SUBGRADE STABILITY CHALLENGES WITH RUBBLIZE AND ROLL PAVEMENT REHABILITATION





























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HOW DID WE GET HERE?





WAY-21-0.00

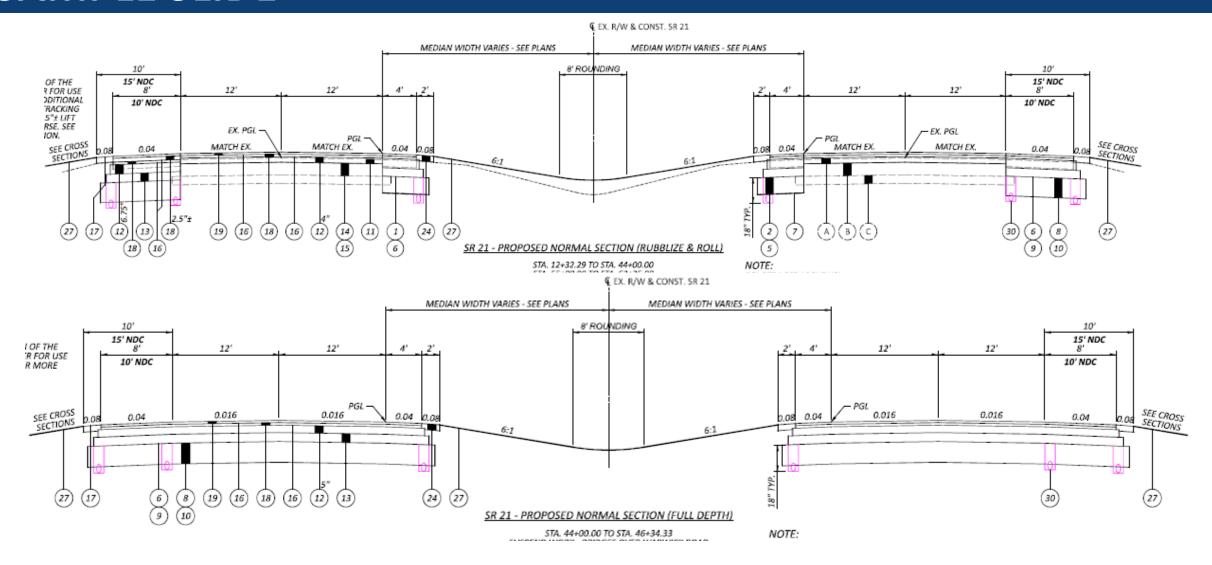
Major arterial route – 6 miles of major rehab

- 4-Lane divided road
- 2 Proposed R-cut intersections, 2 Proposed at grade intersections, 4 Bridge rehabilitations
- \$30M DB contract, but...
- ODOT performs borings, designs pavement (9" Rubblize and Roll,
 7.25" asphalt), intersections, etc.
- Full depth replacement at outside shoulder widenings, intersections, bridge approaches with 14" cement stabilization





SAMPLE SLIDE







GEOTECHNICAL PROFILE

- Rubblize and Roll is not an option when the average N_{60L} value for the subgrade below the existing pavement is less than 12.
- During construction, the Rubblize and Roll is attempted before an area is selected for excavation and replacement. The actual excavation areas will be selected based on the inability to Rubblize and Roll.



GEOTECHNICAL PROFILE

| Chemical Stabilization Options | | | | | | | | | | |
|--------------------------------|----------------------|--------|--|--|--|--|--|--|--|--|
| 320 | Rubblize & Roll | Option | | | | | | | | |
| 206 | Cement Stabilization | Option | | | | | | | | |
| | Lime Stabilization | No | | | | | | | | |
| 206 | Depth | NA | | | | | | | | |

| Excavate and Replace | | | | | | | | | |
|-----------------------|----|--|--|--|--|--|--|--|--|
| Stabilization Options | | | | | | | | | |
| Global Geotextile | | | | | | | | | |
| Average(N60L): | 0" | | | | | | | | |
| Average(HP): | 0" | | | | | | | | |
| Global Geogrid | | | | | | | | | |
| Average(N60L): | 0" | | | | | | | | |
| Average(HP): | 0" | | | | | | | | |

