Cycle, s	78.0	Reference Phase	2									
Offset, s	0	Reference Point	End									
Uncoordinated	Yes	Simult. Gap E/W	On									
Force Mode	Fixed	Simult. Gap N/S	On									
		Green	50.0	20.0	0.0	0.0	0.0	0.0				
		Yellow	4.0	4.0	0.0	0.0	0.0	0.0				
		Red	0.0	0.0	0.0	0.0	0.0	0.0				

Timer Results	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase		4		8		2		6
Case Number		8.0		8.0		6.0		6.0
Phase Duration, s		24.0		24.0		54.0		54.0
Change Period, ( Y+R <sub>c</sub> ), s		4.0		4.0		4.0		4.0
Max Allow Headway ( MAH ), s		3.2		3.2		3.2		3.2
Queue Clearance Time ( g <sub>s</sub> ), s		3.0		6.2		15.4		18.1
Green Extension Time ( g <sub>e</sub> ), s		0.2		0.2		6.2		6.2
Phase Call Probability		1.00		1.00		1.00		1.00
Max Out Probability		0.00		0.00		0.04		0.05

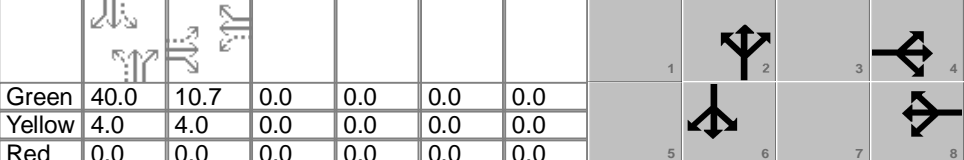
Movement Group Results	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6	16
Adjusted Flow Rate ( v ), veh/h	33			115			22	548	539	57	558	558
Adjusted Saturation Flow Rate ( s ), veh/h/ln	1805			1599			513	1900	1871	527	1900	1899
Queue Service Time ( g <sub>s</sub> ), s	0.0			1.0			1.8	11.3	11.3	4.7	11.6	11.6
Cycle Queue Clearance Time ( g <sub>c</sub> ), s	1.0			4.2			13.4	11.3	11.3	16.1	11.6	11.6
Green Ratio ( g/C )	0.26			0.26			0.64	0.64	0.64	0.64	0.64	0.64
Capacity ( c ), veh/h	514			474			345	1218	1200	354	1218	1218
Volume-to-Capacity Ratio ( X )	0.063			0.243			0.063	0.450	0.450	0.160	0.458	0.458
Back of Queue ( Q ), ft/ln ( 50 th percentile)	10.9			40.3			4.6	90.6	89.2	12.6	92.6	92.6
Back of Queue ( Q ), veh/ln ( 50 th percentile)	0.4			1.6			0.2	3.6	3.6	0.5	3.7	3.7
Queue Storage Ratio ( RQ ) ( 50 th percentile)	0.00			0.00			0.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay ( d <sub>1</sub> ), s/veh	22.0			23.1			10.5	7.1	7.1	11.1	7.1	7.1
Incremental Delay ( d <sub>2</sub> ), s/veh	0.0			0.1			0.0	0.1	0.1	0.1	0.1	0.1
Initial Queue Delay ( d <sub>3</sub> ), s/veh	0.0			0.0			0.0	0.0	0.0	0.0	0.0	0.0
Control Delay ( d ), s/veh	22.0			23.2			10.5	7.2	7.2	11.2	7.2	7.2
Level of Service ( LOS )	C			C			B	A	A	B	A	A
Approach Delay, s/veh / LOS	22.0	C		23.2	C		7.2	A		7.4	A	
Intersection Delay, s/veh / LOS	8.3						A					

Multimodal Results	EB		WB		NB		SB	
Pedestrian LOS Score / LOS	2.28	B	2.28	B	1.64	B	1.64	B
Bicycle LOS Score / LOS	0.54	A	0.68	A	1.40	A	1.45	A

## HCS7 Signalized Intersection Results Summary

General Information				Intersection Information			
Agency		Duration, h	0.25				
Analyst		Analysis Date	7/15/2019				
Jurisdiction		Time Period					
Urban Street	US 41	Analysis Year	2019				
Intersection	Hillsdale Road	File Name	2045 No Build_SIGNAL_AM Peakx.xus				
Project Description	2045 AM Peak						

Demand Information	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Demand ( v ), veh/h	0	28	18	124	10	54	14	821	32	43	1155	0

Signal Information												
Cycle, s	58.7	Reference Phase	2									
Offset, s	0	Reference Point	End									
Uncoordinated	Yes	Simult. Gap E/W	On									
Force Mode	Fixed	Simult. Gap N/S	On									
		Green	40.0	10.7	0.0	0.0	0.0	0.0				
		Yellow	4.0	4.0	0.0	0.0	0.0	0.0				
		Red	0.0	0.0	0.0	0.0	0.0	0.0				

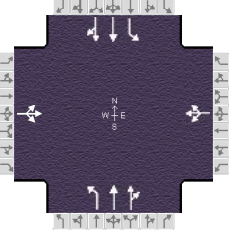
Timer Results	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase		4		8		2		6
Case Number		8.0		8.0		6.0		6.0
Phase Duration, s		14.7		14.7		44.0		44.0
Change Period, ( Y+R <sub>c</sub> ), s		4.0		4.0		4.0		4.0
Max Allow Headway ( MAH ), s		3.2		3.2		3.1		3.1
Queue Clearance Time ( g <sub>s</sub> ), s		3.4		10.5		12.2		11.3
Green Extension Time ( g <sub>e</sub> ), s		0.4		0.3		7.1		7.1
Phase Call Probability		0.98		0.98		1.00		1.00
Max Out Probability		0.00		0.00		0.02		0.02

Movement Group Results	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6	16
Adjusted Flow Rate ( v ), veh/h	0			204			15	467	460	47	1255	0
Adjusted Saturation Flow Rate ( s ), veh/h/ln	0			1375			449	1900	1875	613	1900	1885
Queue Service Time ( g <sub>s</sub> ), s	0.0			7.2			1.0	6.1	6.1	2.1	9.3	0.0
Cycle Queue Clearance Time ( g <sub>c</sub> ), s	0.0			8.5			10.2	6.1	6.1	8.2	9.3	0.0
Green Ratio ( g/C )				0.18			0.68	0.68	0.68	0.68	0.68	0.68
Capacity ( c ), veh/h				353			358	1294	1276	476	2587	
Volume-to-Capacity Ratio ( X )	0.000			0.578			0.043	0.361	0.361	0.098	0.485	0.000
Back of Queue ( Q ), ft/ln ( 50 th percentile)	0			63.1			1.9	33	32.5	4.9	49.7	0
Back of Queue ( Q ), veh/ln ( 50 th percentile)	0.0			2.5			0.1	1.3	1.3	0.2	2.0	0.0
Queue Storage Ratio ( RQ ) ( 50 th percentile)	0.00			0.00			0.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay ( d <sub>1</sub> ), s/veh				23.3			6.9	4.0	4.0	5.7	4.5	
Incremental Delay ( d <sub>2</sub> ), s/veh	0.0			0.6			0.0	0.1	0.1	0.0	0.1	0.0
Initial Queue Delay ( d <sub>3</sub> ), s/veh	0.0			0.0			0.0	0.0	0.0	0.0	0.0	0.0
Control Delay ( d ), s/veh				23.9			6.9	4.0	4.0	5.7	4.5	
Level of Service ( LOS )				C			A	A	A	A	A	
Approach Delay, s/veh / LOS	20.3	C		23.9	C		4.1	A		4.6	A	
Intersection Delay, s/veh / LOS	6.3						A					

Multimodal Results	EB		WB		NB		SB	
Pedestrian LOS Score / LOS	2.28	B	2.28	B	1.62	B	1.62	B
Bicycle LOS Score / LOS	0.57	A	0.82	A	1.27	A	1.56	B