| Design CBR | |
|---------------|--|
|---------------|--|

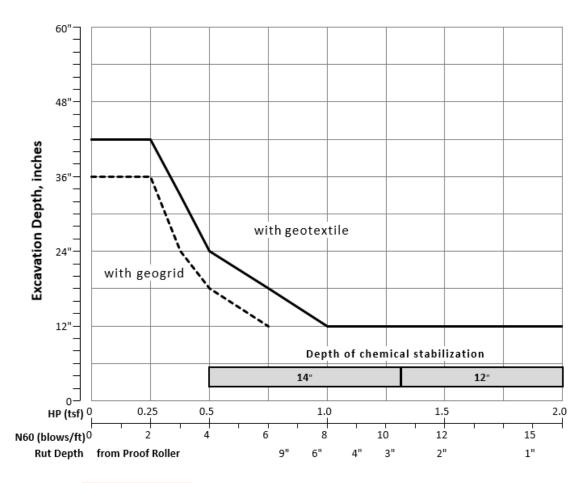
| % Samples within 6 feet of subgrade | | | | | | | | | | |
|-------------------------------------|-----|--------------|-----|--|--|--|--|--|--|--|
| N ₆₀ ≤ 5 | 0% | HP ≤ 0.5 | 1% | | | | | | | |
| N ₆₀ < 12 | 8% | 0.5 < HP ≤ 1 | 1% | | | | | | | |
| 12 ≤ N ₆₀ < 15 | 10% | 1 < HP ≤ 2 | 17% | | | | | | | |
| N ₆₀ ≥ 20 | 64% | HP > 2 | 40% | | | | | | | |
| M+ | 19% | | | | | | | | | |
| Rock | 3% | | · | | | | | | | |
| Unsuitable | 9% | | · | | | | | | | |

| Excavate and Replace at Surface | | | | | | | | | |
|------------------------------------|----|--|--|--|--|--|--|--|--|
| Average | 0" | | | | | | | | |
| Maximum | 0" | | | | | | | | |
| Minimum | 0" | | | | | | | | |

| % Proposed Subgrade Surface | | | | | | | | | |
|-----------------------------|-----|--|--|--|--|--|--|--|--|
| Unstable & Unsuitable | 45% | | | | | | | | |
| Unstable | 40% | | | | | | | | |
| Unsuitable | 6% | | | | | | | | |

| | N ₆₀ | N _{60L} | HP | ш | PL | PI | Silt | Clay | P 200 | Mc | M _{OPT} | GI |
|---------|-----------------|------------------|------|----|----|----|------|------|-------|----|------------------|----|
| Average | 28 | 18 | 3.06 | 21 | 15 | 6 | 26 | 15 | 41 | 13 | 10 | 3 |
| Maximum | 90 | 30 | 4.50 | 40 | 21 | 22 | 67 | 39 | 91 | 26 | 16 | 16 |
| Minimum | 6 | 6 | 0.50 | 15 | 12 | 1 | 6 | 4 | 11 | 5 | 0 | 0 |

| Classification Counts by Sample | | | | | | | | | | | | | | | | | | | |
|---------------------------------|------|-------|-------|-------|-------|-------|-------|-----|------|------|------|-----|------|------|-------|-------|------|------|--------|
| ODOT Class | Rock | A-1-a | A-1-b | A-2-4 | A-2-5 | A-2-6 | A-2-7 | A-3 | A-3a | A-4a | A-4b | A-5 | A-6a | A-6b | A-7-5 | A-7-6 | A-8a | A-8b | Totals |
| Count | 6 | 1 | 34 | 38 | 0 | 1 | 0 | 0 | 27 | 150 | 19 | 0 | 14 | 2 | 0 | 0 | 0 | 0 | 292 |
| Percent | 2% | 0% | 12% | 13% | 0% | 0% | 0% | 0% | 9% | 51% | 7% | 0% | 5% | 1% | 0% | 0% | 0% | 0% | 100% |
| % Rock Granular Cohesive | 2% | | | | | 86% | | | | | | | | 12 | 2% | | | | 100% |
| Surface Class Count | 4 | 1 | 21 | 28 | 0 | 1 | 0 | 0 | 12 | 76 | 5 | 0 | 5 | 1 | 0 | 0 | 0 | 0 | 154 |
| Surface Class Percent | 3% | 1% | 14% | 18% | 0% | 1% | 0% | 0% | 8% | 49% | 3% | 0% | 3% | 1% | 0% | 0% | 0% | 0% | 100% |



| , | VERRIDE TABLE | |
|--------------------|---------------|-------------------|
| Calculated Average | New Values | Check to Override |
| 3.06 | 0.50 | HP |
| 17.79 | 6.00 | N60L |