## HCS7 Signalized Intersection Results Summary

General Information				Intersection Information			
Agency		Duration, h	0.25				
Analyst		Analysis Date	7/15/2019		Area Type	Other	
Jurisdiction		Time Period				PHF	0.92
Urban Street	US 41	Analysis Year	2019		Analysis Period	1 > 7:00	
Intersection	Hillsdale Road	File Name	2045 No Build_SIGNAL_PM Peakx.xus				
Project Description	2045 PM Peak						



Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand ( v ), veh/h	4	27	8	53	27	57	26	1229	54	66	1315	1

Signal Information													
Cycle, s	47.2	Reference Phase	2										
Offset, s	0	Reference Point	End										
Uncoordinated	Yes	Simult. Gap E/W	On	Green	30.0	9.2	0.0	0.0	0.0	0.0			
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	4.0	4.0	0.0	0.0	0.0	0.0			
				Red	0.0	0.0	0.0	0.0	0.0	0.0			

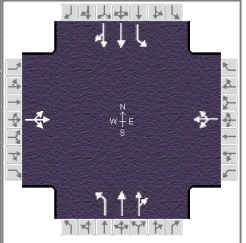
Timer Results	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase		4		8		2		6
Case Number		8.0		8.0		6.0		6.0
Phase Duration, s		13.2		13.2		34.0		34.0
Change Period, ( Y+R <sub>c</sub> ), s		4.0		4.0		4.0		4.0
Max Allow Headway ( MAH ), s		3.2		3.2		3.2		3.2
Queue Clearance Time ( g <sub>s</sub> ), s		2.9		5.6		14.6		18.2
Green Extension Time ( g <sub>e</sub> ), s		0.3		0.3		9.6		9.1
Phase Call Probability		0.92		0.92		1.00		1.00
Max Out Probability		0.00		0.00		0.30		0.37

Movement Group Results	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6	16
Adjusted Flow Rate ( v ), veh/h	42			149			28	702	693	72	715	715
Adjusted Saturation Flow Rate ( s ), veh/h/ln	1806			1617			380	1900	1871	393	1900	1899
Queue Service Time ( g <sub>s</sub> ), s	0.0			1.2			2.2	10.1	10.1	6.1	10.4	10.4
Cycle Queue Clearance Time ( g <sub>c</sub> ), s	0.9			3.6			12.6	10.1	10.1	16.2	10.4	10.4
Green Ratio ( g/C )	0.19			0.19			0.64	0.64	0.64	0.64	0.64	0.64
Capacity ( c ), veh/h	436			421			311	1208	1190	318	1208	1208
Volume-to-Capacity Ratio ( X )	0.097			0.354			0.091	0.581	0.582	0.225	0.592	0.592
Back of Queue ( Q ), ft/ln ( 50 th percentile)	8.2			30.8			3.6	47.7	47.1	10.1	49.1	49.1
Back of Queue ( Q ), veh/ln ( 50 th percentile)	0.3			1.2			0.1	1.9	1.9	0.4	2.0	2.0
Queue Storage Ratio ( RQ ) ( 50 th percentile)	0.00			0.00			0.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay ( d <sub>1</sub> ), s/veh	15.7			16.7			8.7	5.0	5.0	9.7	5.0	5.0
Incremental Delay ( d <sub>2</sub> ), s/veh	0.0			0.2			0.0	0.2	0.2	0.1	0.2	0.2
Initial Queue Delay ( d <sub>3</sub> ), s/veh	0.0			0.0			0.0	0.0	0.0	0.0	0.0	0.0
Control Delay ( d ), s/veh	15.7			16.9			8.7	5.1	5.1	9.8	5.2	5.2
Level of Service ( LOS )	B			B			A	A	A	A	A	A
Approach Delay, s/veh / LOS	15.7	B		16.9	B		5.2	A		5.4	A	
Intersection Delay, s/veh / LOS	6.0						A					

Multimodal Results	EB		WB		NB		SB	
Pedestrian LOS Score / LOS	2.27	B	2.27	B	1.62	B	1.62	B
Bicycle LOS Score / LOS	0.56	A	0.73	A	1.66	B	1.73	B

## HCS7 Signalized Intersection Results Summary

General Information				Intersection Information			
Agency				Duration, h	0.25		
Analyst				Analysis Date	7/15/2019		
Jurisdiction				Area Type	Other		
Urban Street	US 41			PHF	1.00		
Intersection	Hillsdale Road			Analysis Year	2019		
Project Description	2020 AM Peak w/ Radio Rin-out			Analysis Period	1> 7:00		
				File Name	2020 5Rin-out_SIGNAL_AM Peakx.xus		



Demand Information	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Demand ( v ), veh/h	0	22	14	99	8	41	11	631	25	33	888	0

Signal Information													
Cycle, s	82.6	Reference Phase	2										
Offset, s	0	Reference Point	End										
Uncoordinated	Yes	Simult. Gap E/W	On	Green	40.0	11.2	19.3	0.0	0.0	0.0			
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	4.0	4.0	4.0	0.0	0.0	0.0			
				Red	0.0	0.0	0.0	0.0	0.0	0.0			

Timer Results	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase		4		8		2		6
Case Number		12.0		12.0		6.0		6.0
Phase Duration, s		15.2		23.3		44.0		44.0
Change Period, ( Y+R <sub>c</sub> ), s		4.0		4.0		4.0		4.0
Max Allow Headway ( MAH ), s		3.2		3.2		3.1		3.1
Queue Clearance Time ( g <sub>s</sub> ), s		3.5		7.8		16.0		15.0
Green Extension Time ( g <sub>e</sub> ), s		0.0		0.2		4.0		4.0
Phase Call Probability		0.56		0.97		1.00		1.00
Max Out Probability		0.00		0.00		0.00		0.00

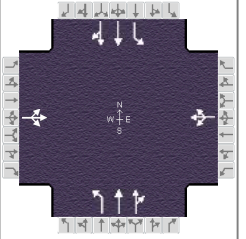
Movement Group Results	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6	16
Adjusted Flow Rate ( v ), veh/h	0			148			11	330	326	33	888	0
Adjusted Saturation Flow Rate ( s ), veh/h/ln	0			1754			636	1900	1874	790	1900	0
Queue Service Time ( g <sub>s</sub> ), s	0.0			5.8			1.0	9.0	9.0	2.2	13.0	0.0
Cycle Queue Clearance Time ( g <sub>c</sub> ), s	0.0			5.8			14.0	9.0	9.0	11.2	13.0	0.0
Green Ratio ( g/C )				0.23			0.48	0.48	0.48	0.48	0.48	
Capacity ( c ), veh/h				411			295	920	908	384	1841	
Volume-to-Capacity Ratio ( X )	0.000			0.360			0.037	0.359	0.359	0.086	0.482	0.000
Back of Queue ( Q ), ft/ln ( 50 th percentile)	0			59			3.5	87.6	86.6	9.7	126.6	0
Back of Queue ( Q ), veh/ln ( 50 th percentile)	0.0			2.4			0.1	3.5	3.5	0.4	5.1	0.0
Queue Storage Ratio ( RQ ) ( 50 th percentile)	0.00			0.00			0.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay ( d <sub>1</sub> ), s/veh				26.5			19.0	13.3	13.3	16.8	14.3	
Incremental Delay ( d <sub>2</sub> ), s/veh	0.0			0.2			0.0	0.1	0.1	0.0	0.1	0.0
Initial Queue Delay ( d <sub>3</sub> ), s/veh	0.0			0.0			0.0	0.0	0.0	0.0	0.0	0.0
Control Delay ( d ), s/veh				26.7			19.0	13.4	13.4	16.8	14.4	
Level of Service ( LOS )				C			B	B	B	B	B	
Approach Delay, s/veh / LOS	31.5	C		26.7	C		13.5	B		14.5	B	
Intersection Delay, s/veh / LOS	15.5						B					

Multimodal Results	EB		WB		NB		SB	
Pedestrian LOS Score / LOS	2.28	B	2.31	B	1.67	B	1.67	B
Bicycle LOS Score / LOS	0.55	A	0.73	A	1.04	A	1.25	A