Average HP
Average N_{60L}











































https://youtu.be/Qj5tBPmLBXA









- Rubblize and Roll requirements
 - ≥75% with maximum dimension < 3 inches above steel
 - ≥90% with maximum dimension < 9 inches below steel (100% < 12 inches)

 All of the rubblized concrete met the gradation requirements, so no problem with the subgrade, right?











R&R Spec

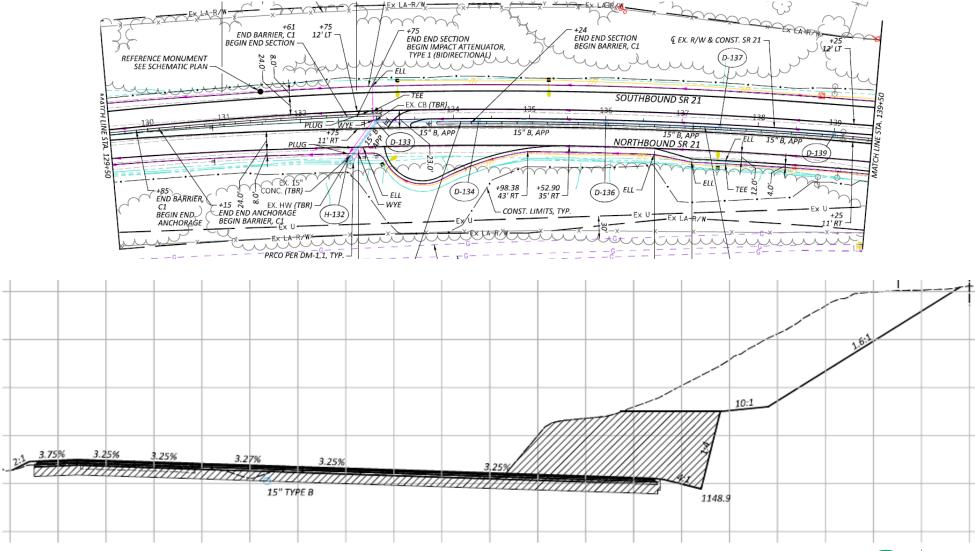
- Do not allow traffic on the rubblized pavement before the asphalt concrete base and intermediate courses are in place.
 - No proof rolling

 Do not allow more than 48 hours to elapse between rubblizing the pavement and placing the initial asphalt concrete course. However, in the event of rain, the Engineer may waive this time limitation to allow sufficient time for the rubblized pavement to dry to the Engineer's satisfaction.





SIDE BAR – R-CUT







SIDE BAR – R-CUT







• 2024

- NB constructed up to intermediate course of asphalt
- Outside shoulder of SB constructed (chemically stabilized subgrade, underdrains, asphalt)

• 2025

- Flip traffic to NB
- Construct SB
- Surface course on everything
- Complete the project one construction season early!





CONSTRUCTION SEASON WEATHER COMPARISON

| | Month | Precip | Monthly Average |
|--------------------------------|--------|--------|------------------------------------|
| Began milling mainline 6/25/24 | Jun-24 | 1.04 | 2.97 |
| | Jul-24 | 2.96 | |
| | Aug-24 | 6.61 | included a 4.25 inches on 8/4/2024 |
| Begn R&R 9/23/24 | Sep-24 | 3.4 | |
| Began paving over R&R 9/25/24 | Oct-24 | 0.85 | |
| | | | |
| Began milling mainline 3/24/25 | Apr-25 | 5.02 | 4.35 |
| Began R&R 6/5/25 | May-25 | 4.76 | |
| Began paving over R&R 6/6/25 | Jun-25 | 3.28 | |



























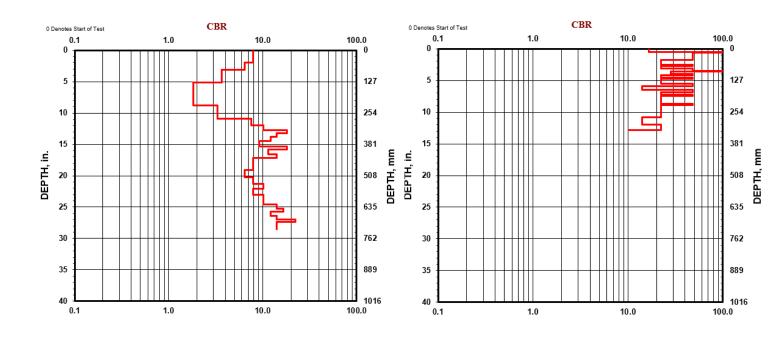
- During the placement of the base asphalt the adjacent lane paved previously began to show signs of failure under loading.
- Initial limits of concern: 3800 feet x 24 feet

 6/25/2025 Perform FWD and DCP, and test asphalt cores



Asphalt –Stability OK

DCP – Variable,
 but mostly OK



FWD – Alarming!





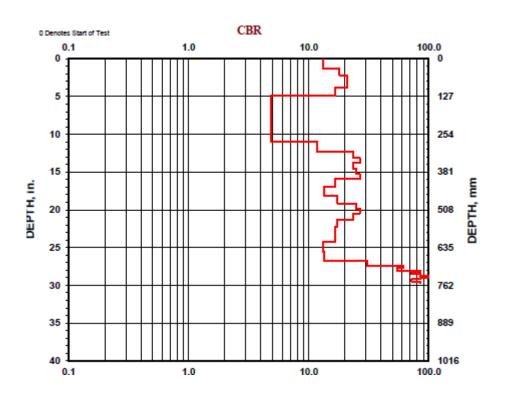
- FWD testing showed very high deflections, structure deficiencies of 5.5 to 7.5 inches.
- District now concerned about ALL R&R (19,175 feet).

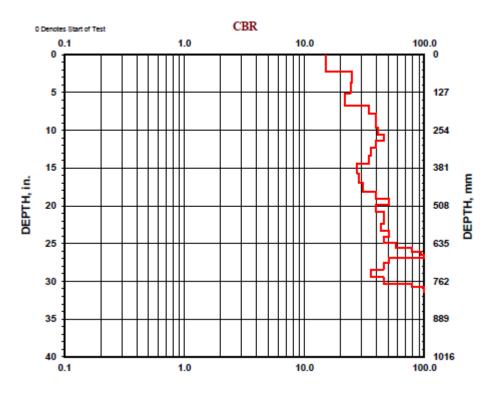
 Test the NB Section (no stability issues, in service) to confirm the design is OK.





• 7/9/2025 – 17 ADCP, FWD NB Section









 7/9/2025 FWD NB -Additional intermediate course needed by section

| Length | Start | Stop | Treatment | 7/9 NB |
|--------|--------|----------|----------------------|--------|
| 270 | 0+30 | 3+00 | Cement Stabilization | 0 |
| 4100 | 3+00 | 44+00 | Rubblize & Roll | 0.36 |
| 234 | 44+00 | 46+34 | Cement Stabilization | 0 |
| | | Warw | ick Bridge | |
| 182 | 48+09 | 49+90 | Cement Stabilization | 0 |
| | | Chippewa | Creek Bridge | |
| 219 | 52+81 | 55+00 | Cement Stabilization | 0 |
| 1700 | 55+00 | 72+00 | 0 | |
| 229 | 72+00 | 0 | | |
| | | | | |
| 183 | 78+17 | 80+00 | Cement Stabilization | 0 |
| 1350 | 80+00 | 93+50 | Rubblize & Roll | 0 |
| 232 | 93+50 | 95+82 | Cement Stabilization | 0 |
| | | Galeho | use Bridge | |
| 4459 | 98+31 | 142+90 | Undercut | 0 |
| 510 | 142+90 | 148+00 | Cement Stabilization | 0 |
| 4750 | 148+00 | 195+50 | Rubblize & Roll | 0.09 |
| 2475 | 195+50 | 220+25 | Cement Stabilization | 0 |
| 2375 | 220+25 | 244+00 | Rubblize & Roll | 0.88 |
| 1000 | 244+00 | 254+00 | Cement Stabilization | 0 |
| 4900 | 254+00 | 303+00 | Rubblize & Roll | 1.46 |
| 467 | 303+00 | 307+67 | Cement Stabilization | 0 |