# HCS7 Signalized Intersection Results Summary

General Information					Intersection Information			
Agency					Duration, h	0.25		
Analyst			Analysis Date	7/15/2019	Area Type	Other		
Jurisdiction			Time Period		PHF	0.92		
Urban Street	US 41		Analysis Year	2019	Analysis Period	1 > 7:00		
Intersection	Hillsdale Road		File Name	2020_5Rin-out_SIGNAL_PM Peakx.xus				
Project Description	2020 PM Peak w/ Radio Rin-out							



Demand Information	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Demand ( v ), veh/h	3	21	6	42	21	44	20	958	42	53	1025	1

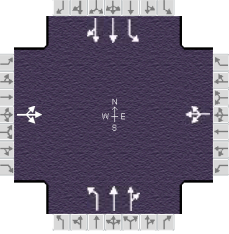
Signal Information												
Cycle, s	78.0	Reference Phase	2									
Offset, s	0	Reference Point	End									
Uncoordinated	Yes	Simult. Gap E/W	On	Green	50.0	20.0	0.0	0.0	0.0	0.0		
Force Mode	Fixed	Simult. Gap N/S	On	Yellow	4.0	4.0	0.0	0.0	0.0	0.0		
				Red	0.0	0.0	0.0	0.0	0.0	0.0		

Timer Results	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase		4		8		2		6
Case Number		8.0		8.0		6.0		6.0
Phase Duration, s		24.0		24.0		54.0		54.0
Change Period, ( Y+R <sub>c</sub> ), s		4.0		4.0		4.0		4.0
Max Allow Headway ( MAH ), s		3.2		3.2		3.2		3.2
Queue Clearance Time ( g <sub>s</sub> ), s		3.0		6.2		15.4		18.2
Green Extension Time ( g <sub>e</sub> ), s		0.2		0.2		6.2		6.2
Phase Call Probability		1.00		1.00		1.00		1.00
Max Out Probability		0.00		0.00		0.04		0.05

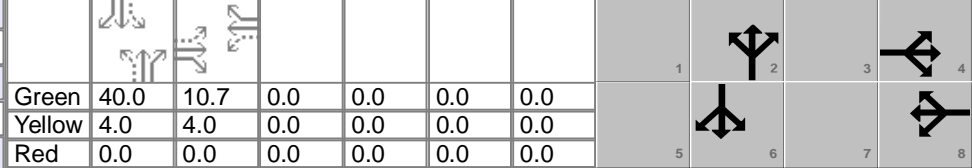
Movement Group Results	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6	16
Adjusted Flow Rate ( v ), veh/h	33			116			22	548	539	58	558	558
Adjusted Saturation Flow Rate ( s ), veh/h/ln	1805			1597			513	1900	1871	527	1900	1899
Queue Service Time ( g <sub>s</sub> ), s	0.0			1.1			1.8	11.3	11.3	4.8	11.6	11.6
Cycle Queue Clearance Time ( g <sub>c</sub> ), s	1.0			4.2			13.4	11.3	11.3	16.2	11.6	11.6
Green Ratio ( g/C )	0.26			0.26			0.64	0.64	0.64	0.64	0.64	0.64
Capacity ( c ), veh/h	514			474			345	1218	1200	354	1218	1218
Volume-to-Capacity Ratio ( X )	0.063			0.245			0.063	0.450	0.450	0.163	0.458	0.458
Back of Queue ( Q ), ft/ln ( 50 th percentile)	10.9			40.9			4.6	90.6	89.2	12.8	92.6	92.6
Back of Queue ( Q ), veh/ln ( 50 th percentile)	0.4			1.6			0.2	3.6	3.6	0.5	3.7	3.7
Queue Storage Ratio ( RQ ) ( 50 th percentile)	0.00			0.00			0.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay ( d <sub>1</sub> ), s/veh	22.0			23.1			10.5	7.1	7.1	11.1	7.1	7.1
Incremental Delay ( d <sub>2</sub> ), s/veh	0.0			0.1			0.0	0.1	0.1	0.1	0.1	0.1
Initial Queue Delay ( d <sub>3</sub> ), s/veh	0.0			0.0			0.0	0.0	0.0	0.0	0.0	0.0
Control Delay ( d ), s/veh	22.0			23.2			10.5	7.2	7.2	11.2	7.2	7.2
Level of Service ( LOS )	C			C			B	A	A	B	A	A
Approach Delay, s/veh / LOS	22.0	C		23.2	C		7.2	A		7.4	A	
Intersection Delay, s/veh / LOS	8.3						A					