- Now you know how we got here
- 7/14/2025 Proof roll 5 miles of pavement
- Much improved in the last month (drainage?)
- Isolated (and few) 1/8-1/4" deflection





- Run FWD
 (again) every
 150 feet
- Additional

 (asphalt)
 structure
 reduced over
 time

| 5104 | Total CY | Est. Add. Cost: \$1,339,813.37 | | | NB Passing Lane | SB Passing Lane | | | SB Drivi | ing Lane | | |
|------|----------|--------------------------------|--------|----------------------|--------------------|--------------------|------------|---------|----------|----------|------|----------------------|
| | | | | | | | | | | | | New Add Structure |
| CY | Length | Start | Stop | Treatment | 7/9 NB | Initial SB | Initial SB | 7/29 SB | Δ (%) | 8/13 SB | Δ(%) | (in.) |
| | 270 | 0+30 | 3+00 | Cement Stabilization | 0 | 0 | 0 | 0 | | 0 | | OK |
| 1082 | 4100 | 3+00 | 44+00 | Rubblize & Roll | 0.36 | 3.44 | 3.08 | 2.39 | 22% | 2.21 | 8% | 2.25 |
| | 234 | 44+00 | 46+34 | Cement Stabilization | 0 | 0 | 0 | 0 | | 0 | | OK |
| | | | Warw | ick Bridge | | | | | | | | |
| | 182 | 48+09 | 49+90 | Cement Stabilization | 0 | 0 | 0 | 0 | | 0 | | OK |
| | | Chippewa Creek Bridge | | | | | | | | | | |
| | 219 | 52+81 | 55+00 | Cement Stabilization | 0 | 0 | 0 | 0 | | 0 | | OK |
| | 1700 | 55+00 | 72+00 | Rubblize & Roll | 0 | 0.37 | 0.5 | 0.27 | | 0.04 | 85% | OK |
| | 229 | 72+00 | 74+29 | Cement Stabilization | 0 | 0 | 0 | 0 | | 0 | | OK |
| | | | CSX | Bridge | | | | | | | | |
| | 183 | 78+17 | 80+00 | Cement Stabilization | 0 | 0 | 0 | 0 | | 0 | | OK |
| | 1350 | 80+00 | 93+50 | Rubblize & Roll | 0 | 0.39 | 0.2 | 0.62 | | 0.33 | 47% | OK |
| | 232 | 93+50 | 95+82 | Cement Stabilization | 0 | 0 | 0 | 0 | | 0 | | OK |
| | | | Galeho | use Bridge | | | | | | | | |
| | 4459 | 98+31 | 142+90 | Undercut | 0 | 0 | 0 | 0 | | 0 | | OK |
| | 510 | 142+90 | 148+00 | Cement Stabilization | 0 | 0 | 0 | 0 | | 0 | | OK |
| 1114 | 4750 | 148+00 | 195+50 | Rubblize & Roll | 0.09 | 3.67 | 4.1 | 2.89 | 30% | 1.98 | 31% | 2 |
| | 2475 | 195+50 | 220+25 | Cement Stabilization | 0 | 0 | 0 | 0 | | 0 | | OK |
| 1184 | 2375 | 220+25 | 244+00 | Rubblize & Roll | 0.88 | 6.9 | 5.52 | 4.07 | 26% | 4.14 | -2% | 4.25 |
| | 1000 | 244+00 | 254+00 | Cement Stabilization | 0 | 0 | 0 | 0 | | 0 | | OK |
| 1724 | 4900 | 254+00 | 303+00 | Rubblize & Roll | 1.46 | 4.52 | 4.92 | 3.11 | 37% | 2.91 | 6% | 3 |
| | 467 | 303+00 | 307+67 | Cement Stabilization | 0 | 5.78 | 0 | 0 | | 0 | | |