Multimodal Results	EB		WB		NB		SB	
Pedestrian LOS Score / LOS	2.28	B	2.28	B	1.64	B	1.64	B
Bicycle LOS Score / LOS	0.54	A	0.68	A	1.40	A	1.46	A

## HCS7 Signalized Intersection Results Summary

General Information				Intersection Information			
Agency		Duration, h	0.25				
Analyst		Analysis Date	7/15/2019				
Jurisdiction		Time Period					
Urban Street	US 41	Analysis Year	2019				
Intersection	Hillsdale Road	File Name	2045_5Rin-out_SIGNAL_AM Peakx.xus				
Project Description	2045 AM Peak w/ Radio Rin-out						

Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand ( v ), veh/h	0	28	18	124	10	52	14	791	32	42	1112	0

Signal Information												
Cycle, s	58.7	Reference Phase	2									
Offset, s	0	Reference Point	End									
Uncoordinated	Yes	Simult. Gap E/W	On									
Force Mode	Fixed	Simult. Gap N/S	On									
		Green	40.0	10.7	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
		Yellow	4.0	4.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
		Red	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Timer Results	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase		4		8		2		6
Case Number		8.0		8.0		6.0		6.0
Phase Duration, s		14.7		14.7		44.0		44.0
Change Period, ( Y+R <sub>c</sub> ), s		4.0		4.0		4.0		4.0
Max Allow Headway ( MAH ), s		3.2		3.2		3.1		3.1
Queue Clearance Time ( g <sub>s</sub> ), s		3.4		10.5		11.6		10.7
Green Extension Time ( g <sub>e</sub> ), s		0.4		0.3		6.6		6.6
Phase Call Probability		0.98		0.98		1.00		1.00
Max Out Probability		0.00		0.00		0.01		0.01

Movement Group Results	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6	16
Adjusted Flow Rate ( v ), veh/h	0			202			15	450	444	46	1209	0
Adjusted Saturation Flow Rate ( s ), veh/h/ln	0			1370			470	1900	1874	632	1900	0
Queue Service Time ( g <sub>s</sub> ), s	0.0			7.1			0.9	5.8	5.8	1.9	8.7	0.0
Cycle Queue Clearance Time ( g <sub>c</sub> ), s	0.0			8.5			9.6	5.8	5.8	7.7	8.7	0.0
Green Ratio ( g/C )				0.18			0.68	0.68	0.68	0.68	0.68	
Capacity ( c ), veh/h				352			373	1295	1277	491	2590	
Volume-to-Capacity Ratio ( X )	0.000			0.575			0.041	0.348	0.348	0.093	0.467	0.000
Back of Queue ( Q ), ft/ln ( 50 th percentile)	0			62.3			1.8	31.2	30.8	4.6	46.2	0
Back of Queue ( Q ), veh/ln ( 50 th percentile)	0.0			2.5			0.1	1.2	1.2	0.2	1.8	0.0
Queue Storage Ratio ( RQ ) ( 50 th percentile)	0.00			0.00			0.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay ( d <sub>1</sub> ), s/veh				23.3			6.6	3.9	3.9	5.5	4.4	
Incremental Delay ( d <sub>2</sub> ), s/veh	0.0			0.6			0.0	0.1	0.1	0.0	0.0	0.0
Initial Queue Delay ( d <sub>3</sub> ), s/veh	0.0			0.0			0.0	0.0	0.0	0.0	0.0	0.0
Control Delay ( d ), s/veh				23.9			6.6	4.0	4.0	5.5	4.4	
Level of Service ( LOS )				C			A	A	A	A	A	
Approach Delay, s/veh / LOS	20.3	C		23.9	C		4.0	A		4.4	A	
Intersection Delay, s/veh / LOS	6.2						A					

Multimodal Results	EB		WB		NB		SB	
Pedestrian LOS Score / LOS	2.28	B	2.28	B	1.62	B	1.62	B
Bicycle LOS Score / LOS	0.57	A	0.82	A	1.24	A	1.52	B



# HCS7 Signalized Intersection Results Summary

General Information				Intersection Information					
Agency		Duration, h	0.25						
Analyst		Analysis Date	7/15/2019					Area Type	Other
Jurisdiction		Time Period						PHF	0.92
Urban Street	US 41	Analysis Year	2019					Analysis Period	1 > 7:00
Intersection	Hillsdale Road	File Name	2045_5Rin-out_SIGNAL_PM Peakx.xus						
Project Description	2045 PM Peak w/ Radio Rin-out								

Demand Information	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Demand ( $v$ ), veh/h	4	27	8	53	27	56	25	1201	53	66	1284	1

Signal Information												
Cycle, s	47.2	Reference Phase	2									
Offset, s	0	Reference Point	End									
Uncoordinated	Yes	Simult. Gap E/W	On									
Force Mode	Fixed	Simult. Gap N/S	On									
Green	30.0	9.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Yellow	4.0	4.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Red	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Timer Results	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase		4		8		2		6
Case Number		8.0		8.0		6.0		6.0
Phase Duration, s		13.2		13.2		34.0		34.0
Change Period, ( $Y+R_c$ ), s		4.0		4.0		4.0		4.0
Max Allow Headway ( $MAH$ ), s		3.2		3.2		3.2		3.2
Queue Clearance Time ( $g_s$ ), s		2.9		5.6		14.0		17.5
Green Extension Time ( $g_e$ ), s		0.3		0.3		9.2		8.8
Phase Call Probability		0.92		0.92		1.00		1.00
Max Out Probability		0.00		0.00		0.27		0.33

Movement Group Results	EB			WB			NB			SB		
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Assigned Movement	7	4	14	3	8	18	5	2	12	1	6	16
Adjusted Flow Rate ( $v$ ), veh/h	42			148			27	686	677	72	698	698
Adjusted Saturation Flow Rate ( $s$ ), veh/h/ln	1806			1617			393	1900	1871	405	1900	1899
Queue Service Time ( $g_s$ ), s	0.0			1.1			2.0	9.7	9.7	5.8	10.0	10.0
Cycle Queue Clearance Time ( $g_c$ ), s	0.9			3.6			12.0	9.7	9.7	15.5	10.0	10.0
Green Ratio ( $g/C$ )	0.19			0.19			0.64	0.64	0.64	0.64	0.64	0.64
Capacity ( $c$ ), veh/h	435			421			319	1208	1190	327	1208	1208
Volume-to-Capacity Ratio ( $X$ )	0.097			0.351			0.085	0.568	0.569	0.220	0.578	0.578
Back of Queue ( $Q$ ), ft/ln ( 50 th percentile)	8.2			30.6			3.3	45.6	45.5	9.8	47	47
Back of Queue ( $Q$ ), veh/ln ( 50 th percentile)	0.3			1.2			0.1	1.8	1.8	0.4	1.9	1.9
Queue Storage Ratio ( $RQ$ ) ( 50 th percentile)	0.00			0.00			0.00	0.00	0.00	0.00	0.00	0.00
Uniform Delay ( $d_1$ ), s/veh	15.7			16.7			8.4	4.9	4.9	9.3	4.9	4.9
Incremental Delay ( $d_2$ ), s/veh	0.0			0.2			0.0	0.2	0.2	0.1	0.2	0.2
Initial Queue Delay ( $d_3$ ), s/veh	0.0			0.0			0.0	0.0	0.0	0.0	0.0	0.0
Control Delay ( $d$ ), s/veh	15.7			16.9			8.4	5.0	5.1	9.5	5.1	5.1
Level of Service (LOS)	B			B			A	A	A	A	A	A
Approach Delay, s/veh / LOS	15.7	B		16.9	B		5.1	A		5.3	A	
Intersection Delay, s/veh / LOS	5.9						A					