- Average CBR estimated from FWD
- Increased an average of 13%
- Design CBR=9

| | | | NB Passing Lane | SB Passing Lane | SB Driving Lane | | | | |
|-----------------------|--------|----------------------|--------------------|--------------------|-----------------|---------|---------|------|-----------|
| Start | Stop | Treatment | 7/9 NB | Initial SB | Initial SB | 7/29 SB | 8/13 SB | % | |
| 0+30 | 3+00 | Cement Stabilization | 8 | 9 | 9 | 11 | 10 | 11% | |
| 3+00 | 44+00 | Rubblize & Roll | 14 | 13 | 12 | 15 | 14 | 17% | Deficient |
| 44+00 | 46+34 | Cement Stabilization | 10 | 10 | 9 | 9 | 10 | 11% | |
| Warwick Bridge | | | | | | | | | |
| 48+09 | 49+90 | Cement Stabilization | 12 | 14 | 12 | 14 | 13 | 8% | |
| Chippewa Creek Bridge | | | | | | | | | |
| 52+81 | 55+00 | Cement Stabilization | 10 | 9 | 11 | 9 | 12 | 9% | |
| 55+00 | 72+00 | Rubblize & Roll | 8 | 8 | 8 | 8 | 8 | 0% | |
| 72+00 | 74+29 | Cement Stabilization | 10 | 9 | 7 | 8 | 8 | 14% | |
| CSX Bridge | | | | | | | | | |
| 78+17 | 80+00 | Cement Stabilization | 8 | 9 | n/a | n/a | 8 | | |
| 80+00 | 93+50 | Rubblize & Roll | 7 | 8 | 9 | 10 | 11 | 22% | |
| 93+50 | 95+82 | Cement Stabilization | 6 | 11 | 9 | 11 | 12 | 33% | |
| Galehouse Bridge | | | | | | | | | |
| 98+31 | 142+90 | Undercut | 22 | 19 | 22 | 23 | 24 | 9% | |
| 142+90 | 148+00 | Cement Stabilization | 13 | 9 | 8 | 9 | 10 | 25% | |
| 148+00 | 195+50 | Rubblize & Roll | 10 | 12 | 10 | 14 | 14 | 40% | Deficient |
| 195+50 | 220+25 | Cement Stabilization | 12 | 12 | 11 | 13 | 14 | 27% | |
| 220+25 | 244+00 | Rubblize & Roll | 9 | 11 | 8 | 10 | 8 | 0% | Deficient |
| 244+00 | 254+00 | Cement Stabilization | 12 | 12 | 11 | 12 | 12 | 9% | |
| 254+00 | 303+00 | Rubblize & Roll | 11 | 12 | 12 | 9 | 9 | -25% | Deficient |
| 303+00 | 307+67 | Cement Stabilization | n/a | 8 | n/a | 8 | 10 | | |
| | | | | | | | | 13% | |





CONCLUSIONS

- Construction water management is important
- We are addicted to global chemical stabilization
 - weatherproof
- Proof roll rubblized concrete
- If we FWD all of our new pavements, will some of them be deficient?
- Anticipate 10-25% of R&R will be unstable





CONCLUSIONS

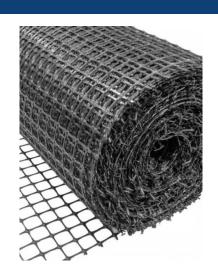


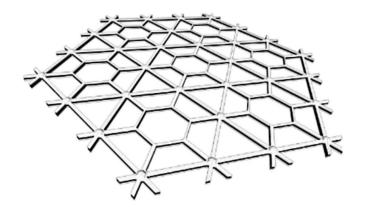




POSSIBLE SOLUTIONS

- Geogrid QPL –Type 2
 - Tensar SQ2020
 - \$2.75/SY installed
- Interax NX750
 - \$9.13/SY installed
 - "We observed some slight waving / rolling...due to pore pressure"













THANK YOU