Multimodal Results	EB		WB		NB		SB	
Pedestrian LOS Score / LOS	2.27	B	2.27	B	1.62	B	1.62	B
Bicycle LOS Score / LOS	0.56	A	0.73	A	1.63	B	1.70	B

# HCS7 Signalized Intersection Results Summary

General Information				Intersection Information			
Agency		Duration, h	0.25				
Analyst		Analysis Date	8/22/2019				
Jurisdiction		Time Period					
Urban Street	RCUT	Analysis Year	2019				
Intersection	SB 41 / North MUT	File Name	R_North R-Cut_2020 AMxx.xus				
Project Description	2020 AM w/ 5Rin-out						

Demand Information	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Demand ( $v$ ), veh/h		0	0	107		0		0			922	

Signal Information												
Cycle, s	100.0	Reference Phase	2									
Offset, s	0	Reference Point	Begin									
Uncoordinated	Yes	Simult. Gap E/W	Off									
Force Mode	Fixed	Simult. Gap N/S	Off									
Green	30.0	60.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Yellow	4.0	4.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Red	1.0	1.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Timer Results	EBL	EBT	WBL	WBT	NBL	NBT	SBL	SBT
Assigned Phase		6	5	2		8		4
Case Number		7.3	2.0	3.0		8.0		8.0
Phase Duration, s		0.0	35.0	35.0		65.0		65.0
Change Period, ( $Y+R_c$ ), s		5.0	5.0	5.0		5.0		5.0
Max Allow Headway ( $MAH$ ), s		0.0	3.1	0.0		0.0		3.0
Queue Clearance Time ( $g_s$ ), s			6.8					17.3
Green Extension Time ( $g_e$ ), s		0.0	0.2	0.0		0.0		2.6
Phase Call Probability			1.00					1.00
Max Out Probability			0.00					0.00

Movement Group Results	EB			WB			NB			SB		
	L	T	R	L	T	R	L	T	R	L	T	R
Approach Movement												
Assigned Movement		6	16	5		12		8			4	
Adjusted Flow Rate ( $v$ ), veh/h		0	0	116		0		0			1002	
Adjusted Saturation Flow Rate ( $s$ ), veh/h/ln		1900	1610	1810		1610		1900			1809	
Queue Service Time ( $g_s$ ), s		0.0	0.0	4.8		0.0		0.0			15.3	
Cycle Queue Clearance Time ( $g_c$ ), s		0.0	0.0	4.8		0.0		0.0			15.3	
Green Ratio ( $g/C$ )		0.96	0.00	0.90		0.30		0.60			0.60	
Capacity ( $c$ ), veh/h		2	2	543		483		1140			2171	
Volume-to-Capacity Ratio ( $X$ )		0.000	0.000	0.214		0.000		0.000			0.462	
Back of Queue ( $Q$ ), ft/ln ( 50 th percentile)		0	0	54.2		0		0			144.2	
Back of Queue ( $Q$ ), veh/ln ( 50 th percentile)		0.0	0.0	2.2		0.0		0.0			5.8	
Queue Storage Ratio ( $RQ$ ) ( 50 th percentile)		0.00	0.00	0.00		0.00		0.00			0.00	
Uniform Delay ( $d_1$ ), s/veh		0.0	0.0	26.2		0.0		0.0			11.1	
Incremental Delay ( $d_2$ ), s/veh		0.0	0.0	0.9		0.0		0.0			0.7	
Initial Queue Delay ( $d_3$ ), s/veh		0.0	0.0	0.0		0.0		0.0			0.0	
Control Delay ( $d$ ), s/veh		0.0	0.0	27.1		0.0		0.0			11.8	
Level of Service (LOS)				C							B	
Approach Delay, s/veh / LOS	0.0			27.1		C	0.0			11.8		B
Intersection Delay, s/veh / LOS				13.4						B		

Multimodal Results	EB		WB		NB		SB	
Pedestrian LOS Score / LOS	1.96	B	1.93	B	1.88	B	1.66	B
Bicycle LOS Score / LOS	0.49	A		F	0.49	A	1.31	A